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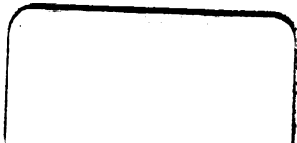
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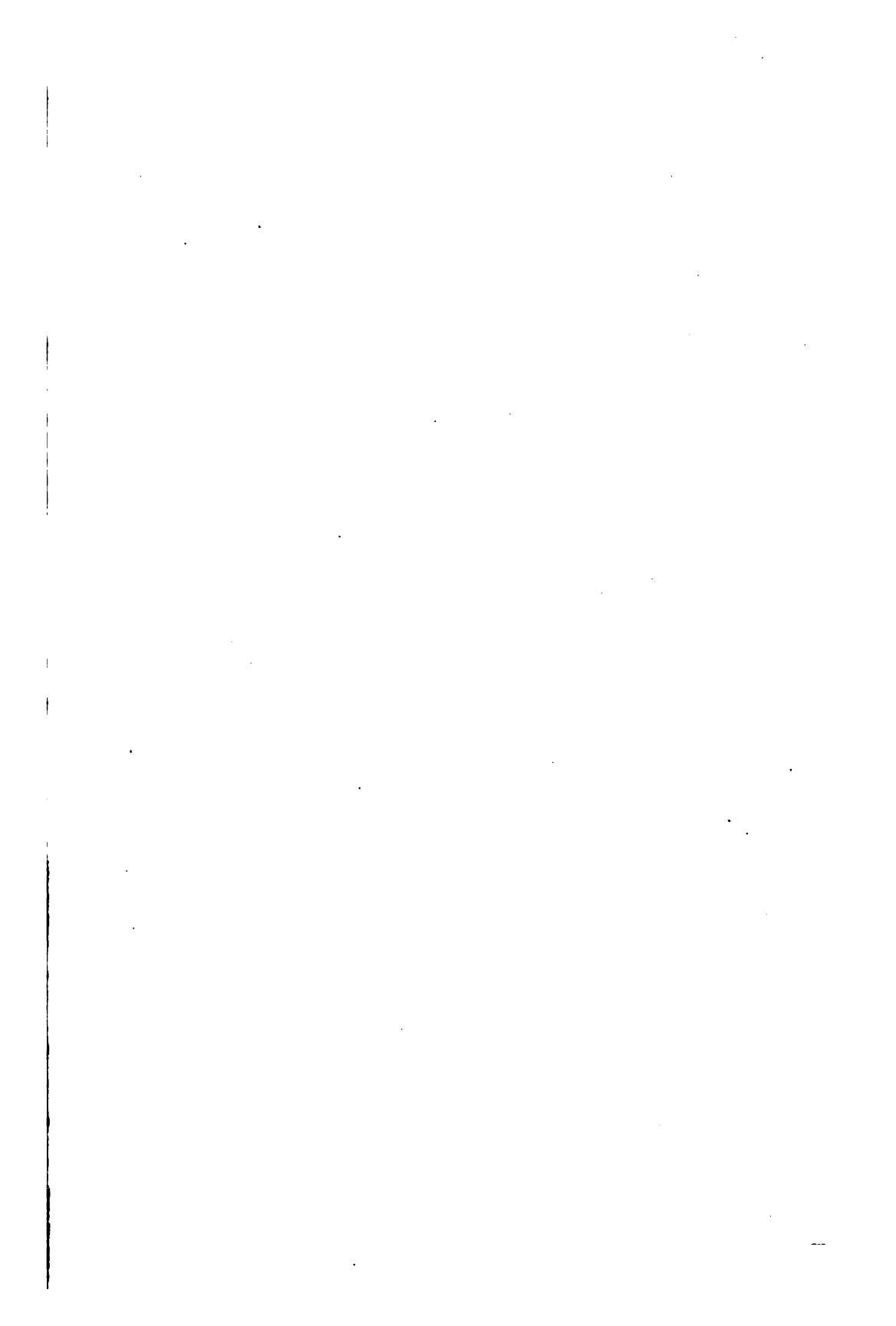
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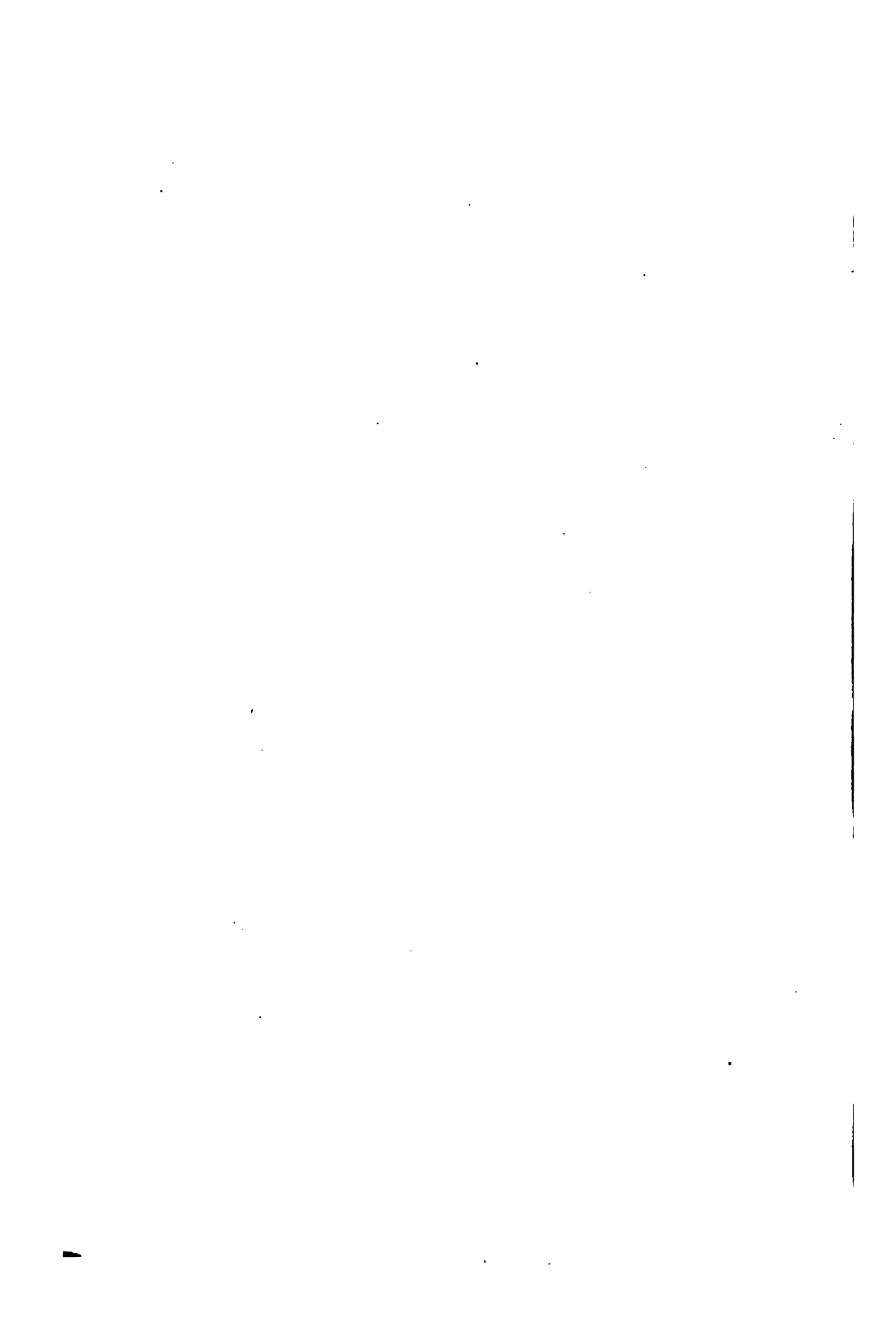












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GEOLOGICAL SURVEY OF ALABAMA

EUGENE ALLEN SMITH, STATE GEOLOGIST

**BULLETIN No. 7.**

A PRELIMINARY REPORT

ON A PART OF THE

**Water Powers of Alabama**

BY

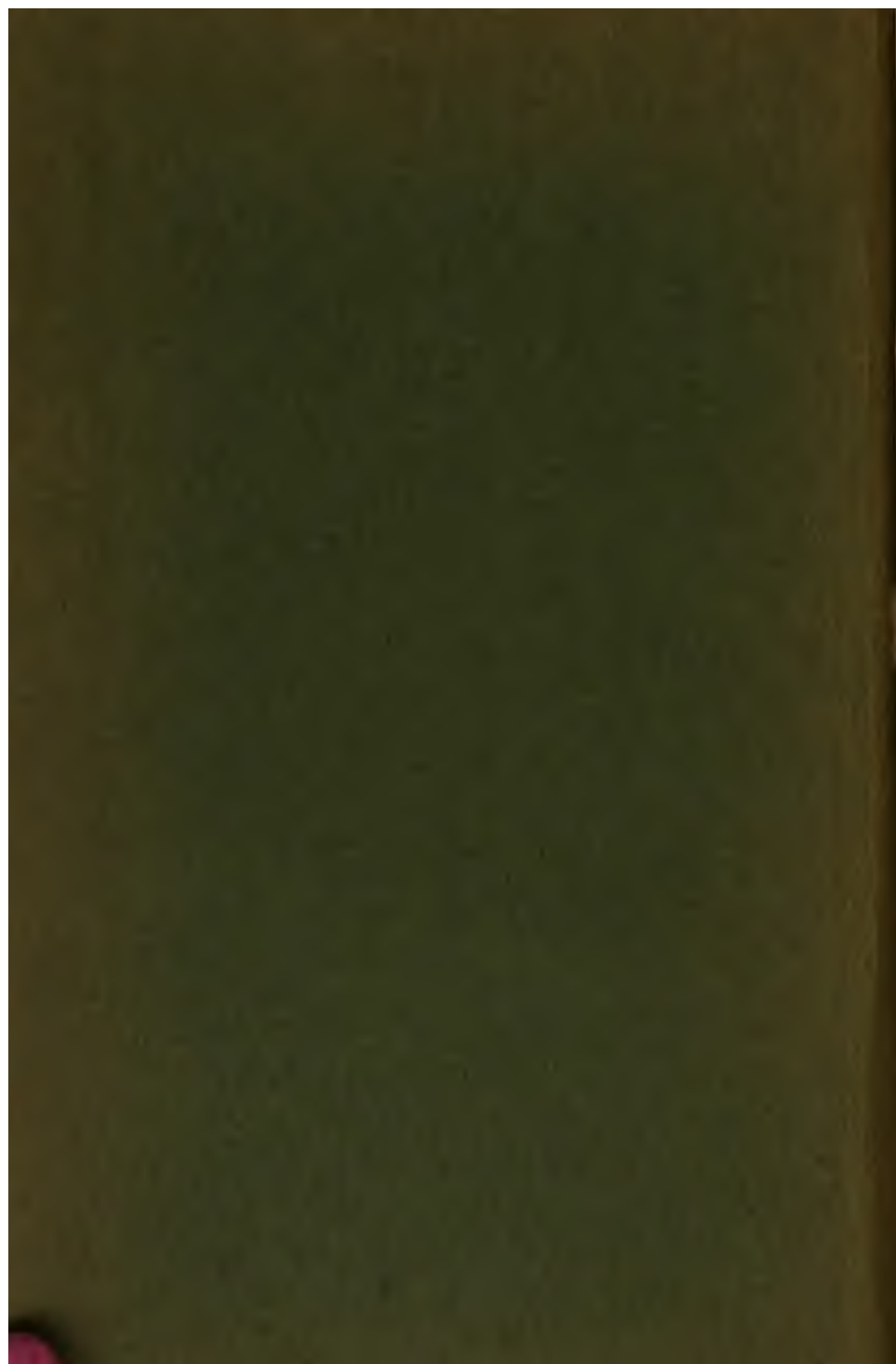
H. M. HALL,

CONSULTING ENGINEER, U. S. GEOL. SURVEY.

FOR GEORGIA, FLORIDA, TENNESSEE AND MISSISSIPPI.



1903



# GEOLOGICAL SURVEY OF ALABAMA

EUGENE ALLEN SMITH, STATE GEOLOGIST

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## BULLETIN No. 7

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BY

B. M. HALL,

\* CONSULTING ENGINEER, U. S. GEOL. SURVEY,  
FOR GEORGIA, FLORIDA, TENNESSEE AND MISSISSIPPI.



MONTGOMERY, ALA.:  
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*To His Excellency, William D. Jelks, Governor of Alabama:*

Dear Sir: I have the honor to transmit herewith a preliminary report on the Water Powers of Alabama, by B. M. Hall, of the United States Geological Survey, Consulting Engineer, Hydrographic Division, for Alabama, Florida, Georgia, Tennessee, and Mississippi.

Our Alabama Geological Survey, in coöperation with the United States Geological Survey, has for a number of years been engaged in a systematic investigation of the Water Resources of the State. In this investigation we have naturally been less interested in that portion of the rainfall which passes back into the atmosphere by evaporation, than in those portions which, temporarily at least, become more or less incorporated with the materials forming our land surface, and which on that account may be considered as forming a part of our territory. And our investigation of this earth water, (to use a term to distinguish it from the atmospheric water), may appropriately be followed along two lines: It may be concerned, 1, with that part of the water which, collecting in rivulets, creeks and rivers, flows on towards the sea by open channels, *i. e.*, the "run-off"; or 2, it may take into account that part which soaks into the ground, and reaches the water courses or the sea only after an underground passage of greater or less duration, *i. e.*, the ground water or the "in-soak," if we may be allowed the use of such a word.

While the proportion of the rainfall which appears in the run-off of the streams varies between very wide limits, depending on the geological formations, the locality, etc., in Alabama on an average, about fifty per cent. of the rainfall is lost by evaporation and the remainder forms the run-off of the streams, and, curiously enough, only a small percentage of this run-off is supplied by the surface water alone, for most of it reaches the water courses by underground seepage.

In the course of this underground circulation the water may reach the surface from springs, from ordinary shallow and deep wells, and from artesian wells, and may be utilized for domestic and municipal water supply, and rarely, in Alabama at least, for irrigation and for power.

The present writer has had charge of this branch of the investigation, and his report on the Artesian and other underground Water Systems of the State is now in manuscript, and practically ready for the printers.

Most of the material for this report has been collected by the Alabama Geological Survey.

The run-off, on the other hand, is utilized for transportation, for domestic and municipal water supply, and for power, and this branch of the subject has been in charge of Mr. Hall, who has for some years been employed by the United States Geological Survey in collecting records of the gage heights, and in making surveys and discharge measurements of the principal streams of Alabama (and adjacent States), from which the values of these streams for the various purposes above enumerated may be closely estimated. In the collection of these data, the Alabama Geological Survey has contributed to the extent of paying the observers of the gage heights at seven stations along Alabama streams, but with this exception and apart from the map, the present Report has been prepared without cost to the State of Alabama. We are also indebted to the United States Geological Survey for most of the illustrations which appear in the body of the Report, and these cuts, as well as most of the data from which this Report has been compiled by Mr. Hall, have been published in the Annual Reports of the Director of the National Geological Survey.

While the present report deals with only one of the many uses to which the run-off of our streams may be put, viz., for the production of power, this is in many respects, especially in Alabama, the most important of these uses, for the great increase in the applications of electricity has of late turned attention to the utilization for its production, of water powers which have heretofore been allowed to run to waste, and there can be little doubt but that in a comparatively short time, all the available water power of the State will be turned to account.

Very respectfully,

EUGENE A. SMITH,  
*State Geologist.*

University of Alabama,  
Dec. 1, 1902.





GEOLOGICAL SURVEY OF ALABAMA  
EUGENE ALLEN SMITH, STATE GEOLOGIST

# HYDROGRAPHIC MAP OF ALABAMA

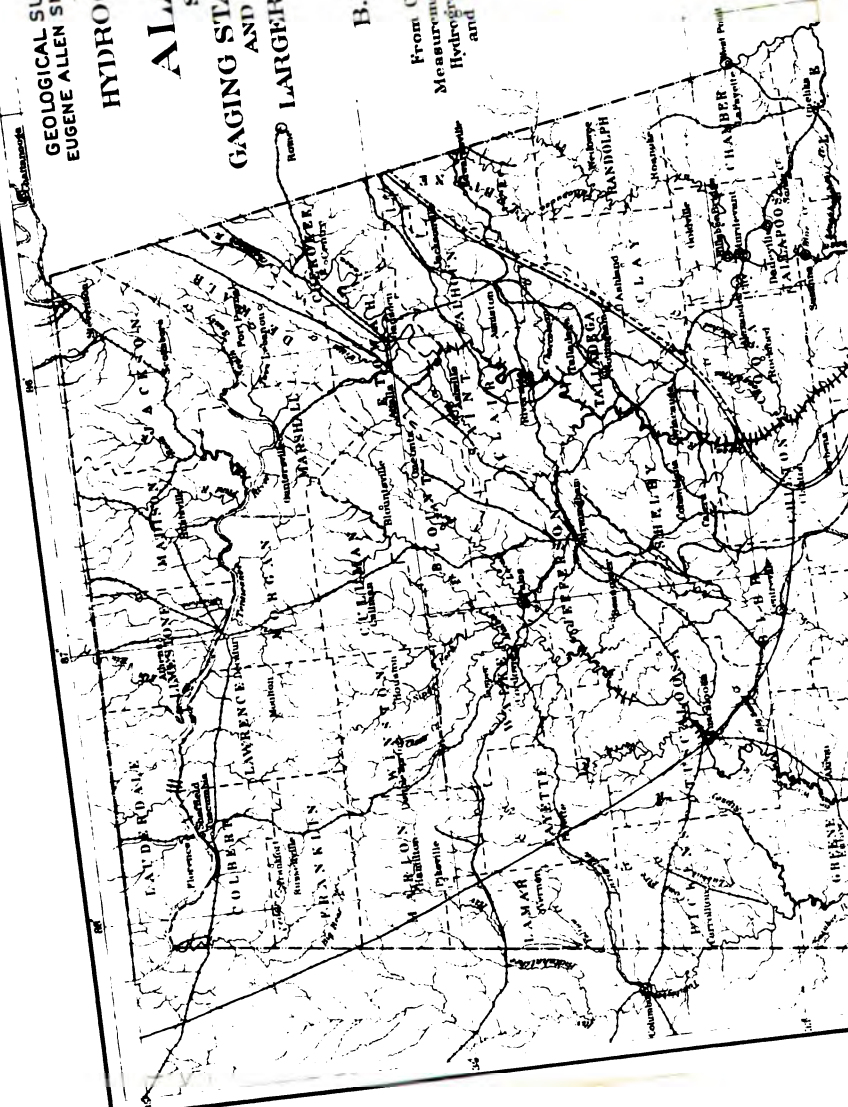
SHOWING  
GAGING STATIONS, FALL LINES,  
AND SOME OF THE  
LARGER WATER POWERS

Compiled by  
B.M. HALL, C. & M. E.,  
Consulting Engineer

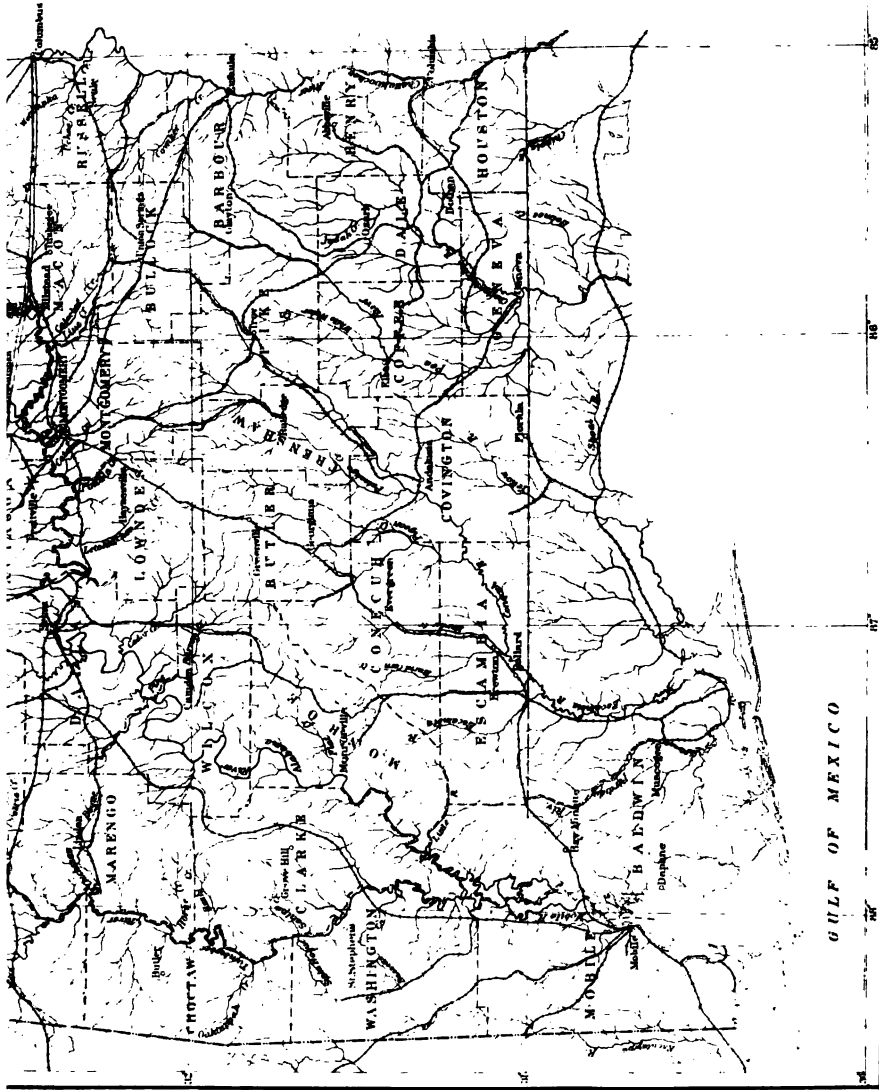
From Co-operative Surveys and River  
Measurements made under his Direction as  
Hydrographer for U. S. Geological Survey  
and ALABAMA Geological Survey  
1895 to 1902.

## LEGEND

- Hydrographic Gage Stations
- × Large Utilized Water Powers
- △ Large Undeveloped Water Powers
- Fall Line between Crystalline and Paleozoic
- Fall Line between Crystalline and Paleozoic



Note: The rivers are smaller.  
Streams are not shown.





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## PREFACE.

Very recently two large Water Powers have been developed on the Tallapoosa River, one of which is at Tallassee, Ala., and the other is three miles above Tallassee. These developments have awakened considerable interest in the undeveloped powers of the State, and this Bulletin undertakes to answer in a general way the numerous inquiries concerning them. Some of the largest of these unpublished powers are:

*Power Site No. 3*, on Tallapoosa River, at Double Bridges, about ten miles above Tallassee, where a head of 40 feet can be obtained. And other similar powers farther up the river.

*Black and Sanford Shoal* on Big Sandy Creek, near Dadeville, with 80 feet of fall.

*Thirty-one locks on the Coosa River*, capable of furnishing 1,300 to 4,500 horse power each, or an aggregate of 100,000 horse power during low season of an ordinary year like 1900.

*Seven power sites on the Cahaba River* capable of furnishing from 500 to 1,100 horse power each.

*Squaw Shoals* on the Black Warrior, with 43 feet of fall.

Also the following shoals on the Tennessee River:

Shoal:	Fall in feet.	Minimum H. P. dryest years.	Minimum H. P. average years.
Elk River Shoal .....	26	15,600	30,550
Big Muscle Shoal .....	85	51,000	99,875
Little Muscle Shoal .....	23	13,800	27,025
Colbert Shoal .....	21	12,600	24,675

These and other powers will be described more fully in Chapters II to VIII.

The water powers of Alabama are conveniently located for running cotton factories and other manufacturing plants, and also for generating electricity that can be transmitted to cities for power, light, etc. The larger powers are all close to water transportation, and are also on important railroads. These advantages will naturally make them more valuable than if they were otherwise located.

B. M. HALL.

Nov. 1, 1902.

## CHAPTER I.

---

### DRAINAGE BASINS, STREAMS, AND WATER POWERS.

---

#### DRAINAGE BASINS.

The five principal drainage basins of the State are:

First—The Apalachicola Basin, draining to the Chattahoochee and Apalachicola River, and entering the Gulf at Apalachicola, Fla.

Second—The Choctawhatchee Basin, draining to the Gulf through Choctawhatchee Bay.

Third—The Pensacola Basin, draining to Pensacola Bay and Perdido Bay, near Pensacola, Fla.

Fourth—The Mobile Basin, including the waters of Tallapoosa, Coosa, Cahaba, Alabama, Warrior, and Tombigbee Rivers, and draining into the Gulf at Mobile, Ala.

Fifth—The Tennessee Basin, draining into the Tennessee River, and thence through the Mississippi to the Gulf at New Orleans.

The water powers of the State are mainly in the Mobile and Tennessee Basins, which practically cover the entire State, except a small area in the southeast corner.

The area of crystalline rocks in Alabama is a triangle on the east side of the State, including Cleburne, Randolph, Chambers, Lee, Tallapoosa, Clay, Coosa, and parts of Elmore, Chilton, and Talladega counties. The "fall line," or escarpment dividing the Crystalline region from the Cretaceous formation of the Coastal plain on the southwest, runs from Columbus, Ga., crossing the Tallapoosa River at Tallassee, and the Coosa at Wetumpka. The northwestern boundary of the area of the Crystalline rocks which divides it from the Paleozoic formations, recrosses the Coosa River near Marble Valley postoffice, in Coosa county, and runs in a northeasterly direction towards Cedartown, Ga., crossing the Alabama line near Warner.

The line between the Paleozoic region and the Cretaceous formation runs from a point near Strasburgh, in Chilton county,



in a northwesterly direction to Tuscaloosa, thence in a northerly direction to a point near Tuscumbia, and thence northwesterly to the Mississippi line.

The southwestern boundary of the Cretaceous passes from Fort Gaines, approximately, through Clayton, Troy, Snow Hill, and Livingston, in a northwesterly direction.

It may be said in a general way that the streams have their greatest falls in passing from an older to a younger geological formation. Tallassee Falls, on the Tallapoosa, and Wetumpka Falls, on the Coosa, are made in passing from the Crystalline to the Cretaceous. Those on Talladega Creek and other small streams in entering the Coosa Valley from the southeast in Talladega, Calhoun, and Cleburne counties, are from the Crystalline to the Paleozoic. The shoals above Centerville, on the Cahaba, above Tuscaloosa, on the Black Warrior, and near Tuscumbia, on the Tennessee River, are made in passing from the Paleozoic to the Cretaceous. As the Coosa River runs off of the Paleozoic on to the Crystalline near Talladega Springs, the shoals above this point reverse the general order by being made in passing from a younger to an older formation.

#### STREAMS AND WATER POWERS.

The following is a statement, according to water-shed, of the important streams and such data concerning them as can be compiled from the work of the Alabama Geological Survey, the United States Weather Bureau, and the United States Engineering Corps, combined with the hydrographic investigations of the United States Geological Survey under the direction of the compiler of this report. Aside from certain surveys made to obtain maps and profiles of Tallapoosa River and Big Sandy Creek, the work done by the Hydrographic Division of the United States Geological Survey in this State deals exclusively with the amount of water flowing in the streams, and is intended to give a safe basis for calculation of low water volumes at all seasons of the year, and for several consecutive years, in order to arrive at their value for water power, irrigation, municipal supply, mining, navigation, etc. In order to do this certain convenient stations have been established on important rivers. At each of these stations a gage rod is set to show the fluctuations of the streams; and a gage reader is employed to observe the height of the water every morning at the same hour,

and to make a weekly report of the same to the Hydrographer-in-charge. As far as possible the river stations of the United States Weather Bureau and the United States Engineer Corps have been utilized for this purpose. From time to time the Hydrographer or one of his field assistants, visits the station and makes an accurate meter discharge measurement of the stream, noting the height of the water on the gage at the time the discharge measurement is made. After a number of such discharge measurements have been made at different gage-heights, a rating table is made from the data thus obtained, which gives the amount of water flowing in the stream, at that station, for any gage-height shown on the rod. Thus, by inspection of the table of daily gage-heights, the flow of the stream is shown for every day in the year, or years, covered by the observation of gage-height. At seasons of uniform low water, when the daily fluctuations of the rod are very slight for weeks at a time, discharge measurements are made of the stream at many points above and below the gage station in order to establish a relation between the discharge at these points and at the station. In like manner the principal tributaries are measured for the same purpose, where it is practicable to do so. In this way it is possible to arrive at a close estimate of the flow of all the streams of the water-shed, and make a rating of the gage for each that will represent its flow under average conditions, not including the floods caused by local rains. Such tributaries as have not been measured can be estimated by water-shed comparison with similar tributaries that have been measured.

In the following statement the actual gage-heights and discharge measurements are given in order to show the data upon which the conclusions are based. The regular gage stations that have been utilized are:

<i>Station.</i>	<i>Stream.</i>	<i>Observer.</i>	<i>Paid by.</i>
1—Milstead, Ala.....	Tallapoosa River	Seth Johnson...	Ala. Geo. Sv.
2—Sturdevant, Ala....	Tallapoosa River	B. F. Neighbors....	Ala. Geo. Sv.
3—Dadeville, Ala.....	Big Sandy Creek	T. H. Finch.....	Ala. Geo. Sv.
4—Alexander City, Ala.	Hillabee Creek...	J. H. Chisolm...	Ala. Geo. Sv.
5—Nottingham, Ala....	Talladega Creek	R. M. McClatchy	Ala. Geo. Sv.
6—Riverside, Ala.....	Coosa River.....	J. W. Foster.....	Ala. Geo. Sv.
7—Cordova, Ala.....	Black Warrior	R. A. B. Logan.....	Ala. Geo. Sv.
8—Montgomery, Ala....	Alabama River...	U. S. W. B.....	U. S. W. B.
9—Selma, Ala.....	Alabama River...	U. S. W. B.....	U. S. W. B.
10—Tuscaloosa, Ala....	Black Warrior	R. W. S. Wyman, Jr	U. S. Eng. C.
11—Eples, Ala.....	Tombigbee River	J. C. Horton.....	A. G. S. Ry.
12—Rome, Ga.....	Coosa River.....	W. M. Towers...	U. S. W. B.
13—Chattanooga, Tenn.	Tennessee River	U. S. W. B.....	U. S. W. B.

As the investigations in this State have been confined so far mainly to the Mobile and Tennessee basins, only the streams of these basins will be considered in the following discussion. It is to be remembered that from West Point, Ga., southwards, the line of Alabama is on the west bank of the Chattahoochee River, along the line where ordinary vegetation ceases to grow. This leaves all of the water power of the main stream on Georgia territory. There are many creeks flowing into the river from Alabama, some of which have considerable fall, as they come from a high plateau. Holland Creek, opposite Columbus, Ga., furnishes the Columbus water supply by gravity, having a fall of 117 feet in less than four miles. No doubt many of the others have as much fall, but as they have not been examined, a report on them cannot be made at present, but a recent reconnoissance along the Chattahoochee gives the following estimate of power obtained from some of them, 12 hours per day for each foot of fall, if the water is stored during the 12 idle hours:

Big Uchee Creek, Russell County.....	7 H. P. per foot of fall.
Ihagee Creek, Russell County.....	2 H. P. per foot of fall.
Hatchechubbee Creek, Russell County...	7 H. P. per foot of fall.
Cowikee Creek, Barbour County.....	11 H. P. per foot of fall.
Yattayabba Creek, Henry County.....	9 H. P. per foot of fall.
Omussee Creek, Henry county.....	7 H. P. per foot of fall.

---

#### EXPLANATION OF STATION RECORDS AND TABULAR STATEMENTS DEDUCED THEREFROM.

---

##### GAGE HEIGHTS.

The "Table of Gage Heights" is a record of the height of water on a gage rod, graded to feet and hundredths of a foot, set into the river vertically, and fastened permanently to a convenient tree or pier. The rod is read every day in the year, at the same time of day, which is about 8 o'clock in the morning. Inches are not used in these records, as the daily height of water on the gage is written in feet and decimals of a foot.

## DISCHARGE MEASUREMENTS.

These records show the date, the gage height at time of measurement, and the amount of water in cubic feet per second, or "second-feet," that is found by the measurement to be flowing in the river. (Second-feet means the same as cubic feet per second.) If we imagine a small stream filling a rectangular flume 1 foot wide and 1 foot deep, we have a stream whose sectional area is 1 square foot. The volume of this stream will vary in proportion to the speed with which the water flows through the flume. If the water is moving at a velocity of 1 foot per second, the flow or volume of water is 1 cubic foot per second, and would fill a vessel 5 feet wide, 5 feet long, and 4 feet deep in just 100 seconds, as such a vessel would hold 100 cubic feet of water. If the water in the flume 1 foot wide and 1 foot deep flows with a velocity of 2 feet per second, the volume will be 2 cubic feet per second, or 2 second-feet, and so on for any other velocity. In the same way if the flume is 20 feet wide, and 5 feet deep, its sectional area will be 100 square feet, and if the average velocity is 3 feet per second, the volume will be 300 cubic feet per second, or 300 second-feet. In each of the discharge measurements here enumerated, a cross-section of the stream is measured, and velocities taken with an electric current-meter at many points of the cross-section. Instead of multiplying the entire cross-section by an average velocity, the area was divided up into a large number of small sections by soundings from 5 to 10 feet apart, and the area of each of the small sections multiplied by the velocity at the small section, thus giving the second-feet flowing in each small section. The sum of the discharges of all the small sections makes the total discharge of the stream.

## RATING TABLE.

This is a table showing the discharges in second-feet (cubic feet per second) for all stages of water on the gage. Hence when the gage heights are known, the corresponding discharges can be taken from the rating table and written opposite each daily gage height, thus giving the flow in cubic feet per second on each day in the entire year.

## ESTIMATED MONTHLY DISCHARGE, ETC.

This table gives in the first three columns, the maximum, minimum, and mean discharge for each month in cubic feet per second (second feet.) Column No. 4 gives the "total acre feet" flowing down the stream during each month. An "acre-foot" is the amount of water that would be necessary to cover one acre with a depth of one foot, which is 43,560 cubic feet. It furnishes a convenient unit for storage, where the water is to be used for irrigation. A cubic foot is practically 7.48 gallons, and is usually estimated at 7.5 gallons. An acre-foot is 43,560 cubic feet, or 320,851 gallons. One cubic foot per second flowing for 24 hours will cover an acre to a depth of 1.98 feet. It is therefore customary in round numbers to state that a cubic foot per second for a day of 24 hours is equivalent to 2 acre feet. Now, as one inch of rainfall per hour falling for 12 hours would cover one acre a foot deep, it is evident that rainfall at the rate of 1 inch per hour will produce a flow of 1 cubic foot per second, or 2 acre feet per 24 hours for each acre of watershed, no allowance being made for evaporation or percolation. It is also convenient to remember that 1,000,000 gallons in a reservoir are equal to a little more than 3 acre feet (3.069). In a general way it may be said that water stored in reservoirs is reckoned in acre-feet for irrigation, cubic feet for water power, and in millions of gallons for city water supply.

Columns 5 and 6 give the "run-off" from the drainage area. The run-off in inches and decimals of an inch is given, just as rainfall is given. For instance, a run-off of 2.23 inches from a given drainage area, means that enough water ran off during the month to have covered the entire drainage area or water-shed to a depth of 2.23 inches. This is convenient in estimating the proportion of the rain-fall on any drainage area that can be stored for irrigation, city water supply, or other purposes. The run-off in second-feet per square mile of drainage area, is obtained by dividing the mean discharge for the month by the number of square miles in the drainage area, and is useful in estimating the mean discharge of a tributary whose drainage area is known, and in comparing different drainage areas. The "run-off" is not a fixed percentage of the rainfall, but is that part of the rainfall which is not lost by evaporation into the air, or by percolation in subterranean outlets. Being a remainder and

not a percentage, it necessarily forms a much larger proportion of a heavy annual rainfall than it does of a small annual rainfall. For instance, in the Crystalline region of Georgia or Alabama where the annual precipitation is 45 to 55 inches, the run-off from the water-sheds is equal to fully one-half of the rainfall, while in regions having a precipitation of only 10 to 20 inches annually, the run-off is frequently less than one-fifth of the rainfall. Again, the geological character of the water-shed makes a vast difference in the run-off, even where the annual rainfall is the same, and where practically the same conditions of climate, topography, forest area and cultivation exist. There will be a smaller run-off from the water-shed having permeable geological strata underneath it, into which a part of the rain water can percolate, and furnish the supply to artesian wells in the lower country under which the same strata run, without regard to surface topography. In a comparison of two such water-sheds, one in the crystalline region, and the other in a regularly stratified formation, the difference of run-off should form a basis for estimating the artesian supply obtainable from the latter as a fountain head.





PLATE A. Bridge at Millstead on Tallapoosa River.



## CHAPTER II.

### TALLAPOOSA RIVER AND TRIBUTARIES.

#### 1. TALLAPOOSA RIVER AT MILSTEAD, ALABAMA.

Tallapoosa River rises in the west-central part of Georgia and flows in a southwesterly direction into Alabama, where it joins the Coosa, to form Alabama River, 6 miles above Montgomery, Alabama. Its upper tributaries drain an area between the Chattahoochee and Coosa basins. At Tallassee, Alabama, it crosses the southern fall line. The shoals at this place have a fall of 60 feet, forming an obstruction to navigation. The drainage area is largely wooded, with cultivated fields at short intervals. A gaging station was established at Milstead on August 7, 1897, at the bridge of the Tallassee & Montgomery Railway, about one-fourth of a mile from Milstead, Alabama. The bridge is of iron, two spans of about 155 feet each, with short wooden trestles at each end. The initial point of measurement is the end of the iron bridge, left bank, downstream side. The rod of wire gage is fastened to outside of guard rail on downstream side of bridge. The bench mark is top of second cross beam from left-bank pier, downstream end, and is 60.00 feet above datum. The channel is straight at the bridge, and bends above and below. The current is sluggish at low water and obstructed by center pier of bridge. The banks are high, but overflow at extreme high water for several hundred feet on each side. The bed is fairly constant, and all water is confined to the main channel by railroad embankments. The observer is Seth Johnson, a farmer and fruit grower, Milstead, Alabama. The plate A opposite shows this station.

The following discharge measurements were made during 1897 by Max Hall:

May 3, gage height, 6.20 feet; discharge, 7,333 second-feet.  
July 15, gage height, 1.95 feet; discharge, 1,692 second-feet.  
August 7, gage height, 2.42 feet; discharge, 2,292 second-feet.  
September 4, gage height, 1.60 feet; discharge, 1,271 second-feet.  
November 23, gage height, 1.20 feet; discharge, 677 second-feet.  
December 16, gage height, 3.58 feet; discharge, 4,210 second-feet.

*\*Daily gage height, in feet, of Tallapoosa River at Milstead, Alabama, from August to December, 1897.*

Day.	Aug.	Sept.	Oct.	Nov.	Dec.	Day.	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	.....	1.70	0.80	0.90	1.50	17.....	1.90	1.10	0.80	1.20	2.80
2.....	.....	1.80	.80	1.00	1.50	18.....	2.20	1.00	.90	1.20	2.40
3.....	.....	1.60	.80	1.10	1.50	19.....	2.80	1.10	.90	1.20	2.10
4.....	.....	1.60	.70	1.10	1.60	20.....	9.70	1.10	.90	1.10	2.00
5.....	.....	1.60	.70	1.10	1.80	21.....	7.40	1.10	.80	1.20	1.90
6.....	.....	1.40	.70	1.10	2.00	22.....	8.50	1.00	.80	1.10	2.00
7.....	2.45	1.40	.70	1.10	2.10	23.....	5.30	1.00	.90	1.10	2.40
8.....	1.90	1.30	.60	1.10	2.00	24.....	3.40	1.00	.90	1.20	2.40
9.....	1.70	1.20	.70	1.20	1.90	25.....	2.90	1.00	.80	1.20	2.40
10.....	1.50	1.20	.70	1.20	1.90	26.....	2.80	1.00	.90	1.20	2.80
11.....	1.50	1.10	.70	1.30	1.90	27.....	2.30	1.00	.90	1.20	2.60
12.....	2.70	1.10	.70	1.30	2.00	28.....	2.00	.90	.90	1.60	2.60
13.....	2.20	1.10	.70	1.30	1.90	29.....	1.80	.90	.90	1.50	2.30
14.....	2.00	1.10	.80	1.30	5.50	30.....	1.70	.80	.90	1.50	2.20
15.....	1.80	1.40	.80	1.20	4.70	31.....	1.70	.....	.90	.....	2.40
16.....	1.60	1.20	.80	1.20	3.60						

\*See explanation, pages 11 to 14.

*Rating table for Tallapoosa River at Milstead, Alabama, for 1897.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second-feet.	Feet.	Second-feet.	Feet.	Second-feet.	Feet.	Second-feet.
0.5	330	1.5	1,070	3.0	3,129	5.0	5,909
0.6	350	1.6	1,200	3.2	3,407	5.2	6,187
0.7	380	1.7	1,333	3.4	3,685	5.4	6,465
0.8	420	1.8	1,467	3.6	3,963	5.6	6,743
0.9	470	1.9	1,600	3.8	4,241	5.8	7,021
1.0	530	2.0	1,733	4.0	4,519	6.0	7,299
1.1	620	2.2	2,007	4.2	4,797	7.0	8,689
1.2	720	2.4	2,285	4.4	5,075	8.0	10,079
1.3	830	2.6	2,573	4.6	5,353	9.0	11,469
1.4	950	2.8	2,851	4.8	5,631		

NOTE—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following discharge measurements were made during 1898 by Max Hall and others:

Jan. 19—Gage height, 2.13 feet; discharge, 1,889 second-feet.  
 Feb. 19—Gage height, 2.20 feet; discharge, 2,045 second-feet.  
 March 18—Gage height, 2.56 feet; discharge, 2,646 second-feet.  
 April 26—Gage height, 5.83 feet; discharge, 6,648 second-feet.  
 May 17—Gage height, 1.55 feet; discharge, 1,059 second-feet.  
 June 22—Gage height, 3.05 feet; discharge, 3,421 second-feet.  
 July 7—Gage height, 1.62 feet; discharge, 1,262 second-feet.  
 Aug. 5—Gage height, 13.67 feet; discharge, 15,295 second-feet.  
 Sept. 3—Gage height, 2.76 feet; discharge, 3,010 second-feet.  
 Nov. 29—Gage height, 5.16 feet; discharge, 5,477 second-feet.

*Daily gage height, in feet, of Tallapoosa River at Milstead, Alabama, for 1898.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	2.90	2.30	1.90	4.80	2.60	1.10	1.50	3.60	4.30	1.30	2.30	4.90
2.....	1.90	2.20	1.80	3.90	2.00	1.10	1.30	2.90	3.20	1.30	2.20	4.40
3.....	1.80	2.10	1.90	3.10	2.20	1.10	1.20	2.80	2.70	1.40	2.20	7.70
4.....	1.70	2.00	2.40	3.00	2.10	1.10	1.10	11.50	2.70	16.00	2.10	10.00
5.....	1.70	2.00	2.90	17.00	2.00	1.10	1.20	14.00	4.00	32.00	2.10	8.60
6.....	1.80	2.00	2.80	18.80	2.00	1.10	1.30	8.90	4.60	23.50	2.20	6.80
7.....	1.80	2.00	2.50	12.10	1.90	1.00	1.70	10.10	6.00	16.40	2.70	5.60
8.....	1.80	2.00	2.30	7.10	1.90	.90	2.90	8.50	5.60	22.80	4.20	4.80
9.....	1.90	2.00	2.20	5.20	1.80	.90	4.90	7.90	4.50	14.00	2.80	4.60
10.....	1.90	1.90	2.00	4.00	1.80	.90	3.60	5.30	3.50	7.90	3.00	6.70
11.....	1.90	1.90	2.00	3.40	1.70	.80	4.40	17.30	2.90	5.90	7.10	6.10
12.....	2.40	1.90	2.00	3.10	1.70	.90	2.00	22.60	2.60	4.90	5.10	5.50
13.....	2.40	1.90	1.90	3.00	1.70	1.70	1.70	10.10	2.50	3.80	5.00	5.00
14.....	2.60	1.90	1.90	2.90	1.60	1.20	2.70	7.70	2.40	3.40	5.90	4.40
15.....	2.40	1.80	2.70	2.80	1.60	1.10	3.50	6.10	2.20	3.00	5.50	4.00
16.....	2.40	1.80	2.60	2.60	1.60	1.20	5.10	4.20	2.00	2.70	5.40	3.80
17.....	2.40	1.80	2.50	2.40	1.60	2.10	2.40	3.40	1.80	2.60	6.20	3.60
18.....	2.30	1.90	2.60	2.40	1.60	1.80	2.80	2.80	1.70	4.40	6.10	3.40
19.....	2.20	2.20	2.50	2.20	1.50	1.40	3.05	2.60	1.60	5.00	14.40	4.30
20.....	2.20	2.30	2.40	2.40	1.50	1.40	2.30	2.80	1.60	4.40	12.20	7.60
21.....	3.10	2.30	2.20	2.80	1.40	1.40	1.90	2.50	1.50	3.70	10.00	7.00
22.....	3.10	2.10	2.00	2.70	1.40	3.00	1.60	2.30	1.50	3.30	8.00	5.20
23.....	3.00	2.00	1.90	2.80	1.30	2.40	1.60	2.20	1.50	3.10	10.00	4.40
24.....	2.80	2.00	1.90	14.50	1.30	2.20	1.50	2.00	1.60	2.70	8.80	6.40
25.....	2.60	1.90	1.80	11.60	1.20	2.10	1.50	2.10	1.80	2.60	7.90	6.00
26.....	2.90	1.80	1.80	5.90	1.20	1.80	2.50	8.10	2.00	2.50	5.30	4.60
27.....	3.60	1.90	1.80	4.30	1.20	1.50	2.60	10.20	1.80	2.40	4.60	4.20
28.....	3.90	1.90	1.70	3.30	1.30	1.80	4.10	8.40	1.60	2.40	4.10	3.80
29.....	3.10	.....	2.20	2.95	1.20	2.70	2.80	7.00	1.50	2.40	4.90	3.70
30.....	2.65	.....	4.20	2.80	1.20	1.90	2.90	5.20	1.50	2.40	5.40	3.70
31.....	2.40	.....	5.30	.....	1.10	.....	3.80	5.10	.....	2.30	.....	4.00

*Rating table for Tallapoosa River at Milstead, Alabama, for 1898.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second Ft.	Feet.	Second Ft.	Feet.	Second Ft.
0.8	540	2.4	2,380	8.0	8,820
0.9	655	2.5	2,495	8.5	9,395
1.0	770	2.6	2,610	9.0	9,970
1.1	885	2.7	2,725	9.5	10,545
1.2	1,000	2.8	2,840	10.0	11,120
1.3	1,115	2.9	2,955	10.5	11,695
1.4	1,230	3.0	3,070	11.0	12,270
1.5	1,345	3.5	3,645	11.5	12,845
1.6	1,460	4.0	4,220	12.0	13,420
1.7	1,575	4.5	4,795	12.5	13,995
1.8	1,690	5.0	5,370	13.0	14,570
1.9	1,805	5.5	5,945	13.5	15,145
2.0	1,920	6.0	6,520	14.0	15,720
2.1	2,035	6.5	7,095	14.5	16,295
2.2	2,150	7.0	7,670	15.0	16,870
2.3	2,265	7.5	8,245		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following discharge measurements were made during 1899 by Max Hall:

April 17—Gage height, 6.34 feet; discharge, 7,444 second-feet.  
 April 18—Gage height, 5.63 feet; discharge, 6,853 second-feet.  
 May 17—Gage height, 2.80 feet; discharge, 3,000 second-feet.  
 June 26—Gage height, 2.05 feet; discharge, 1,847 second-feet.  
 September 9—Gage height, 1.36 feet; discharge, 1,016 second-feet.  
 November 8—Gage height, 1.25 feet; discharge, 972 second-feet.  
 December 18—Gage height, 2.66 feet; discharge, 2,844 second-feet.

# WATER-POWERS OF ALABAMA.

19

*Daily gage height, in feet, of Tallapoosa River, at Milstead, Alabama, for 1899.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	5.00	17.00	27.00	18.00	4.30	2.40	2.80	3.00	2.40	0.70	1.50	2.40
2.....	4.60	11.60	19.00	12.20	4.00	2.90	2.30	2.70	2.30	.70	1.40	2.50
3.....	3.90	24.50	13.50	6.50	3.90	2.40	1.90	3.10	2.10	.80	1.40	2.60
4.....	3.90	20.00	9.60	6.40	3.80	2.40	1.80	3.10	2.00	.90	1.30	2.60
5.....	3.90	12.90	14.20	8.50	3.70	2.30	1.60	2.50	1.80	1.00	1.30	2.90
6.....	3.80	11.90	13.20	9.70	3.60	2.20	1.50	2.30	1.60	1.40	1.20	2.20
7.....	7.40	17.50	10.10	10.30	3.50	2.10	1.50	2.20	1.60	1.40	1.20	2.00
8.....	8.00	27.00	8.60	13.00	3.60	2.00	2.00	2.00	1.40	1.30	1.20	1.90
9.....	7.10	19.00	7.70	13.00	3.50	1.90	2.20	1.80	1.40	1.30	1.20	1.80
10.....	6.40	13.80	7.20	11.20	3.40	1.70	2.00	1.60	1.30	1.30	1.20	1.90
11.....	18.50	10.00	6.80	8.40	3.80	1.90	1.80	1.50	1.20	1.40	1.30	2.00
12.....	16.80	8.30	6.60	7.00	3.20	2.00	1.70	1.70	1.20	1.40	1.30	15.20
13.....	13.00	7.40	6.50	6.50	3.10	2.00	1.50	1.60	1.20	1.30	1.30	13.20
14.....	11.60	7.00	6.40	6.20	3.10	2.70	1.40	1.60	1.10	1.30	1.30	8.20
15.....	9.40	6.10	7.20	6.00	3.00	2.70	1.30	1.50	1.00	1.20	1.30	5.00
16.....	7.80	10.40	12.20	7.10	2.90	2.60	1.30	4.00	1.00	1.10	1.30	3.70
17.....	12.70	11.50	11.00	6.60	2.80	2.10	1.20	3.90	1.00	1.20	1.60	3.00
18.....	10.00	10.60	10.20	5.60	2.70	1.90	1.10	2.20	1.00	1.10	1.50	2.60
19.....	8.00	9.30	14.80	5.50	2.60	1.80	1.60	1.90	1.10	1.20	1.50	2.50
20.....	6.50	8.30	13.90	5.40	2.50	1.80	1.40	1.70	1.00	1.30	1.50	2.70
21.....	5.70	8.30	10.40	5.20	2.60	1.70	8.40	1.50	.90	1.40	1.40	2.70
22.....	5.30	8.40	8.30	5.00	2.70	1.50	16.75	1.60	1.00	1.50	1.40	2.70
23.....	5.10	7.60	8.10	4.90	2.60	1.50	14.00	2.00	1.00	1.60	1.60	2.60
24.....	5.20	6.90	12.70	6.00	3.30	1.50	16.95	2.60	.90	1.80	1.80	9.30
25.....	5.20	6.40	8.70	10.00	4.60	1.50	7.90	1.90	.90	1.50	2.20	9.40
26.....	5.10	6.30	7.30	7.50	3.30	2.00	6.70	1.80	.90	1.40	4.60	7.20
27.....	4.80	25.00	6.90	6.60	2.80	2.50	6.80	3.70	.90	1.30	6.20	5.00
28.....	4.70	37.00	6.80	5.80	2.60	2.50	8.40	2.80	.80	1.20	4.80	4.00
29.....	5.10	.....	9.00	4.90	2.50	2.20	10.10	2.10	.80	1.50	3.60	3.50
30.....	5.20	.....	8.90	4.60	2.60	2.70	6.40	1.90	.80	1.50	2.80	3.10
31.....	6.50	.....	13.85	.....	2.50	.. ..	4.40	2.30	.....	1.60	.....	2.90

*Rating table for Tallapoosa River at Milstead, Alabama, for 1899.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second Ft.	Feet.	Second Feet.	Feet.	Second Ft.
0.7	320	6.5	7,437	17.5	20,977
0.8	430	7.0	8,052	18.0	21,582
0.9	550	7.5	8,667	18.5	22,179
1.0	672	8.0	9,282	19.0	22,812
1.1	795	8.5	9,897	19.5	23,427
1.2	918	9.0	10,512	20.0	24,042
1.3	1,041	9.5	11,127	20.5	24,657
1.4	1,164	10.0	11,742	21.0	25,272
1.5	1,287	10.5	12,357	21.5	25,887
1.6	1,410	11.0	12,972	22.0	26,502
1.7	1,533	11.5	13,587	22.5	27,117
1.8	1,656	12.0	14,202	23.0	27,732
1.9	1,779	12.5	14,817	23.5	28,347
2.0	1,902	13.0	15,432	24.0	28,962
2.5	2,517	13.5	16,047	24.5	29,577
3.0	3,132	14.0	16,662	25.0	30,192
3.5	3,747	14.5	17,277	25.5	30,807
4.0	4,362	15.0	17,892	26.0	31,422
4.5	4,977	15.5	18,507	26.5	32,037
5.0	5,592	16.0	19,122	27.0	32,652
5.5	6,207	16.5	19,737	27.5	33,267
6.0	6,822	17.0	20,352	27.9	33,779

NOTE—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

During the year 1900 the following discharge measurements were made by Max Hall:

Feb. 23—Gage height, 9.20 feet; discharge, 9,956 second-feet.

March 5—Gage height, 6.70 feet; discharge, 7,088 second-feet.

Dec. 3—Gage height, 2.95 feet; discharge, 3,031 second-feet.

*Daily gage height, in feet, of Tallapoosa River near Milledge, Alabama, for 1900.*

Day.	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	2.70	2.50	13.20	5.50	6.30	2.60	9.00	6.50	2.70	1.80	2.50	3.30
2.....	2.60	2.40	13.10	5.30	5.90	2.70	9.10	4.50	8.00	1.60	3.30	3.30
3.....	2.40	2.30	10.70	5.00	5.40	3.00	10.60	3.50	8.10	1.50	6.10	3.30
4.....	2.30	4.50	8.00	5.30	4.90	3.50	7.10	2.80	4.40	1.80	6.50	3.50
5.....	2.30	4.20	6.80	4.80	4.70	3.30	8.00	2.70	3.00	3.30	7.10	3.90
6.....	2.30	4.30	6.10	4.70	3.90	3.50	5.20	2.50	2.40	6.40	5.50	3.90
7.....	2.20	4.40	5.60	4.60	3.80	3.30	4.40	2.30	2.20	3.40	4.10	3.80
8.....	2.20	3.80	10.90	4.60	3.70	5.90	4.00	2.20	2.00	3.20	3.50	3.80
9.....	2.20	5.20	13.80	4.50	3.60	9.70	3.50	2.10	1.80	2.60	3.00	3.10
10.....	2.20	8.90	12.70	4.60	3.50	6.90	7.60	2.00	1.70	2.60	2.90	3.10
11.....	2.60	19.00	10.00	5.50	3.30	5.60	5.50	1.90	1.70	3.00	2.70	2.80
12.....	7.30	30.00	7.90	10.60	3.20	4.60	4.10	1.90	1.60	2.80	2.60	2.80
13.....	6.00	43.25	6.60	11.50	3.10	5.30	4.70	1.80	1.60	3.40	2.60	2.80
14.....	4.50	42.00	5.90	9.00	3.00	4.00	6.70	1.80	1.60	3.30	2.50	7.70
15.....	4.00	31.90	5.40	6.60	2.90	3.80	6.00	2.30	14.00	3.30	2.50	9.10
16.....	3.40	22.80	7.00	5.30	2.90	3.50	5.10	2.70	25.60	2.70	2.50	7.30
17.....	3.00	13.50	7.20	4.70	2.80	4.50	3.80	3.10	18.00	2.70	2.50	5.20
18.....	2.90	8.90	5.60	13.90	2.80	5.90	3.80	2.50	11.00	2.20	2.50	4.20
19.....	3.70	7.00	6.40	17.00	2.80	5.10	3.60	4.00	5.30	2.10	2.50	4.00
20.....	9.50	6.10	6.00	15.00	2.90	5.90	3.10	4.20	8.60	2.00	2.50	8.60
21.....	7.50	6.50	11.40	16.90	2.80	5.90	3.00	2.50	8.00	1.90	2.50	17.00
22.....	5.90	9.80	10.50	13.30	3.00	5.50	2.80	2.20	2.50	2.00	4.00	13.50
23.....	4.60	9.50	7.60	10.30	3.10	5.40	2.70	2.00	2.30	4.80	4.00	10.40
24.....	3.90	8.90	15.50	13.20	3.40	20.00	2.60	2.30	2.20	12.10	3.90	11.30
25.....	3.50	8.40	15.20	12.50	3.50	25.04	2.50	3.30	2.00	10.50	8.90	8.70
26.....	3.10	8.00	16.00	9.40	5.00	20.00	2.50	3.40	2.00	9.00	10.50	6.60
27.....	3.00	7.00	13.70	7.50	4.50	16.00	2.80	8.60	2.00	6.00	8.80	5.60
28.....	2.80	6.00	11.20	6.40	3.20	18.00	2.90	2.70	2.00	4.20	6.70	5.00
29.....	2.70	.....	8.70	6.20	2.70	13.80	3.20	2.60	2.00	2.20	4.50	4.60
30.....	2.60	.....	7.10	6.10	3.00	9.00	8.10	2.10	1.90	2.90	8.70	4.30
31.....	2.50	.....	6.20	.....	2.50	.....	10.60	3.20	.....	2.70	.....	11.00

The following measurements were made by Max Hall and James R. Hall during 1901:

Feb. 12—Gage height, 10.70 feet; discharge, 11,759 second-feet.

March 13—Gage height, 5.55 feet; discharge, 5,644 second-feet.

October 29—Gage height, 1.70 feet; discharge, 1,583 second-feet.

*Daily gage height of Tallapoosa River at Milledgeville, Ala., for 1901.*

Day.	Jan.	Feb.	Mar.	Apr.	May.	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	14.30	7.40	5.30	26.00	5.00	8.10	5.10	2.10	4.00	2.20	1.70	2.00
2.....	15.50	7.20	5.20	22.00	4.80	8.60	4.20	2.20	3.50	2.90	1.70	2.00
3.....	14.50	7.50	5.10	23.00	4.70	9.7	4.30	2.30	2.90	6.10	1.70	1.95
4.....	11.00	24.07	5.00	18.00	4.60	10.80	4.00	2.20	2.70	6.40	1.80	2.20
5.....	9.80	26.00	4.90	14.20	4.40	8.00	3.30	2.10	2.40	5.10	1.95	2.40
6.....	8.70	18.30	4.70	10.80	4.30	7.1	3.60	2.10	2.20	4.00	1.95	2.40
7.....	7.40	12.60	4.50	9.60	4.20	12.10	4.00	2.60	2.10	3.20	2.00	2.30
8.....	6.10	10.50	4.40	7.20	4.20	10.10	4.80	2.60	2.00	2.50	1.90	2.20
9.....	5.30	13.10	4.40	6.80	4.10	6.30	4.70	2.30	2.00	2.10	1.90	2.20
10.....	5.00	14.00	4.70	6.20	3.90	5.90	3.70	2.30	1.90	2.00	2.00	2.20
11.....	6.00	12.60	7.70	5.80	3.80	4.50	2.90	2.10	1.90	1.90	1.90	2.30
12.....	24.20	10.90	6.00	5.60	3.80	4.10	2.60	2.30	1.80	1.90	1.80	2.50
13.....	30.50	9.80	5.50	6.10	3.80	4.10	2.60	2.90	1.80	1.90	1.80	2.40
14.....	22.00	8.20	6.00	8.80	4.50	5.1	2.10	2.50	2.80	2.00	1.75	2.60
15.....	18.00	7.70	5.50	10.70	4.30	7.30	2.20	2.10	4.20	2.10	1.75	6.00
16.....	11.50	6.90	5.10	8.70	4.00	5.70	2.30	3.20	3.10	2.10	1.80	13.40
17.....	12.20	6.60	4.50	7.70	3.60	6.00	5.30	8.90	3.00	2.00	1.80	9.00
18.....	12.10	6.50	4.10	11.00	3.50	4.80	8.00	5.60	1.90	1.90	1.80	6.00
19.....	9.70	6.30	4.00	23.00	3.00	4.40	4.30	6.60	9.10	1.90	1.90	3.90
20.....	8.00	6.20	6.00	22.00	5.50	4.00	3.50	5.90	7.10	1.80	2.10	3.60
21.....	7.10	5.90	7.90	13.40	10.50	3.1	3.30	9.60	5.00	1.80	2.20	3.40
22.....	6.80	5.80	6.30	10.70	16.20	3.30	3.10	7.20	3.80	1.70	2.20	3.20
23.....	6.40	5.70	6.20	8.90	14.20	3.30	2.80	20.75	2.70	1.70	2.30	3.10
24.....	6.20	6.00	10.10	7.60	12.00	3.20	3.50	21.00	2.30	1.70	2.20	3.10
25.....	7.00	6.10	8.90	6.90	9.00	9.10	2.40	9.40	2.10	1.70	2.20	3.00
26.....	6.80	6.10	10.20	6.50	10.00	3.10	2.20	5.90	2.10	1.60	2.10	2.90
27.....	6.70	6.00	14.30	6.10	8.40	2.90	2.50	4.20	2.00	1.70	2.30	2.95
28.....	6.70	5.50	17.50	5.80	6.40	2.80	2.50	5.90	2.10	1.70	2.20	3.05
29.....	6.60	.....	15.00	5.50	6.00	2.60	2.30	8.70	2.30	1.70	2.10	38.00
30.....	6.80	.....	9.90	5.20	5.10	2.60	2.20	7.70	2.50	1.70	2.00	47.00
31.....	7.00	.....	31.50	.....	4.80	.....	2.20	4.70	.....	1.70	.....	39.00



*Rating table for Tallapoosa River at Miltstead, Ala., for 1900 and 1901.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second Ft.	Feet.	Second Ft.	Feet.	Second Ft.
1.5	1,337	4.1	3,262	18.0	19,900
1.6	1,460	4.2	4,375	19.0	21,025
1.7	1,562	4.3	4,487	20.0	22,150
1.8	1,675	4.4	4,600	21.0	23,275
1.9	1,787	4.5	4,712	22.0	24,400
2.0	1,900	4.6	4,825	23.0	25,525
2.1	2,012	4.7	4,937	24.0	26,650
2.2	2,125	4.8	5,050	25.0	27,775
2.3	2,237	4.9	5,162	26.0	28,900
2.4	2,350	5.0	5,275	27.0	30,025
2.5	2,462	5.5	5,837	28.0	31,150
2.6	2,575	6.0	6,400	29.0	32,275
2.7	2,687	6.5	6,962	30.0	33,400
2.8	2,800	7.0	7,525	31.0	34,525
2.9	2,912	7.5	8,087	32.0	35,650
3.0	3,025	8.0	8,650	33.0	36,775
3.1	3,137	8.5	9,212	34.0	37,900
3.2	3,250	9.0	9,775	35.0	39,025
3.3	3,362	10.0	10,900	36.0	41,150
3.4	3,475	11.0	12,025	37.0	41,275
3.5	3,587	12.0	13,150	38.0	42,400
3.6	3,700	13.0	14,275	39.0	43,525
3.7	3,812	14.0	15,400	40.0	44,650
3.8	3,925	15.0	16,525	41.0	45,775
3.9	4,037	16.0	17,650	42.0	46,900
4.0	4,150	17.0	18,775	43.0	48,025

NOTE—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*\*Estimated monthly discharge of Tallapoosa River at Milstead, Ala.*

[Drainage area, 3,840 square miles.]

Month.	Discharge in second-feet.			Total in acre-feet.	Run-off.	
	Maximum.	Minimum.	Mean.		Depth in inches.	Second-feet per square mile.
1897.						
August, 7-31 .....	12,440	1,070	3,173	157,340	0.77	0.83
September .....	1,467	420	742	44,155	0.21	0.19
October .....	470	380	424	26,070	0.12	0.11
November .....	1,200	470	729	43,379	0.21	0.19
December .....	6,604	1,070	2,214	136,135	0.67	0.58
1898.						
January .....	4,105	1,575	2,426	149,170	0.72	0.63
February .....	2,265	1,690	1,912	106,187	0.52	0.50
March .....	5,715	1,575	2,313	142,222	0.69	0.60
April .....	21,240	2,150	5,748	342,029	1.67	1.50
May .....	2,610	885	1,493	91,802	0.45	0.39
June .....	3,070	540	1,314	78,188	0.38	0.34
July .....	5,485	885	2,493	153,290	0.75	0.65
August .....	25,610	1,920	7,418	456,118	2.22	1.93
September .....	6,520	1,345	2,637	156,912	0.77	0.69
October .....	36,420	1,115	7,280	447,633	2.19	1.90
November .....	16,180	2,035	6,049	359,940	1.76	1.58
December .....	11,120	3,530	5,741	353,003	1.73	1.50
1899.						
January .....	22,197	4,116	8,417	517,541	2.53	2.19
February .....	44,952	6,945	15,688	871,267	4.26	4.09
March .....	32,652	7,314	12,399	762,385	3.72	3.23
April .....	21,582	5,100	9,016	536,489	2.62	2.35
May .....	4,731	2,517	3,351	206,045	1.00	0.87
June .....	2,999	1,287	2,040	121,388	0.59	0.53
July .....	20,290	795	4,985	306,516	1.50	1.30
August .....	4,362	1,287	2,222	136,625	0.67	0.58
September .....	2,394	430	984	58,552	0.29	0.26
October .....	1,656	320	1,014	62,348	0.30	0.26
November .....	7,068	918	1,787	106,334	0.53	0.47
December .....	18,138	1,656	4,728	290,713	1.42	1.23
1900.						
January .....	10,335	2,125	3,728	229,226	1.12	0.97
February .....	48,305	2,237	12,950	719,206	3.50	3.37
March .....	17,650	5,723	10,208	627,665	3.07	2.66
April .....	18,775	4,712	9,016	536,489	2.62	2.35
May .....	6,736	2,462	3,718	228,611	1.12	0.97
June .....	27,831	2,575	8,317	494,896	2.42	2.17
July .....	11,572	2,462	5,405	332,340	1.63	1.41
August .....	6,960	1,675	2,814	173,026	0.84	0.73
September .....	28,447	1,337	4,975	296,033	1.45	1.30
October .....	13,262	1,337	3,787	232,854	1.14	0.99
November .....	11,460	2,462	4,224	251,345	1.23	1.10
December .....	18,775	2,800	6,475	398,132	1.95	1.69
The year .....	48,305	1,337	6,301	4,519,823	22.09	1.64

\*See explanation page 13.

*Estimated monthly discharge of Tallapoosa River near Milstead, Ala.*

[Drainage area, 3,840 square miles.]

Month.	Discharge in second-feet.			Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Depth in inches.	Second- feet per square mile.
1901.					
January .....	33,962	5,275	11,476	3.45	2.99
February .....	28,900	5,837	10,440	2.83	2.72
March .....	35,087	4,150	8,374	2.52	2.18
April .....	28,900	5,499	12,020	3.49	3.13
May .....	17,875	3,587	6,440	1.94	1.68
June .....	13,262	2,775	5,976	1.74	1.56
July .....	5,387	2,012	3,398	1.01	.88
August .....	23,275	2,012	5,904	1.78	1.54
September .....	9,887	1,675	3,137	.91	.82
October .....	6,849	1,562	2,364	.71	.62
November .....	2,237	1,562	1,855	.54	.48
December .....	*70,000	1,843	8,282	2.49	2.16
The year .....	*70,000	1,562	6,639	23.41	1.73

\*Approximate.

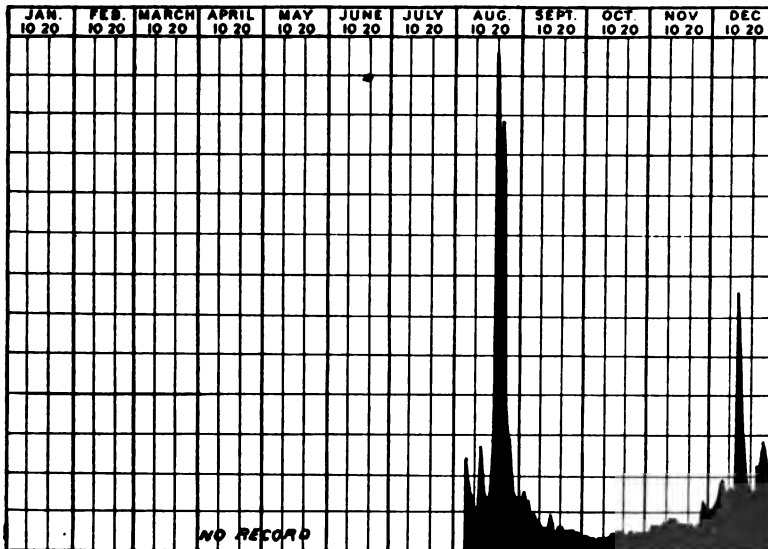


Fig. 1—Discharge of Tallapoosa River at Milstead, Ala., 1897.

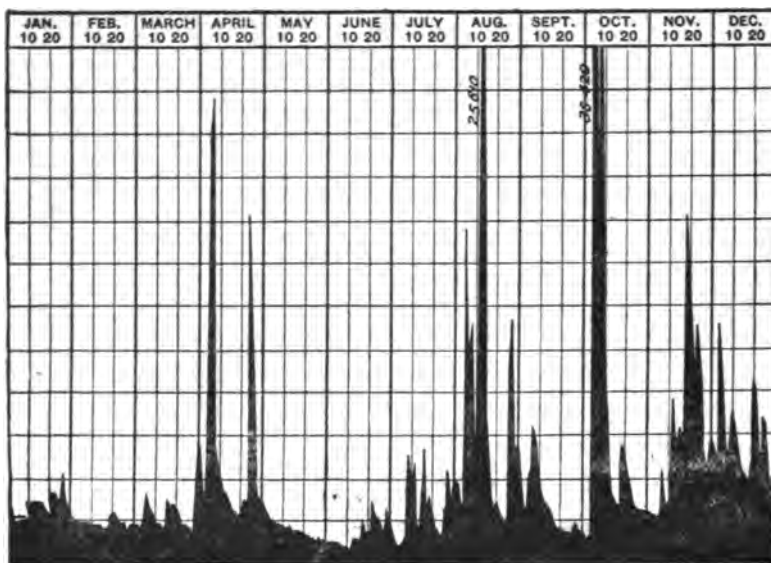


Fig. 2—Discharge of Tallapoosa River at Milstead, Ala., 1898.

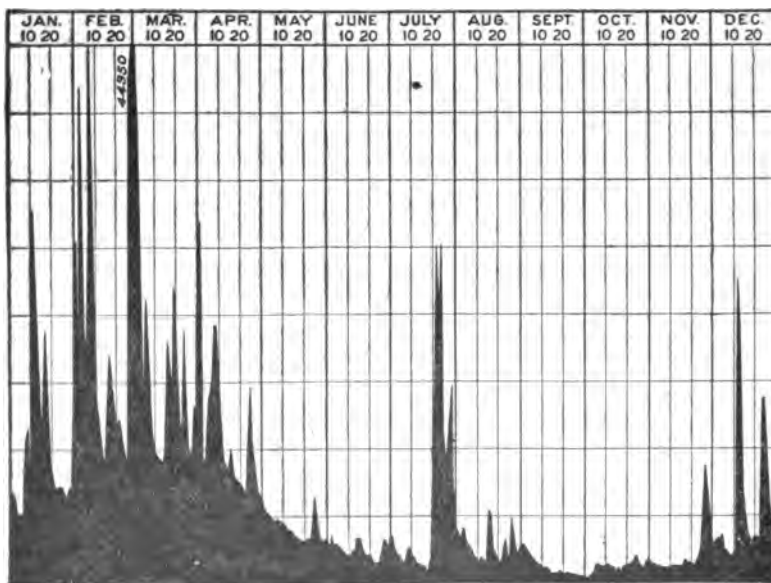


Fig. 3—Discharge of Tallapoosa River at Milstead, Ala., 1899.

The amount of water delivered from the drainage basin as measured at the points named below has been computed in terms of depth in inches. The normals given are the monthly averages for times during which measurements or computations were had. The figures for the yearly normal are the sums of these monthly averages.

*Depth of run-off in inches from the drainage basin of Tallapoosa River at Milstead, Ala.*

Day.	Jan.	Feb.	Mar.	Apr.	May.	Jun.	July.	Aug.	Sept.	Oct.	Nov.	Dec.	Annual.
Normal . . .	1.96	2.78	2.50	2.60	1.13	1.28	1.23	1.38	.86	1.09	1.02	1.90	19.70
1898 . . . . .	.72	.52	.69	1.67	.45	.38	.75	.77	.77	2.19	1.76	1.73	13.85
1899 . . . . .	2.53	4.26	3.72	1.00	.59	1.50	.67	.29	.30	.30	.53	1.42	19.43
1900 . . . . .	1.12	3.50	3.07	2.62	1.12	2.42	1.63	.84	1.45	1.14	1.53	1.95	13.08
1901 . . . . .	3.45	2.83	2.52	3.49	1.94	1.74	1.01	1.78	.91	.71	.54	2.49	13.41

*Minimum monthly discharge of Tallapoosa River at Milstead, Ala., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

	1899			1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January . . .	4.116	374	1	2.125	193	4	5.275	480	1
February . .	6.945	631	1	2.237	203	1	5.837	531	1
March . . . .	7.314	665	1	5.725	520	2	4.150	377	1
April . . . .	5.100	464	1	4.712	428	1	5.500	500	1
May . . . . .	2.517	229	3	2.462	224	1	3.587	326	2
June . . . . .	1.287	117	4	2.575	234	1	2.575	234	2
July . . . . .	.795	72	1	2.462	224	2	2.012	183	1
August . . .	1.287	117	3	1.675	152	1	2.012	183	5
September .	.430	39	3	1.337	122	2	1.675	152	2
October . . .	.320	29	2	1.337	122	1	1.562	142	10
November . .	.918	83	5	2.462	244	9	1.562	142	3
December . .	1.656	151	1	2.800	255	3	1.843	168	1

NOTE—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "net H. P. per foot of fall" in this table for that month.

## 2. TALLAPOOSA RIVER NEAR SUSANNA, ALABAMA.

This station was established July 27, 1900, by J. R. Hall. It is located at the mouth of Blue Creek, which is 10 feet above the east landing of McCarty's ferry, 13 miles southwest of Dadeville, and 3 miles from Susanna, the nearest postoffice. The rod is graduated to feet and tenths; it is 18 feet long, and is nailed vertically to a tree overhanging the water on the south side of the creek at the junction of the creek and the river. The gage is referred to a bench mark on a white hickory tree about 40 feet from the rod on the south bank of the creek, and is 376.67 feet above tide water. Discharge measurements are made from a boat held in place by a wire stretched across the river, upon which the distances from the initial point are tagged. The section is an exceptionally good one, depth and current being almost uniform the entire width of the stream. The observer is T. A. Walls, a farmer who lives 1 mile from the station. During 1900 and 1901 the following discharge measurements were made by James R. Hall:

## 1900:

July 27—Gage height, 1.80 feet; discharge, 2,309 second-feet.

August 9—Gage height, 1.55 feet; discharge, 1,900 second-feet.

September 28—Gage height, 1.50 feet; discharge, 1,809 second-feet.

November 24—Gage height, 2.40 feet; discharge, 3,629 second-feet.

## 1901:

July 9—Gage height, 2.80 feet; discharge, 5,628 second-feet.

Feb. 27—Gage height, 2.90 feet; discharge, 5,135 second-feet.

NOTE—The gage was washed away, and this station was discontinued on March 30th, 1901.

*Daily gage height, in feet, of Tallapoosa River, near Susanna, Ala., for 1900.*

Day.	July	Aug.	Sept.	Oct.	Nov.	Dec.	Day.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	.....	5.80	2.40	1.40	1.80	2.00	17.....	.....	1.95	8.40	2.35	1.75	2.60
2.....	.....	4.00	3.80	1.40	1.70	2.00	18.....	.....	1.95	4.80	2.30	1.75	2.60
3.....	.....	2.00	4.80	1.40	1.70	2.10	19.....	.....	1.80	3.00	2.20	1.80	2.40
4.....	.....	1.80	4.20	1.35	1.65	2.20	20.....	.....	1.75	2.50	2.10	1.85	4.50
5.....	.....	1.80	2.25	1.30	1.65	2.30	21.....	.....	1.70	1.80	1.90	1.85	5.30
6.....	.....	2.10	1.50	2.80	1.65	2.50	22.....	.....	1.70	1.80	1.70	1.90	4.50
7.....	.....	2.20	1.45	3.00	1.60	2.40	23.....	.....	1.90	1.80	3.90	2.40	4.00
8.....	.....	1.70	1.45	2.50	1.60	2.40	24.....	.....	2.00	1.70	6.00	2.40	4.00
9.....	.....	1.55	1.40	1.85	1.60	2.40	25.....	.....	2.05	1.60	5.00	3.00	3.70
10.....	.....	1.50	1.35	1.80	1.60	2.30	26.....	.....	2.50	1.50	4.30	4.90	3.20
11.....	.....	1.40	1.35	1.75	1.60	2.10	27.....	.....	1.80	2.15	1.50	4.10	4.20
12.....	.....	1.40	1.35	1.70	1.55	2.00	28.....	.....	1.90	2.00	1.50	2.30	3.90
13.....	.....	1.40	1.35	1.90	1.55	1.90	29.....	.....	1.80	1.90	1.45	2.20	3.00
14.....	.....	1.40	1.30	2.40	1.60	3.90	30.....	.....	4.00	1.80	1.45	1.90	2.80
15.....	.....	1.40	1.35	2.45	1.60	3.90	31.....	.....	.680	2.25	.....	1.85	.....
16.....	.....	1.90	11.70	2.40	1.60	2.80							2.90

*Daily gage height of Tallapoosa River at Susanna, Ala., for 1901.*

Day.	Jan.	Feb.	Mar.	Day.	Jan.	Feb.	Mar.	Day.	Jan.	Feb.	Mar.
1.....	6.0	3.40	2.70	12.....	13.5	4.00	3.10	23.....	3.0	2.80	2.70
2.....	6.0	3.45	2.60	13.....	11.5	3.50	2.90	24.....	3.1	2.90	3.30
3.....	5.1	3.30	2.65	14.....	8.0	3.04	2.60	25.....	3.4	3.00	3.40
4.....	4.5	11.50	2.70	15.....	6.1	3.30	2.50	26.....	3.1	3.00	3.60
5.....	3.9	9.50	2.65	16.....	4.5	3.20	2.40	27.....	3.2	2.90	7.40
6.....	3.5	6.50	2.60	17.....	5.0	3.10	2.40	28.....	3.1	2.80	6.90
7.....	3.2	4.40	2.50	18.....	4.5	3.10	2.40	29.....	3.0	.....	6.10
8.....	3.0	4.30	2.40	19.....	3.9	3.05	2.45	30.....	3.2	.....	4.10
9.....	2.9	4.80	2.50	20.....	3.4	3.00	3.00	31.....	3.3	.....	.....
10.....	2.8	4.90	2.70	21.....	3.2	2.90	3.40				
11.....	3.4	4.50	3.30	22.....	3.1	2.85	2.90				

*Rating table for Tallapoosa River, at Susanna, Ala., 1900 and 1901.*

Gage Height.	Discharge.	Gage Height.	Discharge.
1.0	.....	4.0	11,030
1.2	.....	4.2	11,930
1.4	1,680	4.4	12,830
1.6	1,960	4.6	13,730
1.8	2,320	4.8	14,630
2.0	2,740	5.0	15,530
2.2	3,230	5.5	17,780
2.4	3,850	6.0	20,030
2.6	4,730	6.5	22,280
2.8	5,630	7.0	24,530
3.0	6,530	8.0	29,030
3.2	7,430	9.0	33,530
3.4	8,330	10.0	38,030
3.6	9,230	11.0	42,530
3.8	10,130	11.7	45,680

Note—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Tallapoosa River, near Susanna, Alabama.*

[Drainage area, 2,610 square miles.]

Month	Discharge in second-feet.			Total in acre-feet.	Run-off.	
	Maxi-mum.	Mini-mum.	Mean.		Depth in inches.	Second feet per square mile.
1900.						
July 27 to 31.....	.....	.....	8,364	.....	.....	.....
August .....	19,130	1,680	3,258	200,327	1.44	1.25
September .....	45,680	1,570	6,083	361,964	2.60	2.33
October .....	20,030	1,570	4,776	293,665	2.11	1.83
November .....	15,080	1,885	3,676	218,737	1.57	1.41
December .....	19,130	2,520	6,288	386,634	2.78	2.41

*Estimated monthly discharge of Tallapoosa River, near Susanna, Alabama.*

[Drainage area, 2,610 square miles.]

Month.	Discharge in second-feet			Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Depth in inches.	Second- feet per square mile.
1901.					.
January .....	53,780	5,630	13,265	5.86	5.08
February .....	44,780	5,630	11,303	4.51	4.33
March .....	26,330	3,850	7,546	3.31	2.89

*Minimum monthly discharge of Tallapoosa River at Susanna, Ala., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

[Drainage area, 2,610 square miles.]

	1900				1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.		Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
July .....	2,320	211	2	January .....	5,630	512	1
August .....	1,680	153	5	February .....	5,630	512	2
September .....	1,570	143	1	March .....	3,850	350	4
October .....	1,570	143	1				
November .....	1,885	171	2				
December .....	2,520	229	1				

NOTE—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "net H. P. per foot of fall" in this table for that month.

3. TALLAPOOSA RIVER, NEAR STURDEVANT, ALABAMA.

This station was established July 19, 1900, by J. R. Hall. It is located at the bridge, Columbus & Western Division of the Central of Georgia Railroad, a fourth of a mile west of Sturdevant. The gage rod is 20 feet high, and is graduated to feet and tenths. It is in two sections, and is fastened verti-



cally, the shorter section to a post at the edge of the waer on the east bank, about 20 feet below the bridge, and the longer section to the first stone pier from the east bank. It is so set that when the water rises above the short section it is on the long section, and the readings are made as from one continuous rod. The initial point of sounding is the east end of the bridge. The section is broken by three piers and by some large rocks below the bridge. The gage is referred to a bench mark consisting of a nail in the southwest corner of pier No. 2, east side of the river, 455.70 feet above tide water, and 14.20 feet above the zero of the gage. The observer is B. F. Neighbors, farmer and postmaster at Sturdevant, who lives a fourth of a mile from the station. During 1900 the following discharge measurements were made by James R. Hall:

July 20—Gage height, 2.85 feet; discharge, 2,603 second-feet.

August 13—Gage height, 1.95 feet; discharge, 1,887 second-feet.

*Daily gage height, in feet, of Tallapoosa River near Sturdevant, Ala. for 1900.*

Day.	July	Aug	Sept	Oct	Nov	Dec	Day.	July	Aug	Sept	Oct	Nov	Dec
1.....		4.30	3.40	1.80	2.50	2.90	17.....		3.00	7.00	2.60	2.30	3.20
2.....		3.40	6.10	1.70	3.40	2.80	18.....		2.80	5.00	2.20	2.20	3.10
3.....		2.80	4.20	1.60	4.70	2.70	19.....		2.95	4.00	3.80	2.10	2.20
4.....		2.50	2.90	1.60	3.60	3.00	20.....		2.80	2.90	3.10	2.00	5.20
5.....		2.40	2.50	3.30	3.30	3.20	21.....		3.15	2.40	2.70	1.90	2.50
6.....		2.35	2.20	3.00	3.20	3.30	22.....		2.75	2.30	2.50	2.10	3.20
7.....		2.25	2.00	3.00	2.90	3.20	23.....		2.65	2.40	2.40	5.00	3.50
8.....		2.20	1.90	2.90	2.70	3.00	24.....		2.55	2.70	2.30	7.30	3.20
9.....		2.10	1.80	3.10	2.60	2.80	25.....		2.65	2.50	2.20	6.40	4.80
10.....		2.00	1.70	3.20	2.50	2.70	26.....		2.50	2.80	2.10	5.40	5.40
11.....		1.95	1.60	3.30	2.40	2.60	27.....		2.50	2.60	2.00	4.20	5.40
12.....		1.90	1.60	3.40	2.40	2.60	28.....		2.70	2.90	2.00	3.60	3.40
13.....		2.00	1.60	3.50	2.40	2.70	29.....		6.50	2.40	2.00	2.90	3.20
14.....		2.10	1.80	3.50	2.30	4.70	30.....		7.60	2.30	1.90	2.70	3.10
15.....		2.60	8.80	3.00	2.30	3.00	31.....		5.00	2.80		2.60	7.50
16.....		3.40	12.00	2.60	2.30	3.70							

During the year 1901 James R. Hall made one measurement, as follows:

March 8—Gage height, 3.40 feet; discharge, 3,774 second-feet.

During the year 1902 the following discharge measurements have been made at Sturdevant by W. E. Hall:

July 11—Gage height, 1.85 feet; discharge, 1,440 second-feet.

September 17—Gage height, 0.80 feet; discharge, 658 second-feet.

October 9—Gage height, 1.08 feet; discharge, 858 second-feet.

November 12—Gage height, 1.34 feet; discharge, 1,000 second-feet.

*Daily gage height of Tallapoosa River at Sturdevant, Ala., for 1901.*

Day.	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	7.0	4.6	3.8	7.4	3.8	5.5	4.0	3.1	3.2	2.4	1.6	1.7
2.....	6.8	4.5	3.8	6.8	3.7	5.5	3.5	2.8	2.9	4.5	1.6	1.7
3.....	6.0	5.8	3.7	8.3	3.7	5.3	3.2	2.5	2.7	6.0	1.6	2.1
4.....	5.4	12.9	3.7	7.4	3.6	5.2	3.0	2.3	2.6	3.4	1.6	2.2
5.....	4.9	9.4	3.6	5.7	3.5	4.8	2.9	2.3	2.4	2.7	1.7	2.3
6.....	5.2	7.7	3.6	5.0	3.5	4.6	2.8	2.2	2.3	2.3	1.8	2.2
7.....	5.1	6.5	3.5	4.7	3.4	5.6	4.3	2.1	2.2	2.1	1.8	2.1
8.....	4.0	5.4	3.4	4.4	3.4	4.9	4.0	2.4	2.1	2.0	1.8	2.1
9.....	4.0	5.7	3.4	4.2	3.3	3.8	3.4	2.2	2.0	2.0	1.8	2.2
10.....	4.1	5.4	3.9	4.0	3.3	3.6	3.0	2.0	2.0	2.0	1.8	2.2
11.....	6.0	5.2	4.3	3.9	3.3	3.4	2.6	2.2	2.0	1.9	1.7	2.3
12.....	14.1	5.1	4.0	3.9	3.2	3.3	2.3	2.4	2.0	1.9	1.7	2.2
13.....	11.0	4.8	3.8	4.1	3.2	3.2	2.2	2.3	2.0	2.1	1.7	2.1
14.....	9.2	4.6	3.6	6.0	3.4	4.9	2.1	2.2	4.0	2.1	1.8	3.0
15.....	6.1	4.4	3.4	6.4	3.3	4.4	2.1	2.0	3.4	2.0	1.8	7.8
16.....	5.3	4.3	3.3	5.3	4.2	4.1	2.0	5.3	2.8	2.0	1.8	5.9
17.....	5.7	4.2	3.3	4.7	3.1	3.8	2.2	7.2	3.6	1.9	1.8	4.1
18.....	5.3	4.2	3.2	4.4	3.0	3.6	3.8	5.9	7.0	1.9	1.8	3.5
19.....	4.0	4.1	3.2	8.5	3.0	3.3	3.6	5.3	5.2	1.8	2.0	3.0
20.....	4.4	4.1	3.6	9.0	3.3	3.1	3.2	6.6	4.3	1.8	2.2	2.8
21.....	4.3	4.0	4.3	6.7	6.1	3.0	3.0	5.8	3.4	1.7	2.2	2.6
22.....	4.2	4.0	3.9	5.2	7.6	2.1	2.8	5.6	3.0	1.7	2.3	2.5
23.....	4.1	4.0	3.7	4.8	7.0	2.8	2.4	11.7	2.4	1.7	2.4	2.5
24.....	4.2	4.0	4.5	4.6	6.5	2.7	2.2	8.3	2.3	1.7	2.2	3.0
25.....	4.3	4.0	4.3	4.4	5.3	2.7	2.1	5.2	2.2	1.7	2.1	2.8
26.....	4.2	4.0	5.4	4.3	6.8	2.6	2.1	4.0	2.1	1.7	2.1	2.7
27.....	4.1	3.9	8.7	4.2	4.5	2.6	2.1	3.3	2.0	1.6	2.0	2.6
28.....	4.3	3.9	8.2	4.1	4.4	2.5	2.1	4.4	2.0	1.6	1.9	2.8
29.....	4.5	.....	7.5	4.0	4.3	2.5	2.0	5.2	2.2	1.6	1.8	15.7
30.....	4.5	.....	5.0	3.9	4.2	5.2	2.1	4.3	2.3	1.6	1.8	17.2
31.....	4.6	.....	8.9	.....	4.0	.....	2.8	3.5	.....	1.6	.....	12.0

#### 4. SURVEY OF TALLAPOOSA RIVER IN ALABAMA.

This survey of a part of Tallapoosa River in Alabama was made in June and July, 1900, under supervision of B. M. Hall, resident Hydrographer, by Field Engineer James R. Hall, levelman and topographer.

The survey began at the Hydrographic Station on the Tallapoosa River, at Milstead, Ala., and ran up the river 64 miles to head of shoal above Griffin's Ferry. The elevations are sea-level elevations.

#### DESCRIPTION OF RIVER.

The entire river above Milstead runs on granite bed-rock, and has numerous bluffs along its banks, forming excellent sites for dams.

There are two large developed water powers on the river: The Tallassee Falls plant, and the Montgomery Power Company's plant, both of which are near the lower end of the survey. (*See Plates B and C, opposite.*)

LYLE R. MESSAGE KILLS ON TWINBOON RIVER

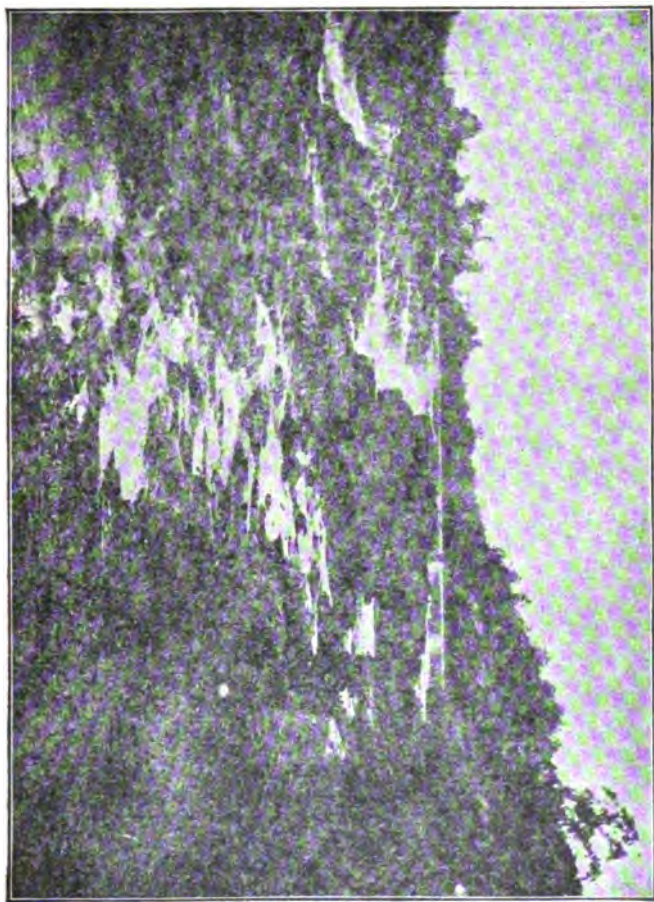




PLATE B. Tallassee Falls on Tallapoosa River.





Plate C. Power House and Dam, Montgomery Power Company, on Tallapoosa River, near Tallassee, Ala.

The Tallassee Falls dam and canal, which are six miles above Milstead, utilize a fall of 64 feet, with the whole river. This power and its large cotton manufacturing plant recently completed, is described in the Twentieth Annual Report, U. S. Geological Survey, Part IV, Pages 192-193. This power was capable of realizing 8,900 net H. P. without storage during low water of October, 1901. A break which occurred in the dam on December 29, 1901, has decreased the present available head, but does not stop the machinery.

The Montgomery Power Company dam has a 40-foot dam, nine and a half miles above Milstead. It backs the water six and a half miles up the river, and forms an immense storage basin. This being almost completed in December, 1901, was partly washed out by a great flood December 29, 1901. The water wheels, dynamos, pole line and wiring to Montgomery are all installed, and ready for work as soon as the dam is repaired. The distance to Montgomery is about 27 miles.

With river at stage of lowest water during October, 1901, this plant will develop at the wheels 5,572 net H. P. from the run of the river without drawing on the storage.

The equalizing storage of this dam will add fully 25 per cent. to this power and to the power at Tallassee for continuous running without materially lowering the head at either plant.

The following list of distances and elevations of water and bench marks shows the fall of the river from point to point. The total fall in 56 of the 64 miles surveyed is 364 feet.

*Elevations and bench marks along Tallapoosa River from Milstead,  
Ala., to Griffin Shoals.*

Distance from Milstead.	Description or location.	Bench mark elevation above sea level.	Elevation above sea level.
6.0	River surface of tailwater at Tallassee mills.....		206.3
6.2	Water above crest of Tallassee dam.....		209.9
8.6	Upper end of Tallassee Pond.....		209.9
9.5	River below Montgomery Power Company's dam.....		296.25
9.5	Crest of Montgomery Power Company's dam.....		335.25
15.7	Upper end of Montgomery Power Co.'s Pond.....		335.25
16.5	Water at Double Bridge Ferry .....		351.46
16.8	Water at mouth of Wind Creek .....		352.46
16.8	Bench mark No. 7, bunch of mulberry trees at the mouth of Wind Creek .....	357.86	
17.8	Bench mark No. 22, crooked willow on small branch at north end of Taylor's field.....	363.30	
17.8	Water at bench mark No. 22.....		356.18
18.5	Water opposite mouth of Kowaliga Creek.....		357.16
18.75	Bench mark No. 33, mulberry 100 feet above old Baker field .....	371.73	
18.75	Water at bench mark No. 33.....		359.75
19.4	Bench mark No. 42, willow at Garnetts Ford.....	364.60	
19.4	Water at Garnetts Ford .....		360.55
19.7	Bench mark No. 46, pine at mouth of High Falls Branch .....	373.98	
19.7	Water at "blue hole" at mouth of High Falls Branch .....		362.40
20.1	Water at "blue hole" at foot of Long Branch shoals.....		362.40
21.0	Bench mark No. 62, mulberry, 300 yards above mouth of Long Branch .....	382.45	
21.0	Water at bench mark No. 62, top of Long branch shoals .....		367.23
21.3	Bench mark No. 70, white hickory at McCarty's Ferry, mouth of Blue Creek .....	376.61	
21.3	Water at McCarty's Ferry, mouth of Blue Creek.....		367.80
23.0	Top of shoal opposite mouth of Peru Branch.....		372.55
23.8	Water at mouth of Gold Mine Branch .....		355.17
23.8	Bench mark No. 100, mulberry at mouth of Gold Mine Branch .....	386.00	
24.4	Bench mark No. 110, water oak at Robinson's Ferry.....	404.40	
24.4	Water at Robinson's Ferry .....		380.20
25.6	Water at top of Upper Robinson Shoals.....		389.10
25.6	Bench mark No. 124, small sycamore at mouth of small branch .....	395.10	
27.7	Water at mouth of small branch in Pace's field.....		390.90
28.7	Bench mark No. 140, water oak at foot of Hardy Shoals, in Pace's field .....	414.30	
29.5	Bench mark No. 150, dead stump 100 feet below the mouth of Big Sandy Creek.....	398.08	
29.5	Water at mouth of Big Sandy Creek.....		
30.0	Bench mark No. 165, big red oak at Young's Ferry.....	413.50	
30.0	Water at Young's Ferry .....		394.00
31.0	Water at Cherokee Bluff .....		394.60
31.2	Bench mark No. 175, big walnut 200 yards above Monowa Creek .....	416.75	
34.0	Bench mark No. 180, 16-inch pine tree at third bar of Seago Shoals .....	424.72	
34.0	Water at third bar of Seago Shoals, opposite bench mark No. 180 .....		379.92
35.8	Bench mark No. 190, large white oak at east land- ing at Walkers Ferry .....	436.90	
35.8	Water at Walkers Ferry .....		429.65
37.0	Bench mark No. 210, leaning white oak at mouth of small branch at upper end of Upshaw place.....	438.60	
37.4	Water at bench mark No. 210 .....		432.00
37.6	Water at top of fish trap .....		431.47
38.3	Bench mark No. 215, 16-inch white oak on small branch at upper end of Locke's old field.....	448.90	



Distance from Milled.	Description or location.	Bench mark elevation above sea level.	Elevation above sea level.
38.3	Water at bench mark No. 215.....		438.00
39.3	Water under Central railroad bridge at Sturdevant, Alabama.....		444.26
39.3	Bench mark on top of rail over first pier of the east end of Central Railroad bridge.....	505.90	
41.2	Bench mark No. 270, large water oak at east landing of Dennis Ferry.....	457.15	
41.2	Water at Dennis Ferry.....		445.85
42.2	Water at mouth of branch on left bank of river.....		448.20
45.3	Water 600 feet below mouth of Hillabee Creek.....		472.60
48.3	Bench mark No. 310, water oak at east landing of Welch's Ferry.....	504.15	
48.3	Water at Welch's Ferry.....		492.30
50.0	Bench mark No. 330, beech 150 feet above mouth of Freeman's Branch.....	526.62	
50.0	Water 150 feet above mouth of Freeman's Branch.....		521.04
52.0	Water at Whaleys Ferry.....		529.48
52.0	Bench mark No. 340, birch at Whaleys Ferry.....	539.38	
55.4	Bench mark No. 350, 10-inch birch at Millers Ferry.....	552.16	
55.4	Water at Millers Ferry.....		544.00
60.8	Water at Griffins Ferry.....		557.10
60.8	Bench mark No. 380, double ash tree on left bank at Griffins Ferry.....	564.76	
62.0	Bench mark No. 390, 12-inch birch at head of Griffins Shoals.....	573.87	
62.0	Water at head of Griffins Shoals.....		570.30

Surveys have been made for a large dam, 35 or 40 feet in height, at or near Double Bridge Ferry, to back the water beyond Robinson's Ferry, a distance of about 8 miles up the river. There is an excellent site for a dam, and the project is entirely feasible. The horse power in proportion to head would be the same as that available at the Montgomery Power Company's dam.

From the mouth of Big Sandy Creek to a point one mile above Griffin's Ferry, a distance of 32 miles, the fall of the Tallapoosa River is 176.5 feet. Nearly all of this fall can be utilized for power by developments similar to those which have been made, and proposed below. A study of the profile and of the above table of distances and elevations will give the distribution of the fall, showing the distance to which dams of certain height will back the water, at the various shoals, but the question of the best power sites, and the proper plan of development, height and location of dams, etc., for any point will depend on the special conditions favorable or unfavorable for dams and canals, the width of river bed, or flooded areas above, and the value of farming lands which

may be flooded. All of which can be determined only by special investigation and surveys. It will be safe, however, to assume that a practicable site for a dam 40 feet high or under, can be found in the vicinity of any location which may be selected, and the power obtainable can be estimated by using the volume of water, or its equivalent net horse power per foot of fall, and the proposed head to be developed.

The water supply or discharge of Tallapoosa River at different points may be closely approximated from the foregoing records of Milstead, Susanna, and Sturdevant Hydrographic Stations, and also at Dadeville, and Alexander City stations, on the tributaries.

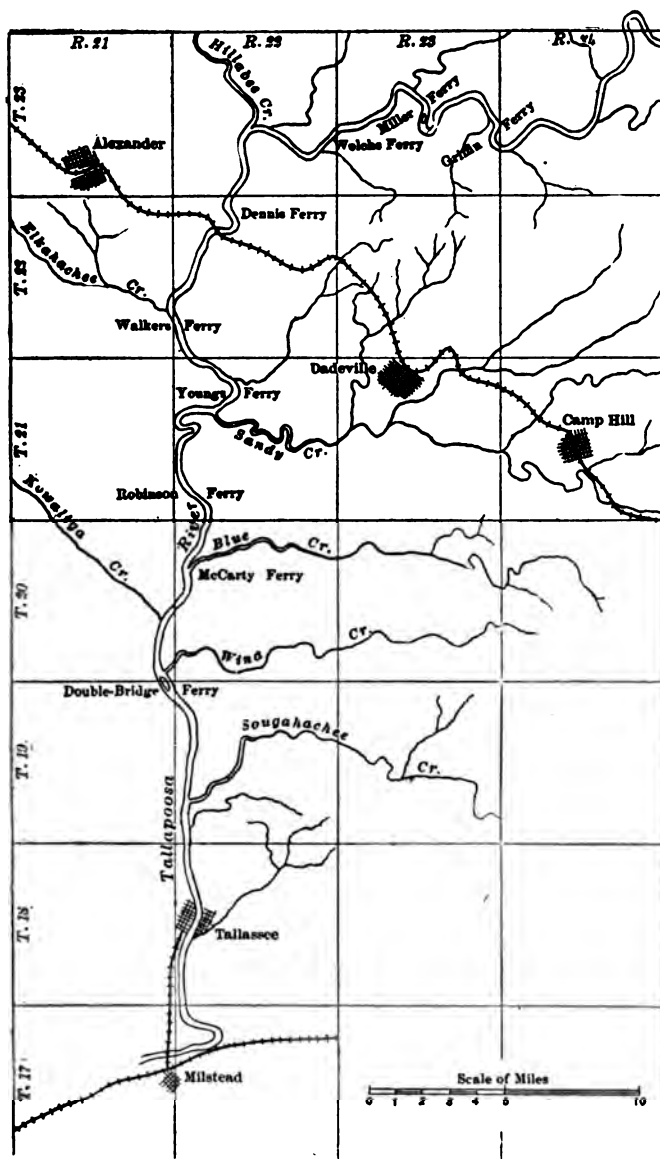


Fig. 4—Map of Tallapoosa River from top of Griffin Shoals, Ala., to Milstead, Ala.

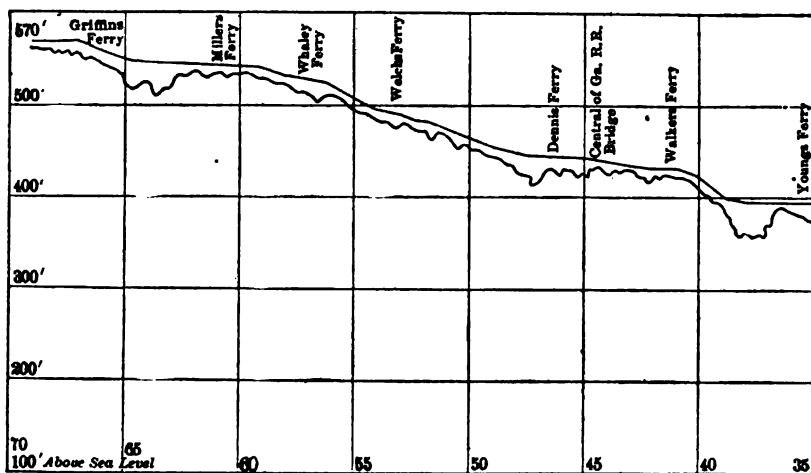


Fig. 5—Profile of Tallapoosa River from top of Griffin Shoals, Ala., to Milstead, Ala.

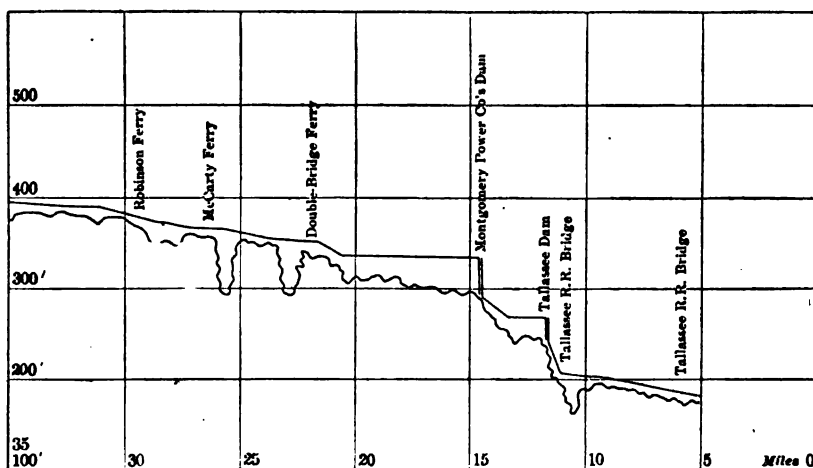


Fig. 6—Profile of Tallapoosa River from top of Griffin Shoals, Ala., to Milstead, Ala.—Continued.

## 5. BIG SANDY CREEK. NEAR DADEVILLE, ALABAMA.

This station, which was established by J. R. Hall, August 2, 1900, is located about  $4\frac{1}{2}$  miles southwest of Dadeville, at the highway bridge on the Dadeville-Susanna road. The gage, which is graduated to feet and tenths, is 16 feet high, and is fastened vertically to the first pier on the north side of the creek. The initial point of sounding is at the gage rod. The section is good for ordinary or flood measurements, but is rather wide and shoaly for low-water measurements. The latter can, however, be made a short distance from the gage. The observer is T. H. Finch, Dadeville, Alabama. During 1900 the following measurements were made by James R. Hall:

July 6—Gage height, 1.20 feet; discharge, 260 second-feet.  
 August 8—Gage height, 1.00 foot; discharge, 110 second feet.  
 August 8—Gage height, 1.00 foot; discharge, 116 second-feet.  
 August 25—Gage height, 1.35 feet; discharge, 281 second-feet.  
 Nov. 16—Gage height, 1.10 feet; discharge, 155 second-feet.  
 Dec. 31—Gage height, 2.00; discharge, 870 second-feet.

The measurements of August 8 and November 16 were made a half mile below Smith's bridge.

*Daily gage height, in feet, of Big Sandy Creek near Dadeville, Ala., for 1900.*

Day.	Aug.	Sept.	Oct.	Nov.	Dec.	Day.	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	1.00	1.10	0.95	1.00	1.10	17.....	1.80	3.90	1.10	1.05	1.45
2.....	1.20	1.10	.90	1.30	1.15	18.....	1.20	1.50	1.00	1.10	1.30
3.....	1.10	1.40	.90	2.00	1.25	19.....	1.10	1.10	1.05	1.10	2.40
4.....	1.10	1.30	.90	1.80	1.40	20.....	1.00	1.05	1.05	1.10	4.50
5.....	1.10	1.20	3.50	1.40	1.25	21.....	1.00	1.00	1.00	1.10	3.50
6.....	1.05	1.05	1.80	1.20	1.30	22.....	.90	1.00	1.50	1.30	1.70
7.....	1.05	1.00	1.25	1.20	1.25	23.....	.90	1.00	1.45	1.25	1.50
8.....	1.00	1.00	1.20	1.20	1.20	24.....	1.70	1.00	1.40	1.20	1.40
9.....	1.05	.95	1.10	1.15	1.15	25.....	1.40	1.00	1.20	1.50	1.40
10.....	1.00	2.00	1.15	1.15	1.10	26.....	1.60	1.00	1.15	1.90	1.35
11.....	1.00	1.80	1.10	1.15	1.10	27.....	1.15	1.05	1.10	1.80	1.35
12.....	9.05	1.40	1.20	1.15	1.10	28.....	1.10	1.05	1.05	1.20	1.35
13.....	9.00	1.20	1.30	1.10	1.10	29.....	1.00	1.00	1.05	1.15	1.50
14.....	9.00	1.20	1.15	1.10	2.20	30.....	1.00	.90	1.00	1.10	1.75
15.....	1.80	2.00	1.10	1.10	1.80	31.....	1.80	.....	1.05	.....	2.00
16.....	1.00	2.20	1.05	1.10	1.45						

*Daily gage height, in feet, of Big Sandy Creek near Dadeville, Ala.,  
for 1901.*

Day.	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	1.90	1.35	1.40	4.40	1.40	1.40	1.30	1.10	1.30	4.20	.85	.90
2.....	1.90	1.35	1.40	7.40	1.40	1.40	1.25	1.10	1.30	4.00	.85	.90
3.....	1.95	3.10	1.35	2.70	1.45	4.00	1.20	1.00	1.20	3.00	.85	1.40
4.....	1.70	6.00	1.35	2.00	1.35	5.60	1.20	1.05	1.10	3.50	.85	1.40
5.....	1.60	1.90	1.35	1.80	1.35	1.90	1.40	1.10	1.10	2.00	.85	1.40
6.....	1.50	1.75	1.40	1.60	1.35	3.50	1.50	1.05	1.10	1.80	.90	1.30
7.....	1.45	1.55	1.35	1.50	1.40	2.40	1.40	1.30	1.00	1.80	.90	1.10
8.....	1.40	2.10	1.35	1.50	1.35	1.90	1.25	1.20	1.00	1.50	1.00	1.00
9.....	1.40	3.50	1.40	1.50	1.35	1.50	1.20	1.10	1.00	1.40	1.00	1.10
10.....	1.40	2.20	1.45	1.50	1.30	1.60	1.20	1.10	.90	1.20	1.00	1.00
11.....	1.90	2.00	1.40	1.50	1.30	1.70	1.10	1.10	.90	1.20	1.00	1.00
12.....	1.70	1.70	1.35	1.55	1.35	2.00	1.10	1.20	.85	1.00	.90	1.00
13.....	2.50	1.60	1.35	1.65	1.35	1.80	1.10	1.15	.80	1.90	1.00	1.10
14.....	1.90	1.50	1.35	1.65	1.50	1.90	1.10	1.10	1.40	1.90	1.00	4.40
15.....	1.60	1.50	1.30	1.60	1.50	1.15	1.80	1.00	1.40	1.80	1.00	3.80
16.....	1.55	1.45	1.30	1.50	1.45	1.70	1.50	5.00	1.50	1.80	1.00	3.00
17.....	2.00	1.45	1.30	1.50	1.25	1.50	1.50	1.80	2.00	1.70	1.00	2.90
18.....	2.00	1.45	1.35	1.45	1.40	1.50	1.50	1.80	1.80	1.70	.90	2.50
19.....	1.60	1.50	1.35	6.00	1.40	1.45	1.30	1.40	1.80	1.70	.90	2.40
20.....	1.50	1.50	1.80	2.50	1.70	1.45	1.20	1.30	1.70	1.70	.80	2.40
21.....	1.50	1.45	1.60	2.40	7.00	1.40	1.15	1.20	1.40	1.60	.80	2.00
22.....	1.45	1.40	1.40	2.10	3.40	1.35	1.15	4.50	1.80	1.60	1.00	2.00
23.....	1.45	1.40	1.40	1.80	1.80	1.4	1.15	1.50	1.30	1.00	1.00	1.90
24.....	1.45	1.50	2.20	1.80	1.70	1.40	1.10	1.50	1.20	1.00	.85	1.80
25.....	1.50	1.50	2.10	1.70	1.50	1.35	1.10	1.40	1.20	.90	.85	1.50
26.....	1.55	1.45	1.70	1.60	2.70	1.30	1.10	1.40	1.10	.90	.80	3.00
27.....	1.45	1.45	1.70	1.45	1.80	1.30	1.15	1.20	1.00	*.70	.90	3.00
28.....	1.40	1.45	1.40	1.45	1.50	1.20	1.20	2.00	1.00	*.60	.90	21.00
29.....	1.40	.....	1.40	1.45	1.50	1.15	1.15	1.50	1.80	*.70	.90	16.00
30.....	1.40	.....	2.30	1.45	1.45	1.20	1.10	1.40	1.80	*.70	.90	8.00
31.....	1.40	.....	13.10	.....	1.40	.....	1.10	1.40	.....	.80	.....	4.00

\*Water was being held back by dams above in the morning when readings were made; 0.8 is assumed as minimum for October.

## WATER-POWERS OF ALABAMA.

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*Rating table for Big Sandy Creek at Dadeville, Ala., for 1900 and 1901.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
0.8	67	4.4	1,868	8.0	3,740	11.6	5,612
0.9	85	4.5	1,920	8.1	3,792	11.7	5,664
1.0	115	4.6	1,972	8.2	3,844	11.8	5,716
1.1	152	4.7	2,024	8.3	3,896	11.9	5,768
1.2	204	4.8	2,076	8.4	3,948	12.0	5,820
1.3	256	4.9	2,128	8.5	4,000	12.1	5,872
1.4	308	5.0	2,180	8.6	4,052	12.2	5,924
1.5	360	5.1	2,232	8.7	4,104	12.3	5,976
1.6	412	5.2	2,284	8.8	4,156	12.4	6,028
1.7	464	5.3	2,336	8.9	4,208	12.5	6,080
1.8	516	5.4	2,388	9.0	4,260	12.6	6,132
1.9	568	5.5	2,440	9.1	4,312	12.7	6,184
2.0	620	5.6	2,492	9.2	4,364	12.8	6,236
2.1	672	5.7	2,544	9.3	4,416	12.9	6,288
2.2	724	5.8	2,596	9.4	4,468	13.0	6,340
2.3	776	5.9	2,648	9.5	4,520	13.1	6,392
2.4	828	6.0	2,700	9.6	4,572	13.2	6,444
2.5	880	6.1	2,752	9.7	4,624	13.3	6,496
2.6	932	6.2	2,804	9.8	4,676	13.4	6,548
2.7	984	6.3	2,856	9.9	4,728	13.5	6,600
2.8	1,036	6.4	2,908	10.0	4,780	13.6	6,652
2.9	1,088	6.5	2,960	10.1	4,832	13.7	6,704
3.0	1,140	6.6	3,012	10.2	4,884	13.8	6,756
3.1	1,192	6.7	3,064	10.3	4,936	13.9	6,808
3.2	1,244	6.8	3,116	10.4	4,988	14.0	6,860
3.3	1,296	6.9	3,168	10.5	5,040	14.1	6,912
3.4	1,348	7.0	3,220	10.6	5,092	14.2	6,964
3.5	1,400	7.1	3,272	10.7	5,144	14.3	7,016
3.6	1,452	7.2	3,324	10.8	5,196	14.4	7,068
3.7	1,504	7.3	3,376	10.9	5,248	14.5	7,120
3.8	1,556	7.4	3,428	11.0	5,300	14.6	7,172
3.9	1,608	7.5	3,480	11.1	5,352	14.7	7,224
4.0	1,660	7.6	3,532	11.2	5,404	14.8	7,276
4.1	1,712	7.7	3,584	11.3	5,456	14.9	7,328
4.2	1,764	7.8	3,636	11.4	5,508	15.0	7,380
4.3	1,816	7.9	3,688	11.5	5,560		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Big Sandy Creek near Dadeville, Ala.*

[Drainage area, 196 square miles.]

Month.	Discharge in second-feet.			Total in acre-ft)	Run-off.	
	Maxi-mum.	Mini-mum.	Mean.		Depth in inches.	Second-ft. per sq. mile.
1900.						
August .....	655	80	207	12,728	1.22	1.06
September .....	3,150	80	355	21,124	2.03	1.82
October .....	2,670	80	264	16,233	1.56	1.35
November .....	870	110	261	15,531	1.50	1.34
December .....	3,870	150	560	34,433	3.31	2.87

*Estimated monthly discharge of Big Sandy Creek near Dadeville, Ala.*

[Drainage area, 196 square miles.]

Month.	Discharge in second-feet.			Total in acre-ft)	Run-off.	
	Maxi-mum.	Mini-mum.	Mean.		Depth in inches.	Second-ft. per sq. mile.
1901.						
January .....	880	308	425	2,511	2.51	2.18
February .....	2,700	282	545	7,920	2.90	2.78
March .....	6,392	256	552	19,185	3.26	2.83
April .....	3,428	334	689	10,644	3.94	3.53
May .....	3,220	230	480	9,660	2.84	2.46
June .....	2,492	178	523	7,896	2.99	2.68
July .....	516	152	227	1,388	1.34	1.16
August .....	2,180	115	369	9,054	2.18	1.89
September .....	620	67	257	2,585	1.47	1.32
October .....	1,764	*45	462	7,308	2.73	2.37
November .....	115	67	92	235	.52	.47
December .....	10,500	85	1,265	34,433	7.48	6.49
The year .....	10,500	*45	490	34,161	34.16	2.51

\*See foot note under gage heights for 1901.



*Minimum monthly discharge of Big Sandy Creek at Dadeville, Ala., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent of the theoretical power.*

[Drainage area, 186 square miles.]

	1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January .....				308	28	7
February .....				282	26	2
March .....				256	23	3
April .....				334	30	5
May .....				230	21	1
June .....				178	16	2
July .....				152	14	9
August .....	80	7	4	115	10	2
September .....	80	7	1	67	6	1
October .....	80	7	3	*45	*4	1
November .....	110	10	1	67	6	3
December .....	150	14	5	85	8	2

\*NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table for that month.

\*See foot note under gage heights for 1901.

A survey made in July, 1900, of Big Sandy Creek from its mouth to the new bridge near Dadeville, Ala., showed a total fall of 157 feet in a distance of 65,000 feet, or about 12 miles.

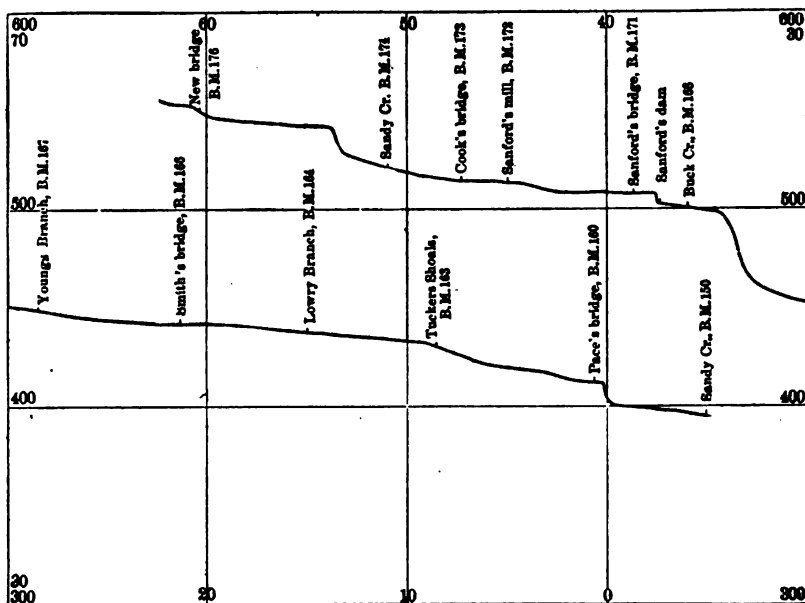


Fig. 7.—Profile of Big Sandy Creek from mouth to new bridge near Dadeville, Ala.

\*NOTE.—The numbers 0, 10..70, represent thousand feet stations.

The profile resulting from the survey is reproduced in Fig. 7. The following list of distances and elevations of water and bench marks shows the fall of the creek from point to point.

*Elevations and bench marks along Big Sandy Creek between its mouth and the new bridge near Dadeville, Ala.*

Distance above mouth.	Description or location.	Bench mark elevation above sea level.	Elevation above sea level.
0	Bench mark No. 150, dead stump at mouth of creek..	398.08	.....
0	Water at mouth of Big Sandy Creek.....		393.80
5,000	Water below Pace's dam .....		402.00
5,000	Water above Pace's dam .....		412.10
5,600	Bench mark No. 160, big pine on north side, 175 feet above Pace's bridge .....	422.30	.....
8,000	Creek surface .....		416.00
10,000	At point of Ivy Bend .....		419.00
11,700	Bench mark No. 162, large walnut at Tucker's house .....	503.85	.....
13,500	Bench mark No. 163, small oak at Tucker's fish trap .....	432.85	.....
13,500	Water above Tucker's fish trap .....		430.00
20,000	Bench mark No. 164, large sycamore at mouth of Lowry branch .....	445.20	.....
20,000	Water at mouth of Lowry branch.....		436.10
26,500	Bench mark No. 166, oak post at north end of Smith's bridge .....	463.95	.....
26,000	Water at Smith's bridge .....		441.70
26,500	Zero of U. S. G. S. gage at Dadeville .....	440.50	.....
33,500	Bench mark No. 167, wahoo tree at mouth of Young branch .....	559.58	.....
33,500	Water at mouth of Young branch .....		446.50
35,360	Water at Barnes basin .....		452.30
37,930	Water at foot of Black Shoals.....		465.00
39,300	Water at top of Black Shoals.....		496.30
41,100	Water at mouth of Buck Creek.....		497.30
41,100	Bench mark No. 168, small double oak at mouth of Buck Creek .....	503.65	.....
41,900	Eddy water below Sanford's dam .....		500.00
42,550	Bench mark No. 169, hickory at Sanfords' mill.....	522.10	.....
42,550	Floor of Sanford's mill .....	514.00	.....
43,770	Water at Sanford's bridge above dam .....		506.70
45,580	Water at head of Sanford Pond .....		506.70
50,000	Water at second shoal above Sanford Pond .....		512.50
52,340	Bench mark No. 173, large white oak near north end of Cook's bridge .....	539.35	.....
52,340	Water at Cook's bridge .....		513.80
56,120	Water opposite mouth of Chattasofka Creek.....		520.60
56,120	Bench mark, 16-inch water oak on west bank of Chattasofka Creek, 50 feet above mouth.....	527.20	.....
58,620	Water at top of old factory shoal .....		540.15
65,350	Water at new bridge .....		550.80
65,350	Bench mark on upstream end of sill on west end of new bridge .....	562.30	.....
65,350	Bench mark No. 176, 6-inch maple at new bridge....	563.00	.....

Water supply of this creek is shown by the foregoing records of Dadeville Hydrographic Station at Smith's Bridge.

The best shoal on this creek is the Sanford and Black Shoal, near Dadeville, which has a fall of 85.8 feet, in a distance of 5.2 miles. With a dam 54 feet high, and a canal 1,370 feet long, a practical working head of 80 feet can be developed, having 1 foot extra for grade of canal, and 4 feet extra for storage at top of dam. The foregoing record shows that from

August 1, 1900, to December 31, 1901, covering a record of 515 days, there were only 37 days in which the flow at Smith's Bridge was less than 115 cubic feet per second. It is, therefore, plain that during the last two years such a plant would have realized, for 90 per cent. of the time, not less than 800 net horse power continuously, 24 hours per day; and that by running 11 hours per day, 6 days per week, and storing the water during the time that the wheels are standing, there would have been 2,000 H. P. or more for use during factory hours, for 90 per cent. of the time during the last two years. By applying the rating table to the gage heights, and finding the discharge for each individual day, the exact power obtainable can be calculated, due allowance being made for the storage capacity, and equalizing effect of the dam.

Of course, this 85 foot fall can be developed in other ways. A low dam, and long canal can be used, or two separate powers can be developed.

#### 6. HILLABEE CREEK, NEAR ALEXANDER CITY, ALABAMA.

This station, which was established August 20, 1900, by J. R. Hall, is located  $6\frac{1}{2}$  miles northeast of Alexander City, on the road leading from that town to Newsite. The gage, which is graduated to feet and tenths, and is placed vertically, is in two sections, the short section, which reads from 0 to 5.50 feet, being fastened to a post in the edge of the water on the north bank 20 feet from the upstream side of the bridge, the long section, which reads from 5.50 feet to 16 feet, being fastened to the upstream end of the first pier on the north bank, and arranged so that when water rises above the short section the readings are made from the long one, both sections being easily read from the north approach to the bridge. The initial point of sounding is on the south side of the first pier on the north bank. The gage is referred to a bench mark at the top of a chord on the downstream side of the bridge at the second pier from the north bank, and is 27.6 feet above the zero of the gage. The bridge is in three spans, having a total length of 276 feet, with a north approach of 116 feet and a south approach of 124 feet, making a total, over all, of 516 feet. The observer is J. H. Chisholm, a farmer, postoffice address Alexander City, Ala. During 1900 the following measurements were made by James R. Hall:

August 29: Gage height, 1.40 feet; discharge 184 second-feet.  
November 28: Gage height, 2 feet; discharge, 390 second-feet.

*Daily gage height of Hillabee Creek at Alexander City, Ala., for 1900.*

Day.	Aug.	Sept.	Oct.	Nov.	Dec	Day.	Aug.	Sept.	Oct.	Nov.	Dec
1.....		2.30	1.30	1.60	1.90	17.....		2.60	1.30	1.50	2.70
2.....		2.30	1.10	2.60	1.90	18.....		2.20	1.30	1.40	2.80
3.....		1.60	1.10	6.80	1.80	19.....		1.60	1.20	1.50	3.00
4.....		1.40	1.10	3.20	1.90	20.....		1.50	1.20	1.60	2.90
5.....		1.30	3.20	2.20	1.80	21.....		1.50	1.20	1.70	6.00
6.....		1.20	2.00	1.80	1.90	22.....		1.40	1.90	2.10	4.00
7.....		1.20	2.80	1.70	1.80	23.....		1.20	6.90	1.90	3.00
8.....		1.10	2.60	1.70	1.70	24.....		1.30	2.90	1.90	2.90
9.....		1.10	2.40	1.80	1.70	25.....		1.40	2.10	5.10	2.90
10.....		1.20	2.30	1.70	1.70	26.....		1.30	1.90	2.90	2.80
11.....		1.10	2.30	1.70	1.70	27.....		1.40	1.80	2.40	2.60
12.....		1.10	1.80	1.70	1.70	28.....		1.30	1.70	2.00	2.50
13.....		1.10	1.60	1.70	1.80	29.....	1.40	1.40	1.60	1.90	2.50
14.....		1.10	1.40	1.70	3.80	30.....	1.30	1.30	1.50	1.90	5.80
15.....		8.10	1.40	1.70	2.90	31.....	1.80		1.50		5.70
16.....		5.00	1.20	1.60	2.80						

*Daily gage height of Hillabee Creek at Alexander City, Ala., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	5.00	2.40	2.30	5.20	2.50	2.50	2.50	1.80	2.60	1.20	1.00	1.00
2.....	4.90	2.40	2.30	4.40	2.50	2.40	2.50	1.40	2.00	1.20	1.00	1.00
3.....	4.90	4.60	2.40	4.80	2.50	3.00	2.40	1.40	1.40	1.10	1.00	2.00
4.....	4.70	9.40	2.40	3.10	2.40	2.50	2.30	1.20	1.20	1.10	1.00	1.60
5.....	3.90	6.00	2.30	2.90	2.40	2.50	2.00	1.10	1.10	1.10	1.30	1.40
6.....	3.60	4.10	2.20	2.80	2.40	2.60	1.80	1.10	1.10	1.20	1.10	1.00
7.....	3.60	3.60	2.10	2.70	2.40	2.50	2.90	1.10	1.10	1.40	1.10	1.20
8.....	3.40	3.10	2.10	2.60	2.40	2.50	2.10	1.00	1.10	1.20	1.00	1.20
9.....	2.60	3.60	2.10	2.60	2.30	2.40	1.80	1.20	1.10	1.20	1.00	1.20
10.....	2.50	3.00	2.40	2.50	2.30	2.40	1.80	1.20	1.10	1.20	1.00	1.90
11.....	8.00	2.90	2.50	2.50	2.30	2.30	1.70	1.10	1.00	1.60	1.00	1.50
12.....	7.60	2.90	2.30	2.40	2.20	2.00	1.60	1.10	1.00	2.00	1.00	1.40
13.....	7.00	2.80	2.20	3.10	2.50	2.10	1.50	1.20	1.20	2.90	1.00	1.40
14.....	5.90	3.00	2.20	2.90	2.30	2.00	1.40	1.60	2.50	1.60	1.00	2.00
15.....	4.50	3.10	2.30	2.80	2.50	2.00	1.30	2.10	2.40	1.40	1.00	3.00
16.....	4.30	2.60	2.40	2.70	2.20	1.90	1.40	4.40	2.40	1.20	1.00	3.00
17.....	4.00	2.60	2.20	2.60	2.20	2.20	1.30	2.90	2.00	1.00	1.00	2.80
18.....	3.50	2.40	2.20	2.60	2.10	1.90	8.40	2.00	1.80	1.00	1.00	2.00
19.....	3.40	2.60	2.10	10.00	2.10	1.80	2.20	2.00	1.60	1.00	1.30	2.60
20.....	3.00	2.70	2.10	3.20	2.90	1.80	1.80	4.00	1.40	1.00	1.30	2.00
21.....	2.90	2.60	3.00	3.10	3.80	1.70	2.00	4.10	1.20	1.00	1.30	1.60
22.....	2.90	2.40	2.40	3.00	2.90	1.70	1.90	3.40	1.40	1.00	1.30	1.60
23.....	2.80	2.60	2.30	2.90	2.50	1.60	1.80	3.10	1.20	1.00	1.30	1.50
24.....	2.60	2.60	2.80	2.80	2.50	1.60	1.80	2.90	1.10	1.00	1.30	1.60
25.....	2.50	2.50	2.70	2.70	2.40	1.70	1.70	2.20	1.10	1.00	1.20	1.60
26.....	2.60	2.40	3.50	2.60	3.90	1.70	1.70	2.20	1.20	1.00	1.20	1.70
27.....	2.60	2.40	3.00	2.50	2.80	1.80	1.40	2.10	2.10	1.00	1.10	1.70
28.....	2.50	2.40	2.70	2.50	2.60	1.80	1.60	3.80	1.60	1.00	1.10	1.80
29.....	2.40		2.60	2.50	2.50	1.70	1.40	3.60	1.40	1.00	1.10	11.00
30.....	2.40		2.70	2.50	2.30	2.60	1.40	3.40	1.20	1.00	1.10	4.90
31.....	2.40		5.20		2.90		2.00	3.00		1.00		3.90

*Rating table for Hillabee Creek at Alexander City, Ala., for years 1900 and 1901.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
<i>Feet.</i>	<i>Second Ft.</i>	<i>Feet.</i>	<i>Second Feet.</i>	<i>Feet.</i>	<i>Second Ft.</i>
1.0	138	2.4	566	3.8	1,182
1.1	146	2.5	610	3.9	1,226
1.2	156	2.6	564	4.0	1,270
1.3	169	2.7	698	4.1	1,314
1.4	184	2.8	742	4.2	1,358
1.5	204	2.9	786	4.3	1,402
1.6	230	3.0	830	4.4	1,446
1.7	263	3.1	874	4.5	1,490
1.8	303	3.2	918	4.6	1,534
1.9	346	3.3	962	4.7	1,578
2.0	390	3.4	1,006	4.8	1,622
2.1	434	3.5	1,050	4.9	1,666
2.2	478	3.6	1,094	5.0	1,710
2.3	522	3.7	1,138		

NOTE.—This table applied to the foregoing "Daily Gage Heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Hillabee Creek near Alexander City, Ala.*

[Drainage area, 214 square miles.]

Month.	Discharge in second-feet.			Run-off.	
	Maximum.	Minimum.	Mean.	Depth in inches.	Second-feet per square mile.
1900.					
September .....	3,074	146	370	1.93	1.73
October .....	2,106	146	387	2.09	1.81
November .....	2,502	184	471	2.45	2.20
December .....	2,150	263	716	3.86	3.35
1901.					
January .....	3,030	566	1,198	6.46	5.60
February .....	3,646	566	920	4.48	4.30
March .....	1,798	434	617	3.32	2.88
April .....	3,910	566	911	4.75	4.26
May .....	1,226	434	624	3.37	2.92
June .....	830	230	439	2.29	2.05
July .....	1,006	169	357	1.93	1.67
August .....	1,446	138	535	2.89	2.50
September .....	654	138	249	1.29	1.16
October .....	786	138	181	.98	.85
November .....	169	138	148	.77	.69
December .....	4,350	138	526	2.84	2.46
The year .....	4,350	138	559	35.37	2.61

*Minimum monthly discharge of Hillabee Creek at Alexander City, Ala., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent of the theoretical power.*

[Drainage area, 214 square miles.]

	1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January .....				566	51	3
February .....				566	51	7
March .....				434	40	5
April .....				566	51	1
May .....				434	40	2
June .....				230	21	2
July .....				169	15	2
August .....	169	15	1	138	13	1
September .....	146	13	6	138	13	2
October .....	146	13	3	138	13	15
November .....	184	17	1	138	13	15
December .....	263	24	5	138	13	2

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table for that month.



## 7. ALABAMA TRIBUTARIES OF THE TALLAPOOSA RIVER, FROM MILSTEAD UP.

From which side.	Name of Stream.	Point on stream.	Drainage area, sq. miles.	Estimated discharge cu. ft. per sec. low water 1900-1901.	Net H. P. per ft. of fall on 80 per cent. turbine.
Left..	Uphabee Creek.....	Mouth of Creek.....	450	45	4.1
Left..	Uphabee Creek.....	Chehaw, Ala.....	390	40	3.6
Left..	Soughatchee Creek.....	Mouth of Creek.....	240	48	4.3
Right	Cedar Creek.....	Mouth of Creek.....	55	14	1.3
Left..	Wind Creek.....	Mouth of Creek.....	28	8	0.7
Right	Kowaliga Creek.....	Mouth of Creek.....	185	40	3.6
Right	Kowaliga Creek.....	Kowaliga, Ala.....	115	35	3.2
Left..	Blue Creek.....	Mouth of Creek.....	60	20	1.8
Left..	Big Sandy Creek.....	Mouth of Creek.....	200	70	6.3
Left..	Big Sandy Creek.....	Smith's Bridge.....	195	67	6.1
Right	Elkahatchee Creek.....	Mouth of Creek.....	75	37	3.3
Right	Hillabee Creek.....	Mouth of Creek.....	220	141	12.8
Right	Hillabee Creek.....	Chisholme's Bridge.....	214	138	12.6
Right	Emuckfaw Creek.....	Mouth of Creek.....	78	46	4.2
Left..	Cohoasancosa Creek.....	Mouth of Creek.....	70	42	3.8
Left..	High Pine Creek.....	Mouth of Creek.....	82	49	4.4
Right	Hurricane Creek.....	Mouth of Creek.....	14	8	0.7
Left..	Corn House Creek.....	Mouth of Creek.....	72	43	3.9
Right	Crooked Creek.....	Mouth of Creek.....	95	57	5.2
Right	Fox Creek.....	Mouth of Creek.....	37	22	2.0
Left..	Little Tallapoosa River...	Mouth of River.....	590	354	32.2
Left..	Little Tallapoosa River...	Ala.-Ga. State Line..	311	186	16.9
		Above Little Tallapoosa River.....	767	460	41.8
Left..	Tallapoosa River.....	Mouth of Creek.....	49	29	2.6
Right	Ketchepedrakee Creek.....	Mouth of Creek.....	55	33	3.0
Right	Cane Creek.....	Mouth of Creek.....	36	21	1.9
Right	Muscadine Creek.....	Mouth of Creek.....	302	181	16.4
Right	Tallapoosa River.....	Ala.-Ga. State Line..			

NOTE.—To find the net horse power available at a shoal on one of the streams near a given point, for low water 1900-1901, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table.

All of these tributaries to the Tallapoosa River are in the Crystalline region, and are very precipitous streams, having fine shoals all along their courses.

No State or Government Surveys have ever been made to determine their profiles, and it is, therefore, impossible at present to make a detailed statement of the water powers. The tabulated statement given above shows the cubic feet per second flowing in the streams, at certain places during low season of ordinary years, like 1900 and 1901.

This flow at any point multiplied by the total practical fall in feet that can be brought upon a water wheel on the given stream at that point, and divided by 11 gives the net available horse power at that point, during low season of a year like 1900 or 1901.

The "Cubic feet per second" flowing at the given points and the corresponding "Drainage areas" can be used to get by pro-

portion the discharge at other points of same stream whose drainage areas are known.

Actual discharge measurements have been made on these streams at various points and at various stages of water, as is shown by the following list of Miscellaneous Discharge Measurements. As that date of these measurements are given, the stage of water as related to minimum for 1900-1901 can be approximated by noting the stage at regular stations on the same dates.

Miscellaneous discharge measurements made by James R. Hall on tributaries of Tallapoosa River.

1900.

August 2—Sougahatchee Creek, Meaders bridge; discharge 125 second-feet.

August 3—Blue Creek, Susanna, Ala., postoffice; 34 second-feet.

August 28—Elkahatchee Creek, Island Home postoffice; discharge, 184 second-feet.

August 30—Timber Cut Creek, Near Welches Ferry; discharge, 18 second-feet.

December 12—Chattasofka Creek, New Bridge, near Dadeville; discharge, 35 second-feet.

1901.

February 11—Wind Creek, Starr's bridge, near Meltons Mill postoffice; discharge, 66 second-feet.

February 11—Sougahatchee Creek, Lovelady's bridge, near Thadeus; discharge 453 second-feet.

February 13—Blue Creek, Farrows Mill, Susanna postoffice; discharge 117 second-feet.

February 13—Channahatchee Creek, Freeman's Mill, Channahatchee postoffice; discharge 80 second-feet.

February 27—Kowaliga Creek, Benson's bridge, Kowaliga postoffice; discharge 154 second-feet.

March 5—Emuckfaw Creek, Hamlett's Mill, Zana postoffice; discharge 113 second-feet.

March 11—Moore's Creek, near Dudleyville; discharge 29 second-feet.

March 12—Chattahaspa Creek, Scott's Mill, near Tiller Crossroads postoffice; discharge 203 second-feet.

March 12—Cohoasanocsa Creek, Leverett's Mill, near Milltown postoffice; discharge 122 second-feet.

March 12—High Pine Creek, Lille's Gin, Happy Land postoffice; discharge 89 second-feet.

March 12—Beaver Dam Creek, near Louina postoffice; discharge 30 second-feet.

March 13—Corn House Creek, Swann's Store, near Level Road postoffice; discharge 31 second-feet.

March 13—Wild Cat Creek, Murphy's Mill, near Gay postoffice; discharge, 32 second-feet.

March 13—Tallapoosa River, below mouth of Little Tallapoosa River, near Goldburg; discharge, 2,400 second-feet.

March 13—Crooked Creek, near Goldberg; discharge 183 second-feet.

March 13—Hurricane Creek, near Almond postoffice; discharge, 29 second-feet.

## CHAPTER III.

### COOSA RIVER AND TRIBUTARIES.

The Regular Stations that will be used in the following discussion are: Riverside, Ala., and Rome, Ga., on the Coosa River; and Nottingham, Ala., on Talladega Creek. Numerous miscellaneous discharge measurements at other points will also be used.

Under the heading of each station all the investigations made at the station are given, together with the facts deduced therefrom.

#### 1. RIVERSIDE STATION ON COOSA RIVER.

This station is at Riverside, Ala., in the Springville quadrangle of the United States Geological Survey map, in latitude  $33^{\circ} 37'$  and longitude  $85^{\circ} 12'$ , at the bridge of the Southern Railway, Georgia Pacific Division, across the Coosa River. The river here flows in a southerly direction, the railroad running from east to west. The town of Riverside is on the right or west bank of the river, and the railroad depot is about 1,000 feet west of the bridge, which is of iron and about 30 feet above low water. Beginning at the left bank, there are two spans of 154 feet each; then a drawbridge 220 feet, revolving on a large center pier; then a stationery span, 80 feet in length, to west or right bank abutment. There is no running water at low stages under the last-named span.

At low water the flowing river is 480 feet wide, including three piers, and is from 4 to 10 feet deep. Very little of the current is too slow to turn any meter. It is somewhat irregular, as there are shoals and some old cribs just above the bridge, but for all stages it is probably the best station that can be found on the river at a bridge and easy of access.

On September 8, 1896, a discharge measurement was made by B. M. Hall, and two bench marks were established. On September 22, 1896, another discharge measurement was made, a wire gage was put in, and Mr. J. W. Foster, sawyer at a large sawmill about 300 feet distant, on right bank of river, below the bridge, was employed as observer.

The initial point is top of left abutment at the edge toward the river, on the downstream side of the bridge, from which side soundings and meter measurements are made. The rod of wire gage is nailed to outside guard rail, downstream side, next to the last panel of stationary bridge before reaching the pier at end of draw span. The rod is 14 feet long and divided to feet and tenths. The bench mark is the top of capstone on the large circular center pier of turn span. It is 26.80 feet above datum of gage at downstream side of pier.

The drainage area is 6,850 square miles, and is mapped on atlas sheets Springville, Anniston, Gadsden, Fort Payne, Rome, Tallapoosa, Marietta, Cartersville, Suwanee, Ellijay, Dalton, Cleveland, Ringgold, and Stevenson of the United States Geological Survey.

The following discharge measurements were made during 1896 by B. M. Hall and others:

September 8: Gage height, 0.70 feet; discharge, 1,630 second-feet.

September 25: Gage height, 0.50 feet; discharge, 1,403 second-feet.

October 30: Gage height, 0.88 feet; discharge, 1,986 second-feet.

December 21: Gage height, 1.57 feet; discharge, 3,272 second-feet.

*Daily gage height in feet of Coosa river at Riverside, Ala., for 1896.*

Day.	Sept.	Oct.	Nov.	Dec.	Day.	Sept.	Oct.	Nov.	Dec.
1.....		0.60	1.10	1.30	17.....		0.55	4.70	2.10
2.....		1.75	1.40	2.10	18.....		.65	4.20	2.20
3.....		3.10	1.20	4.38	19.....		.80	3.20	2.00
4.....		2.75	1.10	3.80	20.....		.85	2.30	1.80
5.....		2.00	1.05	3.20	21.....		.75	1.50	1.70
6.....		1.50	1.10	2.50	22.....		.70	1.40	1.50
7.....		1.20	1.20	2.20	23.....		.60	1.35	1.45
8.....		.85	2.55	1.90	24.....		.55	1.30	1.40
9.....		.70	2.30	1.70	25.....		.60	1.25	1.35
10.....		.60	1.90	1.60	26.....		.70	1.20	1.30
11.....		.65	1.30	1.55	27.....	.45	.80	1.15	1.25
12.....		.60	1.60	1.55	28.....	.45	.85	1.15	1.20
13.....		.60	2.25	1.60	29.....	.45	.90	1.10	1.10
14.....		.60	2.70	1.60	30.....	.50	.95	1.20	1.10
15.....		.55	4.00	1.80	31.....		.85		1.10
16.....		.55	5.20	2.00					

*Rating table for Coosa River at Riverside, Ala., for 1896.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height	Discharge.
<i>Feet.</i>	<i>Second Ft.</i>	<i>Feet.</i>	<i>Second Ft.</i>	<i>Feet.</i>	<i>Second Ft.</i>
0.5	1,400	2.1	4,630	3.7	9,640
0.6	1,500	2.2	4,920	3.8	9,980
0.7	1,630	2.3	5,200	3.9	10,330
0.8	1,780	2.4	5,500	4.0	10,680
0.9	1,930	2.5	5,800	4.1	11,030
1.0	2,100	2.6	6,100	4.2	11,390
1.1	2,280	2.7	6,400	4.3	11,750
1.2	2,480	2.8	6,700	4.4	12,110
1.3	2,680	2.9	7,010	4.5	12,470
1.4	2,880	3.0	7,320	4.6	12,840
1.5	3,090	3.1	7,640	4.7	13,210
1.6	3,320	3.2	7,970	4.8	13,580
1.7	3,560	3.3	8,300	4.9	13,950
1.8	3,820	3.4	8,630	5.0	14,330
1.9	4,080	3.5	8,960	5.1	14,710
2.0	4,360	3.6	9,300	5.2	15,100

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following discharge measurements were made during 1897 by Max Hall and others:

March 31: Gage height, 4.53 feet; discharge, 12,515 second-feet.

June 17: Gage height, 1.54 feet; discharge, 3,747 second-feet.

July 21: Gage height, 5.55 feet; discharge, 16,925 second-feet.

August 20: Gage height, 2.58 feet; discharge, 6,174 second-feet.

November 29: Gage height, 0.80 feet; discharge, 1,854 second-feet.

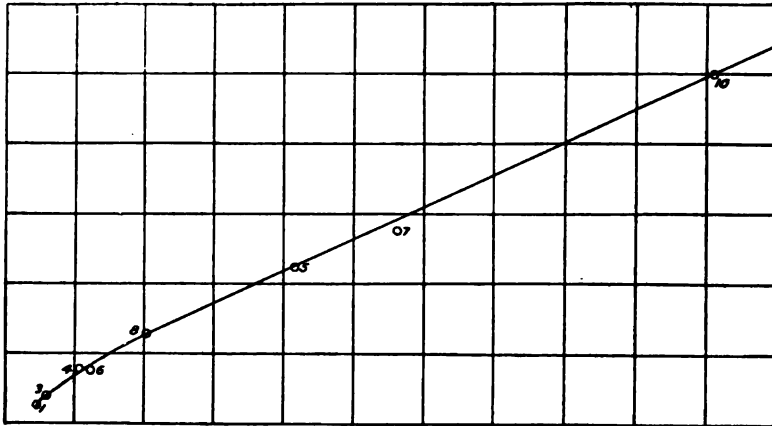


Fig. 8.—Rating curve for Riverside station on Coosa River, Ala.

*Daily gage height, in feet, of Coosa River, at Riverside, Ala.,  
for 1897.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	1.10	2.00	5.00	4.30	3.40	1.90	1.45	2.00	1.20	0.50	0.70	0.80
2.....	1.10	2.50	4.50	4.45	3.10	1.90	1.45	1.30	1.10	.50	.70	.90
3.....	1.10	5.35	4.25	5.20	3.05	1.90	1.40	1.60	1.30	.50	.70	.95
4.....	1.10	7.35	3.90	7.00	3.60	1.85	1.40	1.50	1.60	.50	.65	1.20
5.....	1.10	7.70	4.20	8.60	3.20	1.85	1.45	1.45	1.30	.45	.80	2.50
6.....	1.20	7.90	5.80	9.50	3.00	1.80	1.50	1.40	1.20	.45	1.05	3.00
7.....	1.25	9.00	11.40	10.50	3.80	1.90	1.50	1.50	1.10	.45	1.15	2.90
8.....	1.30	7.70	13.30	11.15	3.70	2.15	2.40	1.50	1.00	.45	1.10	2.40
9.....	1.35	6.40	12.55	12.15	2.65	2.10	2.30	1.60	.90	.45	1.00	2.15
10.....	1.35	5.90	12.65	11.90	2.60	1.90	2.05	1.70	.85	.45	.95	2.00
11.....	1.35	5.20	12.70	10.70	2.50	1.90	2.50	1.70	.80	.40	.85	1.70
12.....	1.30	7.35	12.80	9.10	2.55	2.00	2.70	2.00	.80	.40	.85	1.60
13.....	1.40	8.30	13.45	7.30	2.65	1.90	2.50	2.50	.75	.45	.85	1.60
14.....	2.00	8.20	14.80	6.05	2.10	1.85	2.00	2.30	.75	.45	.85	2.00
15.....	3.50	7.50	14.60	5.60	3.90	1.60	1.80	2.00	.80	1.45	.80	2.50
16.....	4.00	6.60	14.80	5.30	4.00	1.70	1.70	1.80	.85	1.65	.75	3.00
17.....	4.90	5.70	14.70	5.60	4.00	1.50	1.80	1.60	.85	1.40	.70	3.30
18.....	5.35	5.00	14.70	5.40	3.60	1.60	1.95	1.50	.80	1.35	.70	3.15
19.....	5.00	4.50	14.50	5.00	3.20	1.90	2.00	1.90	.80	1.20	.70	2.65
20.....	4.80	4.00	15.30	4.60	3.00	2.00	3.00	2.60	.75	1.00	.65	2.10
21.....	6.50	4.60	14.90	4.30	2.70	1.80	5.20	2.60	.75	.90	.65	2.20
22.....	7.00	4.65	14.70	4.00	2.35	1.70	6.40	1.70	.70	.85	.65	2.90
23.....	7.35	6.00	14.50	3.80	2.30	1.60	8.00	1.60	.70	.80	.65	4.20
24.....	7.00	7.90	13.70	3.60	2.25	1.55	6.20	1.70	.70	.70	.65	4.85
25.....	5.40	9.00	12.20	3.40	2.25	1.50	4.50	1.75	.65	.80	.65	4.95
26.....	4.70	9.60	10.60	3.30	2.15	1.45	4.00	1.60	.65	.75	.65	4.55
27.....	3.80	8.00	8.50	3.25	2.05	1.45	3.00	1.60	.60	.60	.65	3.85
28.....	3.00	6.20	6.50	2.20	2.00	1.40	2.60	1.50	.55	.65	.65	3.20
29.....	2.70	.....	5.30	3.10	2.00	1.45	2.50	1.40	.55	.80	.70	2.95
30.....	2.50	.....	4.90	3.20	1.95	1.45	3.00	1.35	.55	.75	.75	2.85
31.....	2.20	.....	4.60	.....	1.90	.....	2.60	1.30	.....	.70	.....	2.50

*Rating table for Coosa River at Riverside, Ala., for 1897.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second Ft.	Feet.	Second Ft.	Feet.	Second Ft.
0.4	1,350	2.0	4,520	5.0	14,046
0.5	1,400	2.2	5,100	6.0	17,306
0.6	1,500	2.4	5,700	7.0	20,566
0.7	1,650	2.6	6,300	8.0	23,826
0.8	1,820	2.8	6,910	9.0	27,086
0.9	2,010	3.0	7,530	10.0	30,346
1.0	2,210	3.2	8,178	11.0	33,606
1.2	2,630	3.4	8,830	12.0	36,866
1.4	3,070	3.6	9,482	13.0	40,126
1.6	3,540	3.8	10,134	14.0	43,386
1.8	4,020	4.0	10,786		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following discharge measurements were made during 1898 by Max Hall and others:

January 27: Gage height, 10.00 feet; discharge, 30,359 second-feet.

March 9: Gage height, 1.60 feet; discharge, 3,538 second-feet.

May 3: Gage height, 3.22 feet; discharge, 7,758 second-feet.

May 25: Gage height, 1.39 feet; discharge, 3,172 second-feet.

August 3: Gage height, 3.92 feet; discharge, 9,524 second-feet.

September 7: Gage height, 11.05 feet; discharge, 37,811 second-feet.

October 19: Gage height, 6.80 feet; discharge, 14,484 second-feet.

November 22: Gage height, 5.85 feet; discharge, 16,384 second-feet.

*Daily gage height, in feet, of Coosa River at Riverside, Ala.,  
for 1898.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	2.30	6.00	1.65	6.80	3.90	1.90	1.15	4.25	2.20	2.20	2.60	3.70
2.....	2.15	5.25	1.70	7.50	3.60	1.25	1.05	4.00	1.80	1.80	2.50	4.00
3.....	2.10	4.00	1.80	6.80	3.30	1.20	1.00	4.10	1.75	1.70	2.50	3.90
4.....	2.00	3.25	1.80	5.50	3.05	1.20	.95	4.00	5.80	2.00	2.45	3.70
5.....	1.90	3.00	1.80	5.80	2.90	1.30	.95	3.30	9.30	6.80	2.40	3.70
6.....	1.85	2.80	1.75	9.30	2.70	1.25	.90	4.00	10.20	11.20	2.40	3.90
7.....	1.75	2.75	1.70	10.50	2.55	1.20	.95	5.50	11.00	11.90	2.40	4.00
8.....	1.70	2.70	1.70	10.80	2.40	1.10	1.00	5.30	11.30	15.80	2.40	4.00
9.....	1.65	2.60	1.65	10.40	2.30	1.05	1.15	4.50	11.60	14.70	2.45	3.70
10.....	1.60	2.50	1.65	8.90	2.20	1.05	1.65	4.30	10.80	12.50	2.55	3.30
11.....	1.60	2.40	1.60	7.50	2.15	1.00	2.15	4.50	8.70	12.00	2.75	3.25
12.....	1.65	2.30	1.60	6.00	2.10	1.43	2.15	4.70	5.80	11.20	2.75	3.10
13.....	1.80	2.30	1.70	5.00	2.00	1.10	2.10	6.70	4.75	8.80	2.70	3.00
14.....	2.00	2.20	1.80	4.40	1.95	1.25	2.15	5.90	4.10	5.50	2.65	2.90
15.....	3.10	2.10	2.00	4.00	1.90	1.15	2.05	4.70	3.40	4.40	2.70	2.80
16.....	3.00	2.00	2.25	3.70	1.85	1.00	2.30	3.70	3.00	3.60	2.80	2.65
17.....	2.80	1.95	3.00	3.50	1.80	1.65	3.10	3.00	2.70	3.00	3.00	2.60
18.....	2.60	1.90	4.75	4.00	1.75	1.70	3.05	2.75	2.50	3.50	3.15	2.60
19.....	2.80	1.85	5.50	5.10	1.70	1.65	2.50	2.55	2.25	5.40	3.25	2.70
20.....	3.00	1.80	4.70	5.80	1.60	1.55	1.90	2.45	2.20	6.40	3.70	2.90
21.....	4.10	1.80	4.00	5.50	1.70	1.95	1.65	2.40	2.15	6.30	4.20	4.00
22.....	5.80	1.80	3.25	4.00	1.65	2.10	1.50	2.40	2.20	6.00	5.15	3.80
23.....	6.05	1.75	3.00	4.50	1.55	2.30	1.35	2.35	2.55	5.80	7.00	3.40
24.....	6.50	1.75	2.75	5.75	1.50	2.50	1.25	2.30	3.55	5.00	5.90	3.00
25.....	7.20	1.70	2.30	7.10	1.45	2.05	1.20	2.15	4.30	4.35	5.20	2.75
26.....	9.00	1.70	2.15	7.80	1.40	1.75	1.40	2.00	3.90	4.00	4.90	2.70
27.....	10.20	1.70	2.00	7.45	1.55	1.50	2.35	1.80	3.40	3.75	4.60	2.50
28.....	10.65	1.65	2.00	6.45	1.70	1.60	3.15	2.00	3.15	8.30	4.20	2.40
29.....	10.45	.....	2.30	5.50	1.60	1.45	3.10	2.50	3.00	8.00	4.00	2.30
30.....	9.45	.....	8.00	4.75	1.45	1.30	3.40	3.00	2.75	2.75	3.80	2.30
31.....	7.55	.....	4.50	.....	1.35	.....	4.00	2.60	.....	2.70	.....	2.40



*Rating table for Coosa River at Riverside, Ala., for 1898.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second-feet.	Feet.	Second-feet.	Feet.	Second-feet.	Feet.	Second-feet.
0.9	2,140	4.7	12,301	8.5	25,335	12.3	46,960
1.0	2,320	4.8	12,644	8.6	25,678	12.4	47,680
1.1	2,520	4.9	12,987	8.7	26,021	12.5	48,400
1.2	2,720	5.0	13,330	8.8	26,364	12.6	49,120
1.3	2,925	5.1	13,673	8.9	26,707	12.7	49,840
1.4	3,130	5.2	14,016	9.0	27,050	12.8	50,560
1.5	3,340	5.3	14,359	9.1	27,433	12.9	51,280
1.6	3,550	5.4	14,702	9.2	27,800	13.0	52,000
1.7	3,760	5.5	15,045	9.3	28,175	13.1	52,720
1.8	3,970	5.6	15,388	9.4	28,550	13.2	53,440
1.9	4,185	5.7	15,731	9.5	28,965	13.3	54,160
2.0	4,400	5.8	16,074	9.6	29,380	13.4	54,885
2.1	4,620	5.9	16,417	9.7	29,815	13.5	55,600
2.2	4,840	6.0	16,760	9.8	30,250	13.6	56,320
2.3	5,070	6.1	17,103	9.9	30,725	13.7	57,040
2.4	5,300	6.2	17,446	10.0	31,200	13.8	57,760
2.5	5,540	6.3	17,789	10.1	31,725	13.9	58,480
2.6	5,780	6.4	18,132	10.2	32,250	14.0	59,200
2.7	6,030	6.5	18,475	10.3	32,825	14.1	59,920
2.8	6,280	6.6	18,818	10.4	33,400	14.2	60,640
2.9	6,540	6.7	19,161	10.5	34,067	14.3	61,360
3.0	6,800	6.8	19,504	10.6	34,725	14.4	62,080
3.1	7,080	6.9	19,847	10.7	35,442	14.5	62,800
3.2	7,360	7.0	20,190	10.8	36,160	14.6	63,520
3.3	7,655	7.1	20,533	10.9	36,880	14.7	64,240
3.4	7,950	7.2	20,876	11.0	37,600	14.8	64,960
3.5	8,260	7.3	21,219	11.1	38,320	14.9	65,680
3.6	8,570	7.4	21,562	11.2	39,040	15.0	66,400
3.7	8,895	7.5	21,905	11.3	39,760	15.1	67,120
3.8	9,220	7.6	22,248	11.4	40,480	15.2	67,840
3.9	9,560	7.7	22,591	11.5	41,200	15.3	68,560
4.0	9,900	7.8	22,934	11.6	41,920	15.4	69,280
4.1	12,243	7.9	23,277	11.7	42,640	15.5	70,000
4.2	10,586	8.0	23,620	11.8	43,360	15.6	70,720
4.3	10,929	8.1	23,963	11.9	44,080	15.7	71,440
4.4	11,272	8.2	24,306	12.0	44,800	15.8	72,160
4.5	11,615	8.3	24,649	12.1	45,520	15.9	72,880
4.6	11,958	8.4	24,992	12.2	46,240	16.0	73,600

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.



The following discharge measurements were made during 1899 by Max Hall and others:

April 26: Gage height, 9.00 feet; discharge, 29,069 second-feet.

May 3: Gage height, 4.05 feet; discharge, 10,592 second-feet.

May 20: Gage height, 2.70 feet; discharge, 6,276 second-feet.

June 14: Gage height, 2.20 feet; discharge, 5,010 second-feet.

August 26: Gage height, 1.42 feet; discharge, 3,791 second-feet.

September 23: Gage height, 1.00 foot; discharge, 2,457 second-feet.

November 7: Gage height, 0.85 foot; discharge, 2,271 second-feet.

December 9: Gage height, 1.20 feet; discharge, 2,727 second-feet.

*Daily gage height, in feet, of Coosa River at Riverside, Ala., for 1899.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	2.80	5.90	12.10	10.30	5.00	2.70	1.70	3.95	2.30	0.90	0.95	2.60
2.....	2.75	6.30	12.20	10.20	4.50	2.80	1.70	3.06	2.40	.90	.95	2.00
3.....	2.75	7.50	12.30	10.00	4.20	2.80	1.65	2.30	2.80	.90	.95	1.60
4.....	2.80	7.40	12.10	9.80	3.95	2.60	1.60	2.00	2.40	.90	.95	1.50
5.....	2.80	9.10	10.10	8.90	3.80	2.30	1.50	1.90	2.10	.90	.95	1.45
6.....	3.00	12.10	9.00	8.75	3.75	2.10	1.50	1.75	2.00	.90	.95	1.40
7.....	3.50	14.10	8.00	9.00	3.60	2.00	1.45	1.65	1.85	.90	.95	1.35
8.....	4.30	14.30	7.50	10.00	4.00	2.00	1.40	1.50	1.40	.90	.95	1.30
9.....	4.20	14.30	7.25	12.30	3.85	1.95	1.70	1.50	1.30	.95	.95	1.30
10.....	4.40	14.10	7.00	12.00	3.60	1.85	1.60	1.55	1.25	1.20	.95	1.35
11.....	5.20	13.80	6.15	11.70	3.45	1.95	1.50	1.55	1.25	1.30	.95	1.40
12.....	5.90	13.00	5.20	10.00	3.30	1.95	1.50	1.60	2.20	1.40	.95	5.80
13.....	5.60	12.00	5.50	8.90	3.20	2.00	1.40	1.50	2.50	1.30	.95	8.25
14.....	5.00	10.90	7.50	7.90	3.00	2.15	1.40	1.45	2.65	1.20	1.00	8.00
15.....	4.70	8.70	8.30	6.30	3.00	2.80	1.30	1.50	2.00	1.20	1.00	6.00
16.....	4.90	7.90	16.00	5.55	2.95	3.20	1.30	1.50	1.40	1.20	1.00	4.50
17.....	5.00	7.60	17.40	5.25	2.95	2.95	1.30	1.50	1.25	1.10	1.00	3.75
18.....	4.90	7.80	17.00	5.10	2.80	2.50	1.20	1.55	1.20	1.00	1.00	3.60
19.....	4.70	8.10	16.50	5.00	2.80	2.25	1.30	1.50	1.20	.95	1.00	3.40
20.....	4.60	8.20	16.30	4.80	2.75	2.10	1.50	1.45	1.10	.95	1.00	3.00
21.....	4.20	8.00	16.35	4.60	2.75	1.80	1.50	1.45	1.00	.90	.95	2.85
22.....	4.00	7.65	16.20	4.30	2.70	1.70	1.90	1.35	.95	.90	.95	2.75
23.....	3.90	8.00	15.90	4.75	2.70	1.70	3.20	1.30	1.00	.95	.95	3.00
24.....	3.90	8.10	15.70	5.65	2.65	1.60	4.70	1.30	1.00	1.20	.95	5.40
25.....	4.00	7.30	15.50	8.90	2.60	1.75	3.60	1.30	1.00	1.15	1.00	6.40
26.....	4.25	7.00	14.90	9.00	2.50	1.60	3.20	1.60	1.00	1.00	2.15	7.10
27.....	4.15	8.30	13.25	8.90	2.45	1.60	3.00	1.20	.95	1.00	2.20	7.00
28.....	4.00	11.00	11.00	8.30	2.35	1.65	3.60	1.10	.95	.95	3.00	6.60
29.....	3.90	.....	8.00	6.90	2.30	1.70	4.20	1.50	.90	.95	3.00	6.00
30.....	3.75	.....	7.90	5.45	2.20	1.65	5.20	2.10	.90	.90	2.75	4.75
31.....	3.70	.....	8.50	.....	2.70	.....	4.75	2.10	.....	.95	.....	4.00

*Rating table for Coosa river at Riverside, Ala., for 1899.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second feet.	Feet.	Second feet.	Feet.	Second feet
0.9	2,330	5.2	14,740	9.5	30,650	13.8	46,560
1.0	2,460	5.3	15,110	9.6	31,020	13.9	46,930
1.1	2,600	5.4	15,480	9.7	31,390	14.0	47,300
1.2	2,760	5.5	15,850	9.8	31,760	14.1	47,670
1.3	2,920	5.6	16,220	9.9	32,130	14.2	48,040
1.4	3,100	5.7	16,590	10.0	32,500	14.3	48,410
1.5	3,300	5.8	16,960	10.1	32,870	14.4	48,780
1.6	3,500	5.9	17,330	10.2	33,240	14.5	49,150
1.7	3,720	6.0	17,700	10.3	33,610	14.6	49,520
1.8	3,940	6.1	18,070	10.4	33,980	14.7	49,890
1.9	4,160	6.2	18,440	10.5	34,350	14.8	50,260
2.0	4,400	6.3	18,810	10.6	34,720	14.9	50,630
2.1	4,600	6.4	19,180	10.7	35,090	15.0	51,000
2.2	4,900	6.5	19,550	10.8	35,460	15.1	51,370
2.3	5,160	6.6	19,920	10.9	35,830	15.2	51,740
2.4	5,430	6.7	20,290	11.0	36,200	15.3	52,110
2.5	5,700	6.8	20,660	11.1	36,570	15.4	52,480
2.6	5,970	6.9	21,030	11.2	36,940	15.5	52,850
2.7	6,250	7.0	21,400	11.3	37,310	15.6	53,220
2.8	6,530	7.1	21,770	11.4	37,680	15.7	53,590
2.9	6,810	7.2	22,140	11.5	38,050	15.8	53,960
3.0	7,100	7.3	22,510	11.6	38,420	15.9	54,330
3.1	7,400	7.4	22,880	11.7	38,790	16.0	54,700
3.2	7,700	7.5	23,250	11.8	39,160	16.1	55,070
3.3	8,010	7.6	23,620	11.9	39,530	16.2	55,440
3.4	8,330	7.7	23,990	12.0	39,900	16.3	55,810
3.5	8,650	7.8	24,360	12.1	40,270	16.4	56,280
3.6	8,970	7.9	24,730	12.2	40,640	16.5	56,650
3.7	9,290	8.0	25,100	12.3	41,010	16.6	57,020
3.8	9,620	8.1	25,470	12.4	41,380	16.7	57,390
3.9	9,950	8.2	25,840	12.5	41,750	16.8	57,760
4.0	10,300	8.3	26,210	12.6	42,120	16.9	58,130
4.1	10,670	8.4	26,580	12.7	42,490	17.0	58,400
4.2	11,040	8.5	26,950	12.8	42,860	17.1	58,770
4.3	11,410	8.6	27,320	12.9	43,230	17.2	59,140
4.4	11,780	8.7	27,690	13.0	43,600	17.3	59,510
4.5	12,150	8.8	28,060	13.1	43,970	17.4	59,880
4.6	12,520	8.9	28,430	13.2	44,340	17.5	60,250
4.7	12,890	9.0	28,800	13.3	44,710	17.6	60,620
4.8	13,260	9.1	29,170	13.4	45,080	17.7	60,990
4.9	13,630	9.2	29,540	13.5	45,450	17.8	61,360
5.0	14,000	9.3	29,910	13.6	45,720	17.9	61,730
5.1	14,370	9.4	30,280	13.7	46,190	18.0	62,100

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following discharge measurements were made during 1900 by Max Hall and others:

February 10: Gage height, 5.03 feet; discharge, 13,493 second-feet.

March 21: Gage height, 12.50 feet; discharge, 43,759 second-feet.

May 5: Gage height, 4.15 feet; discharge, 11,196 second-feet.

August 21: Gage height, 2.32 feet; discharge, 5,609 second-feet.

December 28: Gage height, 4.25 feet; discharge, 11,335 second-feet.

*Daily gage height, in feet, of Coosa River, near Riverside, Ala.  
for 1900.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	3.50	2.70	6.90	6.65	5.00	2.75	11.60	4.70	1.85	1.55	2.30	5.75
2.....	3.00	2.65	7.55	6.25	5.30	2.80	10.10	3.90	2.00	1.55	2.25	4.35
3.....	2.50	2.60	6.90	6.00	5.30	2.90	8.90	3.00	2.20	1.50	2.50	3.75
4.....	2.40	2.50	6.25	5.60	4.75	2.60	8.20	2.75	2.10	1.50	2.10	3.40
5.....	2.15	2.60	5.40	5.10	4.30	2.70	7.50	2.55	2.00	1.50	2.50	3.30
6.....	2.05	2.80	5.00	4.90	4.20	3.45	6.45	2.40	1.80	1.45	2.40	4.35
7.....	1.85	2.95	4.90	4.75	4.00	3.90	5.50	2.25	1.70	1.40	2.40	6.05
8.....	1.95	3.00	6.00	4.40	3.65	4.10	4.70	2.15	1.60	1.50	2.35	5.40
9.....	2.00	3.75	8.75	4.35	3.40	7.15	5.00	2.10	1.50	2.20	2.30	4.80
10.....	2.00	4.25	10.00	4.30	3.30	8.30	4.30	2.00	1.45	2.35	2.30	4.00
11.....	2.10	5.30	10.55	6.50	3.15	8.00	4.20	2.00	1.35	3.85	2.15	3.60
12.....	3.50	6.50	10.05	12.40	2.95	7.70	4.10	1.90	1.30	3.60	2.10	3.15
13.....	6.00	13.30	8.75	12.90	2.70	6.75	5.65	2.25	1.25	3.80	2.10	2.95
14.....	7.40	15.30	7.50	11.70	2.70	4.30	4.65	2.00	1.20	2.80	2.00	2.80
15.....	7.00	15.20	5.60	9.50	2.65	4.50	3.75	1.30	3.35	3.00	1.90	2.70
16.....	6.40	14.50	6.00	7.20	2.65	4.70	3.60	1.85	6.00	2.90	1.40	2.65
17.....	5.10	14.00	6.30	12.40	2.60	5.00	3.50	2.00	7.00	2.80	1.75	2.60
18.....	4.00	13.25	6.00	18.10	2.60	4.90	3.35	2.00	7.50	2.65	1.70	2.55
19.....	4.25	12.80	6.50	17.55	2.60	6.90	3.10	2.20	6.00	2.50	1.90	2.55
20.....	8.00	12.10	10.00	15.65	2.60	6.90	3.00	2.10	4.35	2.40	2.00	3.00
21.....	9.70	9.00	12.20	13.95	2.65	6.45	2.90	2.20	3.20	2.30	2.50	3.30
22.....	10.00	7.80	12.85	13.15	3.20	6.10	2.70	2.00	2.50	2.20	4.00	5.20
23.....	9.40	6.80	12.60	12.65	2.90	7.10	2.45	2.00	2.00	2.15	3.80	7.00
24.....	8.75	7.20	11.80	12.20	3.00	11.35	2.50	1.85	1.90	3.00	3.20	7.30
25.....	7.75	6.90	10.60	10.80	3.25	12.50	2.60	1.90	1.85	5.25	3.10	5.90
26.....	6.00	6.50	10.30	9.15	3.10	11.10	2.50	2.20	1.90	7.50	3.00	6.35
27.....	4.10	5.25	10.20	7.90	3.20	11.40	2.60	2.10	1.80	5.00	4.35	5.90
28.....	3.60	5.00	9.85	6.50	3.00	14.60	3.70	2.00	1.65	3.80	6.40	4.90
29.....	3.30	.....	9.50	5.70	2.80	13.70	5.70	1.95	1.60	3.00	9.20	4.30
30.....	3.00	.....	8.50	5.35	2.70	11.80	5.30	1.90	1.55	2.65	8.20	4.00
31.....	2.70	.....	7.20	.....	2.60	.....	5.90	1.90	.....	2.50	.....	6.50

The following discharge measurements were made during 1901 by Max Hall and others:

January 8: Gage height, 3.85 feet; discharge, 9,572 second-feet.

March 18: Gage height, 3.70 feet; discharge, 9,333 second-feet.

August 24: Gage height, 12.95 feet; discharge, 44,554 second-feet.

November 14: Gage height, 1.70 feet; discharge, 4,039 second-feet.

*Daily gage height, in feet, of Coosa River, at Riverside, Ala., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	7.30	6.00	3.55	14.35	4.80	5.20	3.00	2.15	5.60	2.70	1.90	1.95
2.....	7.35	6.50	3.50	14.40	4.60	8.20	2.90	2.10	5.00	4.00	1.85	1.90
3.....	6.50	9.00	3.50	13.30	4.30	8.25	2.85	2.10	4.50	3.20	1.85	1.95
4.....	6.00	10.60	3.50	11.90	4.15	7.20	2.80	2.05	4.30	3.00	1.85	1.95
5.....	5.50	11.00	3.40	10.40	4.00	5.75	2.80	2.00	3.65	2.90	1.80	2.20
6.....	5.00	12.00	3.30	10.30	3.95	6.00	2.75	2.20	3.30	2.75	1.80	2.10
7.....	4.25	11.90	3.20	9.00	3.90	4.80	2.75	4.10	3.05	2.60	1.80	2.00
8.....	3.95	11.50	3.10	7.25	3.75	4.30	2.70	4.80	3.00	2.50	1.85	2.00
9.....	3.70	9.90	3.10	6.50	3.60	4.20	3.50	4.25	2.90	2.30	1.85	2.10
10.....	3.60	9.60	3.40	5.60	3.45	6.20	4.00	3.50	2.85	2.30	1.90	2.10
11.....	6.50	9.90	5.50	5.00	3.25	6.00	3.50	2.90	2.75	2.25	1.80	2.05
12.....	14.10	9.60	6.50	4.80	3.15	5.00	2.90	2.75	2.70	2.30	1.80	2.00
13.....	15.70	8.90	7.50	4.80	3.10	4.25	2.50	2.60	2.65	3.00	1.80	2.00
14.....	15.40	7.60	7.00	6.50	3.10	3.75	2.40	3.20	2.65	2.80	1.75	2.90
15.....	15.10	6.50	6.30	7.50	3.00	3.75	2.35	3.20	2.60	2.70	1.75	11.50
16.....	14.90	5.80	5.25	8.30	2.95	3.80	2.20	3.40	2.55	2.50	1.80	12.00
17.....	14.30	5.45	4.25	7.80	2.85	3.80	3.15	5.30	3.90	2.45	1.80	12.50
18.....	13.90	5.10	3.75	6.50	2.80	4.90	2.90	9.60	5.60	2.60	1.80	12.40
19.....	13.30	4.80	3.55	10.50	2.90	5.00	2.90	9.65	6.30	2.40	1.80	11.00
20.....	11.30	4.60	3.55	14.00	2.85	4.50	3.20	10.00	7.50	2.35	1.75	10.20
21.....	8.30	1.25	3.75	14.50	4.00	3.90	3.00	11.50	7.20	2.30	1.75	9.20
22.....	6.25	4.20	3.70	14.30	8.20	3.80	2.95	11.00	6.00	2.30	1.75	8.00
23.....	5.30	4.00	3.50	13.50	9.90	3.50	2.90	11.50	4.50	2.25	1.75	4.50
24.....	4.60	3.90	4.30	12.50	10.75	3.25	2.75	12.50	3.30	2.20	1.85	3.90
25.....	5.30	3.85	13.90	11.30	11.85	3.15	2.70	12.90	3.00	2.10	1.90	4.10
26.....	5.80	3.65	15.90	9.90	11.90	3.00	2.60	12.40	2.70	2.10	2.10	4.60
27.....	6.30	3.65	15.20	7.90	11.80	2.90	2.40	12.00	2.60	2.10	2.00	5.10
28.....	6.40	3.60	14.70	6.80	11.00	3.00	2.20	11.50	2.50	2.05	2.00	6.50
29.....	5.85	.....	14.25	5.90	9.60	2.90	2.30	9.95	2.65	2.05	1.85	11.50
30.....	5.50	.....	14.60	5.20	6.20	3.10	2.40	8.35	2.60	2.00	1.85	15.60
31.....	5.80	.....	15.50	.....	4.60	.....	2.20	6.85	.....	1.95	.....	16.00

*Rating table for Coosa River at Riverside, Ala., for 1900 and 1901.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second-ft.	Feet.	Second-ft.	Feet.	Second-ft.	Feet.	Second-ft.
1.0	2,460	5.6	15,900	10.2	33,900	14.8	52,300
1.1	2,610	5.7	16,250	10.3	34,300	14.9	52,700
1.2	2,760	5.8	16,600	10.4	34,700	15.0	53,100
1.3	2,930	5.9	16,950	10.5	35,100	15.1	53,500
1.4	3,100	6.0	17,300	10.6	35,500	15.2	53,900
1.5	3,300	6.1	17,680	10.7	35,900	15.3	54,300
1.6	3,500	6.2	18,060	10.8	36,300	15.4	54,700
1.7	3,720	6.3	18,440	10.9	36,700	15.5	55,100
1.8	3,940	6.4	18,820	11.0	37,100	15.6	55,500
1.9	4,170	6.5	19,200	11.1	37,500	15.7	55,900
2.0	4,400	6.6	19,580	11.2	37,900	15.8	56,300
2.1	4,650	6.7	19,960	11.3	38,300	15.9	56,700
2.2	4,900	6.8	20,340	11.4	38,700	16.0	57,100
2.3	5,165	6.9	20,720	11.5	39,100	16.1	57,500
2.4	5,430	7.0	21,100	11.6	39,500	16.2	57,900
2.5	5,700	7.1	21,500	11.7	39,900	16.3	58,300
2.6	5,970	7.2	21,900	11.8	40,300	16.4	58,700
2.7	6,250	7.3	22,300	11.9	40,700	16.5	59,100
2.8	6,530	7.4	22,700	12.0	41,100	16.6	59,500
2.9	6,845	7.5	23,100	12.1	41,500	16.7	59,900
3.0	7,100	7.6	23,500	12.2	41,900	16.8	60,300
3.1	7,400	7.7	23,900	12.3	42,300	16.9	60,700
3.2	7,700	7.8	24,300	12.4	42,700	17.0	61,100
3.3	8,015	7.9	24,700	12.5	43,100	17.1	61,500
3.4	8,330	8.0	25,100	12.6	43,500	17.2	61,900
3.5	8,650	8.1	25,500	12.7	43,900	17.3	62,300
3.6	8,970	8.2	25,900	12.8	44,300	17.4	62,700
3.7	9,295	8.3	26,300	12.9	44,700	17.5	63,100
3.8	9,620	8.4	26,700	13.0	45,100	17.6	63,500
3.9	9,960	8.5	27,100	13.1	45,500	17.7	63,900
4.0	10,300	8.6	27,500	13.2	45,900	17.8	64,300
4.1	10,650	8.7	27,900	13.3	46,300	17.9	64,700
4.2	11,000	8.8	28,300	13.4	46,700	18.0	65,100
4.3	11,350	8.9	28,700	13.5	47,100	18.1	65,500
4.4	11,700	9.0	29,100	13.6	47,500	18.2	65,900
4.5	12,050	9.1	29,500	13.7	47,900	18.3	66,300
4.6	12,400	9.2	29,900	13.8	48,300	18.4	66,700
4.7	12,750	9.3	30,300	13.9	48,700	18.5	67,100
4.8	13,100	9.4	30,700	14.0	49,100	18.6	67,500
4.9	13,450	9.5	31,100	14.1	49,500	18.7	67,900
5.0	13,800	9.6	31,500	14.2	49,900	18.8	68,300
5.1	14,150	9.7	31,900	14.3	50,300	18.9	68,700
5.2	14,500	9.8	32,300	14.4	50,700	19.0	69,100
5.3	14,850	9.9	32,700	14.5	51,100		
5.4	15,200	10.0	33,100	14.6	51,500		
5.5	15,550	10.1	33,500	14.7	51,900		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Coosa River at Riverside, Ala.*

[Drainage area, 6,860 square miles.]

Month.	Discharge in second-feet.			Total in acre- feet.	Run-off.	
	Maxi- mum.	Mini- mum.	Mean.		Depth in inches.	Second- feet per square mile.
1896.						
September 27 to 30	1,400	1,350	1,363	10,812	0.03	0.20
October .....	7,640	1,450	2,218	136,380	0.37	0.32
November .....	15,100	2,190	4,637	275,921	0.75	0.68
December .....	12,110	2,280	4,125	253,636	0.69	0.60
1897.						
January .....	21,707	2,420	8,434	518,590	1.42	1.23
February .....	27,086	4,520	18,658	1,036,230	2.83	2.72
March .....	47,624	10,460	32,481	1,997,180	5.47	4.74
April .....	37,355	5,100	17,698	1,053,105	2.87	2.58
May .....	10,786	4,270	7,040	432,875	1.19	1.03
June .....	4,950	3,070	3,915	232,960	0.63	0.57
July .....	23,826	3,070	7,142	439,145	1.20	1.04
August .....	6,300	2,850	3,870	237,960	0.64	0.56
September .....	3,640	1,440	1,976	117,580	0.32	0.29
October .....	3,660	1,350	1,819	111,845	0.31	0.27
November .....	2,525	1,570	1,786	106,275	0.29	0.26
December .....	13,883	1,820	6,566	403,730	1.10	0.96
1898.						
January .....	35,084	3,550	11,572	711,539	1.95	1.69
February .....	16,760	3,655	5,763	320,161	0.87	0.84
March .....	15,045	3,550	5,852	359,828	0.68	0.59
April .....	36,160	8,260	18,133	1,078,986	2.95	2.65
May .....	9,560	3,028	4,684	288,010	0.78	0.68
June .....	5,540	2,320	3,281	195,233	0.54	0.48
July .....	9,900	2,140	4,289	263,722	0.72	0.63
August .....	19,161	3,970	8,758	538,512	1.48	1.28
September .....	41,920	3,865	13,927	828,712	2.26	2.03
October .....	72,160	3,760	19,936	1,225,825	3.36	2.91
November .....	20,190	5,300	8,375	498,345	1.36	1.22
December .....	9,900	5,070	7,376	453,535	1.25	1.08
1899.						
January .....	17,330	6,390	10,865	668,063	1.78	1.54
February .....	48,410	17,330	30,974	1,720,209	4.56	4.38
March .....	60,880	14,740	38,094	2,342,309	6.21	5.39
April .....	41,010	11,410	24,915	1,482,545	3.94	3.53
May .....	14,000	4,900	7,742	476,037	1.27	1.10
June .....	7,700	3,500	4,771	283,894	0.75	0.68
July .....	14,740	2,760	5,318	326,991	0.86	0.75
August .....	10,125	2,600	3,806	234,022	0.62	0.54
September .....	6,530	2,330	3,555	211,537	0.56	0.50
October .....	3,100	2,330	2,510	154,334	0.41	0.36
November .....	7,100	2,395	3,086	183,630	0.49	0.44
December .....	26,025	2,920	10,631	653,675	1.73	1.50

*Estimated monthly discharge of Coosa River near Riverside, Ala.*

[Drainage area, 7,065 square miles.]

Month	Discharge in second-feet.				Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Total in acre- feet.	Depth in inches.	Second feet per square mile.
1900.						
January . . . . .	33,100	4,280	13,344	820,491	2.18	1.89
February . . . . .	54,300	5,700	23,487	1,304,402	3.45	3.32
March . . . . .	44,500	13,450	26,822	1,649,221	4.38	3.80
April . . . . .	65,500	11,350	29,813	1,773,997	4.71	4.22
May . . . . .	14,850	5,970	8,198	504,075	1.34	1.16
June . . . . .	51,500	5,970	22,216	1,321,944	3.51	3.14
July . . . . .	39,500	5,565	13,610	836,846	2.23	1.93
August . . . . .	12,750	4,050	5,147	316,477	0.84	0.73
September . . . . .	23,100	2,760	6,483	385,765	1.03	0.92
October . . . . .	23,100	3,100	6,910	424,879	1.13	0.98
November . . . . .	29,900	3,720	7,673	456,575	1.22	1.09
December . . . . .	22,300	5,835	11,773	723,894	1.93	1.67
The year . . . . .	65,500	2,760	14,623	10518566	27.95	2.07

Month.	Discharge in second-feet			Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Depth in inches.	Second- feet per square mile.
1901.					
January . . . . .	55,900	8,970	26,089	4.25	3.69
February . . . . .	41,100	8,970	21,784	3.21	3.08
March . . . . .	56,700	7,400	20,613	3.37	2.92
April . . . . .	51,100	14,500	30,616	4.83	4.33
May . . . . .	40,700	6,670	16,195	2.64	2.29
June . . . . .	26,100	6,810	12,335	1.95	1.75
July . . . . .	10,300	4,900	6,535	1.07	0.93
August . . . . .	44,700	4,400	20,370	3.32	2.88
September . . . . .	23,100	5,700	9,977	1.57	1.41
October . . . . .	10,300	4,280	5,694	0.93	0.81
November . . . . .	4,650	3,830	4,016	0.64	0.57
December . . . . .	57,100	4,050	18,885	3.08	2.67
The year . . . . .	57,100	3,830	16,092	30.86	2.28



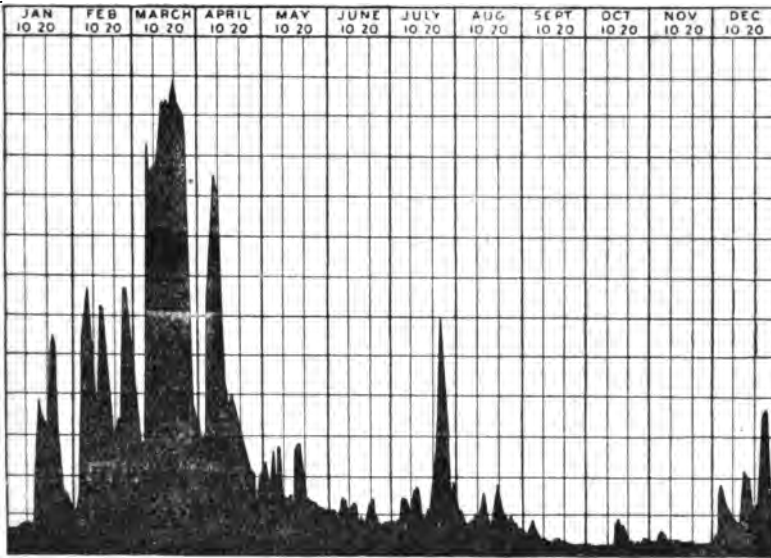


Fig. 9.—Discharge of Coosa River at Riverside, Ala., 1897.

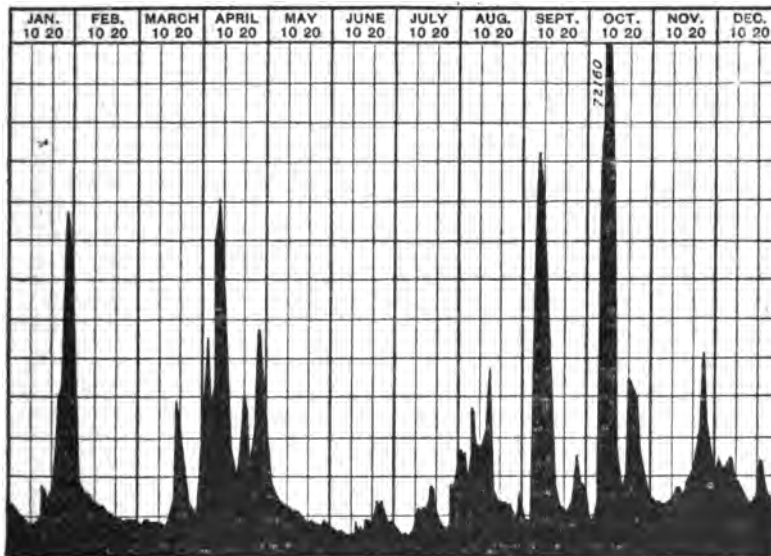


Fig. 10.—Discharge of Coosa River at Riverside, Ala., 1898.

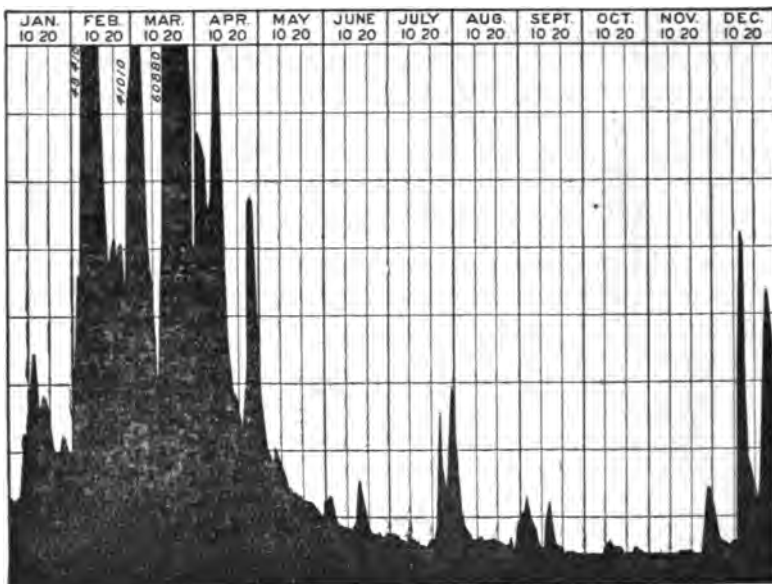


Fig. 11.—Discharge of Coosa River at Riverside, Ala., 1899.

*Minimum monthly discharge of Coosa River at Riverside, Ala., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent of theoretical power.*

[Drainage area, 6,850 square miles.]

	1899			1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days dura- tion of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days dura- tion of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days dura- tion of minimum.
January ..	6,390	581	2	4,285	390	2	8,970	815	1
February ..	17,330	1,575	1	5,700	518	1	8,970	815	1
March ...	14,740	1,340	1	13,450	1,223	1	7,400	673	2
April .....	11,410	1,037	1	11,350	1,032	1	14,500	1,310	2
May .....	4,900	445	1	5,970	543	5	6,687	608	1
June .....	3,500	318	3	5,970	543	1	6,810	619	2
July .....	2,760	251	1	5,565	506	1	4,900	445	3
August ....	2,600	236	1	4,050	369	1	4,400	400	1
September ..	2,330	212	2	2,760	251	1	5,700	518	1
October ...	2,330	212	11	3,100	282	1	4,285	390	1
November ..	2,395	218	17	3,720	338	1	3,830	348	6
December ..	2,920	265	2	5,835	530	2	4,050	368	1

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table; that month.

## 2. COOSA RIVER AT ROME, GEORGIA.

Coosa River is formed by the junction of Etowah and Oostanaula rivers at Rome, Ga. The drainage area is 4,006 square miles. Both of the tributary rivers rise in the northern part of Georgia and flow for the most part through a hilly, broken country, well wooded, about one-fourth of the land being under cultivation. The Coosa River flows in a southwesterly direction into Alabama and joins the Tallapoosa 6 miles above Montgomery, Ala., to form Alabama River. Measurements of flow are made at Rome and at Riverside, 120 miles farther downstream. The measurements at Rome are made on the Oostanaula and Etowah just above their junction. Etowah River is measured at Second avenue bridge and the Oostanaula at Fifth avenue bridge in Rome, and the result added to give the flow of Coosa River. The gage height is taken from the United States Weather Bureau gage at Fifth avenue bridge, on the Oostanaula. There is practically no fall on Oostanaula River from Fifth avenue bridge to the junction, hence the gage is used as Coosa River gage and gives the fluctuations of Coosa River. This gage is a 4 by 6 inch timber, graduated to feet and tenths and fastened to the downstream left-hand corner of the first pier from the left bank. The zero of gage is 575.79 feet above sea level. The United States Weather Bureau has maintained the station here for many years. It is now maintained only as a half-year station, from November 1 to April 30, inclusive, but W. M. Towers, the river observer, kindly reads the gage and furnishes the Survey with monthly reports of the daily gage heights for the entire year without charge. Mr. Towers has kept the records for many years and has predicted floods with great precision. The channel of the Etowah is straight, current swift and unobstructed, but the Oostanaula is rather sluggish and somewhat obstructed by piers. The banks are high, but liable to overflow in times of high water.

The following discharge measurements were made during 1896-97-98 by Max Hall and others:

1896—

September 29: Gage height, 0.20 feet; discharge, 1,209 second-feet.

1897—

May 7: Gage height, 2.75 feet; discharge, 4,646 second-feet.

October 5: Gage height, 0.15 feet; discharge, 990 second-feet.

1898—

May 11: Gage height, 1.90 feet; discharge, 2,946 second-feet.

September 17: Gage height, 2.60 feet; discharge, 3,913 second-feet.

October 11: Gage height, 5.05 feet; discharge, 8,324 second-feet.

October 22: Gage height, 4.10 feet; discharge, 6,489 second-feet.

November 30: Gage height, 3.90 feet; discharge, 6,039 second-feet.

*Daily gage height, in feet, of Coosa River at Rome, Ga., for 1897...*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	1.0	2.5	3.3	7.1	4.1	1.8	1.7	0.8	1.0	0.0	0.5	1.1
2.....	1.0	9.7	3.2	7.5	4.0	2.3	1.9	.7	.5	0.0	.9	1.0
3.....	1.0	11.5	3.1	8.2	3.5	2.0	1.0	1.2	.3	0.0	1.0	1.2
4.....	1.0	9.6	3.3	9.4	3.3	3.0	0.9	1.0	.5	.1	1.0	2.3
5.....	1.0	8.2	3.5	14.8	3.0	2.4	2.0	.8	.4	.1	1.0	3.2
6.....	1.3	5.2	7.6	18.9	3.0	2.0	1.9	.8	.3	.1	.8	3.7
7.....	1.1	5.0	19.7	17.0	3.0	2.0	1.9	1.9	.3	.1	.8	3.2
8.....	1.1	4.3	18.9	14.7	2.8	2.0	3.0	2.0	.2	.1	.8	2.2
9.....	1.0	5.0	15.4	12.1	2.6	2.0	2.1	2.0	.1	.1	.8	1.9
10.....	1.0	4.4	13.5	9.6	2.6	1.9	1.9	1.6	.0	.1	.7	1.7
11.....	.9	4.5	12.0	7.2	2.6	1.9	2.5	2.4	.0	.1	.7	1.5
12.....	.9	7.4	11.5	6.2	3.0	1.9	2.8	1.8	.0	1.1	.7	1.4
13.....	.9	8.7	18.6	5.8	3.4	1.8	2.0	1.3	.0	1.6	.7	1.3
14.....	2.8	7.2	21.3	5.0	4.0	1.7	1.6	.8	.0	1.3	.6	2.2
15.....	6.2	5.5	23.8	6.0	5.0	1.7	1.3	.6	.0	1.0	.0	4.0
16.....	5.0	4.5	23.4	7.4	4.0	2.0	1.0	.6	.0	.8	.6	3.5
17.....	3.5	4.0	22.6	7.0	3.3	2.8	5.2	2.1	.0	.7	.6	2.5
18.....	3.9	3.7	21.4	5.0	2.8	2.3	4.2	3.2	.1	.6	.6	2.2
19.....	5.0	3.4	19.7	4.5	2.7	2.0	4.8	2.4	.2	.6	.6	1.8
20.....	3.5	3.0	18.9	4.0	2.6	1.8	8.8	1.4	.2	.6	.3	1.7
21.....	8.7	4.0	17.7	3.	2.5	1.6	12.8	1.3	.2	1.5	.6	3.2
22.....	9.5	3.9	15.3	3.7	2.4	1.5	7.3	1.5	.2	1.3	.5	4.1
23.....	5.7	5.6	13.7	3.5	2.4	1.3	4.4	1.5	.2	1.0	.5	5.8
24.....	4.0	11.7	12.9	3.5	2.4	1.4	3.9	1.5	.2	.8	.5	5.3
25.....	8.5	8.6	9.1	3.5	2.3	1.8	2.6	1.1	.3	.8	.5	3.7
26.....	3.0	6.7	5.0	3.5	2.2	1.2	2.6	.8	.3	.7	.5	2.8
27.....	2.5	4.7	5.2	3.4	2.1	1.2	3.8	.5	.4	.7	.5	3.0
28.....	2.5	3.5	4.8	3.4	2.0	1.0	3.0	.4	.4	.7	.9	2.8
29.....	2.5	.....	4.5	3.4	2.0	1.1	2.4	.4	.4	.6	1.1	2.3
30.....	2.3	.....	4.2	3.2	1.9	2.0	1.4	.4	.4	.5	1.1	2.0
31.....	4.2	.....	4.0	.....	1.9	...	1.2	.5	.....	.5	.....	2.0

*Daily gage height in feet, of Coosa River, at Rome, Ga., for 1898.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	1.8	3.6	1.2	9.0	2.8	1.4	1.2	4.8	2.0	2.0	2.2	4.2
2.....	1.8	3.1	1.2	6.1	2.6	1.4	1.0	4.4	7.8	2.0	2.2	4.0
3.....	1.7	2.8	1.2	4.2	2.4	1.4	1.0	3.2	21.7	2.0	2.2	3.8
4.....	1.7	2.6	1.2	3.6	2.3	1.4	1.0	4.4	24.3	4.9	2.2	3.8
5.....	1.6	2.4	1.2	9.9	2.2	1.3	1.0	8.0	22.2	22.0	2.0	4.3
6.....	1.6	2.2	1.2	17.2	2.1	1.3	1.3	5.6	20.0	23.8	2.2	5.0
7.....	1.3	2.0	1.2	14.5	2.0	1.3	2.0	4.4	17.6	19.0	2.6	4.3
8.....	1.3	1.8	1.2	10.9	2.0	1.3	2.8	4.4	16.4	18.4	2.4	4.0
9.....	1.3	1.8	1.2	7.0	2.0	1.3	3.2	3.4	9.7	16.6	2.3	3.7
10.....	1.3	1.7	1.2	4.1	2.0	1.3	1.7	3.0	5.0	14.0	2.1	3.4
11.....	1.4	1.6	1.2	4.0	2.0	1.3	2.8	9.9	5.4	5.6	2.0	3.3
12.....	2.0	1.6	1.2	3.8	1.9	1.2	2.0	7.2	4.6	4.2	2.0	3.3
13.....	1.0	1.2	1.2	3.6	1.8	1.4	1.8	4.2	3.8	3.8	2.0	3.2
14.....	4.0	1.3	1.3	3.5	1.8	1.8	1.6	3.4	3.2	3.7	2.3	3.0
15.....	3.8	1.2	1.6	3.5	1.7	1.8	3.7	2.0	3.0	2.5	2.3	3.0
16.....	3.6	1.3	3.7	3.4	1.6	1.7	3.7	2.5	2.9	3.2	2.9	2.8
17.....	3.6	1.2	7.3	3.0	1.5	1.8	2.2	2.0	2.7	3.1	2.9	2.7
18.....	3.2	1.2	5.8	3.0	1.5	1.8	1.9	2.2	2.5	6.5	4.0	2.6
19.....	2.8	1.2	3.7	3.0	1.5	2.2	1.7	2.2	2.3	9.0	5.0	2.6
20.....	4.4	1.2	3.0	3.6	1.4	3.6	1.6	3.2	2.2	6.0	4.5	2.6
21.....	6.5	1.2	2.5	3.6	1.4	3.2	1.5	2.8	2.2	4.2	5.0	2.8
22.....	6.4	1.2	2.5	3.2	1.4	3.0	1.4	3.9	2.3	3.9	4.0	2.9
23.....	5.0	1.2	2.3	3.0	1.4	2.8	1.3	2.2	2.6	4.0	5.0	3.2
24.....	4.5	1.2	2.2	7.2	1.4	2.6	1.8	2.2	4.1	3.9	7.0	3.6
25.....	7.0	1.2	2.1	8.2	1.4	2.0	3.7	1.9	8.1	3.5	4.7	3.0
26.....	14.0	1.2	2.0	6.0	1.4	1.8	3.8	2.7	3.0	3.3	3.9	2.9
27.....	14.6	1.2	1.9	4.6	1.4	1.8	2.9	4.0	2.7	3.1	4.5	2.7
28.....	11.6	1.2	1.8	4.0	1.4	1.8	3.7	4.4	2.5	3.0	4.3	2.6
29.....	8.6	.....	2.0	3.7	1.4	1.6	4.2	3.4	2.8	2.8	4.3	2.5
30.....	4.6	.....	8.5	3.2	1.4	1.4	4.1	2.0	2.1	2.6	3.9	2.4
31.....	8.9	.....	11.4	.....	1.4	...	4.2	2.3	.....	2.4	.....	2.4

*Rating table for Coosa River at Rome, Ga., for 1897 and 1898.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
-0.15	990	2.5	3,760	5.1	8,445	7.7	13,515
0.0	1,070	2.6	3,910	5.2	8,640	7.8	13,710
0.1	1,140	2.7	4,060	5.3	8,835	7.9	13,905
0.2	1,210	2.8	4,220	5.4	9,030	8.0	14,100
0.3	1,280	2.9	4,380	5.5	9,225	8.1	14,295
0.4	1,360	3.0	4,540	5.6	9,420	8.2	14,490
0.5	1,440	3.1	4,700	5.7	9,615	8.3	14,685
0.6	1,520	3.2	4,860	5.8	9,810	8.4	14,880
0.7	1,610	3.3	5,020	5.9	10,005	8.5	15,075
0.8	1,700	3.4	5,180	6.0	10,200	8.6	15,270
0.9	1,800	3.5	5,340	6.1	10,395	8.7	15,465
1.0	1,900	3.6	5,520	6.2	10,590	8.8	15,660
1.1	2,000	3.7	5,715	6.3	10,785	8.9	15,855
1.2	2,110	3.8	5,910	6.4	10,980	9.0	16,050
1.3	2,220	3.9	6,105	6.5	11,175	10.0	18,000
1.4	2,330	4.0	6,300	6.6	11,370	11.0	19,950
1.5	2,450	4.1	6,495	6.7	11,565	12.0	21,900
1.6	2,570	4.2	6,690	6.8	11,760	13.0	23,850
1.7	2,690	4.3	6,885	6.9	11,955	14.0	25,800
1.8	2,810	4.4	7,080	7.0	12,150	15.0	27,750
1.9	2,930	4.5	7,275	7.1	12,345	16.0	29,700
2.0	3,060	4.6	7,470	7.2	12,540	17.0	31,650
2.1	3,190	4.7	7,665	7.3	12,735	18.0	33,600
2.2	3,320	4.8	7,860	7.4	12,930	20.0	37,500
2.3	3,460	4.9	8,055	7.5	13,125	22.0	41,400
2.4	3,610	5.0	8,250	7.6	13,320	24.0	45,300

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following discharge measurements were made during 1899 by Max Hall and others:

January 25—Gage height, 3.80 feet; discharge, 6,540 second-feet.

January 25—Gage height, 3.60 feet; discharge, 5,932 second-feet.

May 19—Gage height, 2.75 feet; discharge, 4,394 second-feet.

June 16—Gage height, 2.40 feet; discharge, 3,352 second-feet.

August 4—Gage height, 1.45 feet; discharge, 2,835 second-feet.

October 13—Gage height, 0.60 foot; discharge, 1,769 second-feet.

*Daily gage height, in feet, of Coosa River, at Rome, Georgia, for 1899.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	3.00	6.90	19.70	13.20	4.00	3.00	1.70	2.20	3.40	0.40	0.7	1.1
2.....	3.40	7.80	15.00	10.60	3.70	2.60	1.50	1.90	2.00	.30	.6	1.1
3.....	3.00	6.00	8.60	7.90	3.70	2.60	1.00	1.70	1.60	.30	.5	1.5
4.....	2.70	9.20	6.60	7.20	3.50	2.00	.90	1.50	1.40	.30	.5	1.3
5.....	2.60	15.30	7.80	9.50	3.50	2.00	2.00	1.40	1.30	.30	.4	1.1
6.....	2.60	18.20	9.00	8.20	3.50	2.00	1.90	1.50	1.30	.50	.4	1.0
7.....	3.60	27.80	8.00	8.20	3.70	2.00	1.90	1.60	1.20	.70	.3	.9
8.....	5.90	24.00	6.80	15.00	3.70	1.90	3.00	1.50	1.00	.70	.3	.8
9.....	5.90	22.40	5.70	13.40	3.60	1.80	2.10	1.80	1.00	.80	.3	.8
10.....	4.90	21.00	5.40	11.20	3.50	1.80	1.90	1.60	1.00	.60	.3	.8
11.....	4.00	19.00	5.20	9.50	3.30	1.80	2.50	1.40	2.90	1.00	.3	.8
12.....	4.50	16.50	4.90	7.00	3.10	2.20	2.80	1.40	2.30	.90	.3	2.8
13.....	4.00	7.00	4.50	6.40	3.10	3.80	2.00	1.20	1.50	.70	.3	6.1
14.....	3.80	5.00	6.00	5.90	3.00	4.00	1.60	1.10	1.00	.70	.4	5.0
15.....	3.60	5.00	16.60	5.60	3.00	3.50	1.30	1.30	.90	.60	.4	3.2
16.....	3.60	5.50	27.70	5.40	2.90	2.50	1.80	1.90	.80	.60	.5	2.0
17.....	4.00	8.90	29.20	5.20	2.80	2.10	5.20	1.60	.60	.60	.9	1.8
18.....	4.20	9.50	25.80	4.80	2.80	2.00	4.20	1.40	.60	.60	.7	1.7
19.....	4.00	8.50	24.90	4.70	2.80	2.00	4.80	1.10	.60	.60	.5	1.3
20.....	3.70	7.70	26.20	4.60	2.80	2.00	8.80	.90	.70	.60	.5	1.6
21.....	3.30	6.80	24.60	4.30	2.60	1.80	12.80	.30	.70	.70	.5	2.0
22.....	3.20	6.90	23.00	4.10	2.60	2.20	7.90	.80	.60	.70	.4	2.0
23.....	3.10	7.30	22.60	4.00	2.40	1.70	4.80	.80	.60	.60	1.0	1.8
24.....	3.50	6.60	21.90	5.40	2.60	1.70	3.90	.70	.50	.50	2.1	7.2
25.....	3.80	5.80	18.00	7.40	2.50	1.70	2.60	.70	.50	.40	1.5	7.5
26.....	3.80	5.50	10.50	9.10	2.40	1.70	2.60	.70	.50	.40	2.5	5.0
27.....	3.30	19.10	7.70	6.70	2.20	2.10	3.80	2.50	.50	.40	3.0	3.5
28.....	3.00	23.40	6.80	5.50	2.20	1.90	3.00	2.50	.60	.40	2.2	3.0
29.....	3.00	.....	8.80	4.80	2.00	1.90	2.40	2.50	.50	.40	1.9	3.0
30.....	2.90	.....	9.30	4.20	2.00	1.80	1.40	2.00	.40	.50	1.4	3.4
31.....	4.40	.....	10.20	.....	3.30	.....	1.20	2.50	.....	.80	.....	2.0

*Rating table for Coosa River at Rome, Georgia, for 1899.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
1.0	2,030	8.1	14,941	15.9	30,619	23.0	44,890
1.1	2,124	8.2	15,142	16.0	30,820	23.1	45,091
1.2	2,218	9.0	16,750	16.1	31,021	23.2	45,292
1.3	2,312	9.1	16,951	16.2	31,222	23.3	45,493
1.4	2,406	9.2	17,152	16.3	31,423	23.4	45,694
1.5	2,500	9.3	17,353	16.4	31,624	23.5	45,895
1.6	2,620	9.4	17,554	16.5	31,825	23.6	46,096
1.7	2,740	9.5	17,755	16.6	32,026	23.7	46,297
1.8	2,860	9.6	17,956	16.7	32,227	23.8	46,498
1.9	2,980	9.7	18,157	16.8	32,428	23.9	46,699
2.0	3,100	9.8	18,358	16.9	32,629	24.0	46,900
2.1	3,260	9.9	18,559	17.0	32,830	24.1	47,101
2.2	3,420	10.0	18,760	17.1	33,031	24.2	47,302
2.3	3,580	10.1	18,961	17.2	33,232	24.3	47,503
2.4	3,740	10.2	19,162	17.3	33,433	24.4	47,704
2.5	3,900	10.3	19,363	17.4	33,634	24.5	47,905
2.6	4,060	10.4	19,564	17.5	33,835	24.6	48,106
2.7	4,220	10.5	19,765	17.6	34,036	24.7	48,307
2.8	4,380	10.6	19,966	17.7	34,237	24.8	48,508
2.9	4,540	10.7	20,167	17.8	34,438	24.9	48,709
3.0	4,700	10.8	20,368	17.9	34,639	25.0	48,910
3.1	4,900	10.9	20,569	18.0	34,840	25.1	49,111
3.2	5,100	11.0	20,770	18.1	35,041	25.2	49,312
3.3	5,300	11.1	20,971	18.2	35,242	25.3	49,513
3.4	5,500	11.2	21,172	18.3	35,443	25.4	49,714
3.5	5,700	11.3	21,373	18.4	35,644	25.5	49,915
3.6	5,900	11.4	21,574	18.5	35,845	25.6	50,116
3.7	6,100	11.5	21,775	18.6	36,046	25.7	50,317
3.8	6,300	11.6	21,976	18.7	36,247	25.8	50,518
3.9	6,500	11.7	22,177	18.8	36,448	25.9	50,719
4.0	6,700	11.8	22,378	18.9	36,649	26.0	50,920
4.1	6,901	11.9	22,579	19.0	36,850	26.1	51,121
4.2	7,102	12.0	22,780	19.1	37,051	26.2	51,322
4.3	7,303	12.1	22,981	19.2	37,252	26.3	51,523
4.4	7,504	12.2	23,182	19.3	37,453	26.4	51,724
4.5	7,705	12.3	23,383	19.4	37,654	26.5	51,925
4.6	7,906	12.4	23,584	19.5	37,855	26.6	52,126
4.7	8,107	12.5	23,785	19.6	38,056	26.7	52,327
4.8	8,308	12.6	23,986	19.7	38,257	26.8	52,528
4.9	8,509	12.7	24,187	19.8	38,458	26.9	52,729
5.0	8,710	12.8	24,388	19.9	38,659	27.0	52,930
5.1	8,911	12.9	24,589	20.0	38,860	27.1	53,131
5.2	9,112	13.0	24,790	20.1	39,061	27.2	53,332
5.3	9,313	13.1	24,991	20.2	39,262	27.3	53,533
5.4	9,514	13.2	25,192	20.3	39,463	27.4	53,734



*Rating table for Coosa River at Rome, Ga., for 1899.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
5.5	9,715	13.3	25,393	20.4	39,664	27.5	53,935
5.6	9,916	13.4	25,594	20.5	39,865	27.6	54,136
5.7	10,117	13.5	25,795	20.6	40,066	27.7	54,337
5.8	10,318	13.6	25,996	20.7	40,267	27.8	54,538
5.9	10,519	13.7	26,197	20.8	40,468	27.9	54,739
6.0	10,720	13.8	26,398	20.9	40,669	28.0	54,940
6.1	10,921	13.9	26,599	21.0	40,870	28.1	55,141
6.2	11,122	14.0	26,800	21.1	41,071	28.2	55,342
6.3	11,323	14.1	27,001	21.2	41,272	28.3	55,543
6.4	11,524	14.2	27,202	21.3	41,473	28.4	55,744
6.5	11,725	14.3	27,403	21.4	41,674	28.5	55,945
6.6	11,926	14.4	27,604	21.5	41,875	28.6	56,146
6.7	12,127	14.5	27,805	21.6	42,076	28.7	56,347
6.8	12,328	14.6	28,006	21.7	42,277	28.8	56,548
6.9	12,529	14.7	28,207	21.8	42,478	28.9	56,749
7.0	12,730	14.8	28,408	21.9	42,679	29.0	56,950
7.1	12,931	14.9	28,609	22.0	42,880	29.1	57,151
7.2	13,132	15.0	28,810	22.1	43,081	29.2	57,352
7.3	13,333	15.1	29,011	22.2	43,282	29.3	57,553
7.4	13,534	15.2	29,212	22.3	43,483	29.4	57,754
7.5	13,735	15.3	29,413	22.4	43,684	29.5	57,955
7.6	13,936	15.4	29,614	22.5	43,885	29.6	58,156
7.7	14,137	15.5	29,815	22.6	44,086	29.7	58,357
7.8	14,338	15.6	30,016	22.7	44,287	29.8	58,558
7.9	14,539	15.7	30,217	22.8	44,488	29.9	58,759
8.0	14,740	15.8	30,418	22.9	44,689	30.0	58,960

The following discharge measurements were made during 1900 by Max Hall and others:

Feb. 21—Gage height, 4.80 feet; discharge, 8,115 second-feet.

May 19—Gage height, 2.30 feet; discharge, 4,496 second-feet.

Sept. 13—Gage height, 0.90 foot; discharge, 1,992 second-feet.

Dec. 8—Gage height, 3.73 feet; discharge, 6,066 second-feet.

*Daily gage height, in feet, of Coosa River, at Rome, Georgia, for 1900.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	2.0	2.0	4.2	4.4	6.2	2.4	10.5	3.4	1.5	1.2	2.1	3.2
2.....	1.6	1.8	5.8	4.2	4.8	2.5	8.0	3.2	1.5	1.2	2.2	2.8
3.....	1.5	1.6	5.6	4.0	4.0	2.8	8.0	3.0	1.7	1.0	2.0	2.6
4.....	1.5	2.0	4.4	4.0	4.0	4.2	7.0	2.8	1.7	1.0	2.3	3.5
5.....	1.5	3.0	4.1	4.0	3.8	4.2	6.5	2.6	1.5	.9	2.3	7.4
6.....	1.5	3.8	3.8	4.0	3.7	4.2	4.2	2.5	1.5	.9	2.1	6.8
7.....	1.5	2.8	5.0	3.8	3.6	4.8	3.8	2.2	1.4	.9	2.1	5.2
8.....	1.5	2.4	8.2	3.8	3.4	13.0	3.8	2.2	1.0	3.8	2.0	3.8
9.....	1.5	4.0	15.0	3.6	3.0	12.6	4.0	2.0	1.0	5.9	1.9	3.6
10.....	1.5	6.9	13.4	3.5	3.0	8.0	4.9	2.0	.8	2.6	1.8	3.2
11.....	2.0	7.0	10.3	6.0	3.0	5.9	3.8	1.8	.8	2.0	1.8	2.8
12.....	7.0	6.4	7.5	11.0	3.0	5.0	3.4	1.8	.8	1.8	1.6	2.6
13.....	9.0	22.6	5.5	7.4	2.5	5.2	3.8	1.8	.8	2.5	1.5	2.6
14.....	7.2	27.2	4.8	5.5	2.4	5.3	3.4	1.7	.8	3.2	1.5	2.4
15.....	5.5	25.3	4.2	4.5	2.4	4.2	3.4	2.0	6.5	3.0	1.5	2.2
16.....	3.5	21.2	5.3	5.6	2.4	3.8	3.3	1.7	11.1	2.0	1.5	2.2
17.....	3.0	18.0	5.6	6.2	2.4	4.8	3.1	1.6	7.0	1.6	1.5	2.2
18.....	2.9	10.7	4.5	11.0	2.4	6.0	3.0	1.8	3.2	1.5	1.4	2.0
19.....	5.0	5.0	5.2	11.1	2.9	6.5	2.8	2.2	2.3	1.5	1.4	2.0
20.....	11.3	4.0	15.9	11.4	3.0	7.2	2.6	2.0	2.0	1.4	1.6	2.8
21.....	10.6	4.1	17.5	13.6	2.6	4.2	2.5	1.6	1.8	1.8	1.8	6.7
22.....	8.5	6.8	14.6	12.7	2.5	3.6	2.4	1.6	1.8	1.8	2.1	8.0
23.....	5.8	7.6	10.4	10.5	2.3	5.5	2.4	1.6	1.6	1.6	2.1	7.0
24.....	4.0	6.0	7.2	8.6	2.9	14.2	2.4	1.9	1.6	1.6	2.0	6.6
25.....	3.4	5.8	8.8	8.5	3.2	18.2	3.6	2.4	1.5	1.5	5.0	6.6
26.....	3.1	5.2	13.0	6.5	2.7	17.0	2.8	2.0	1.5	1.5	11.0	5.6
27.....	2.8	4.6	12.1	5.3	2.6	15.5	6.2	1.8	1.4	1.4	11.5	4.0
28.....	2.6	4.0	8.9	4.8	2.5	15.6	6.8	1.6	1.4	2.2	8.6	3.8
29.....	2.4	.....	5.8	4.3	2.4	14.2	6.2	1.5	1.3	2.2	7.0	3.6
30.....	2.1	.....	5.7	6.0	2.9	10.9	4.5	1.5	1.3	2.1	4.0	3.5
31.....	2.0	.....	5.3	.....	3.0	.....	4.0	1.5	.....	2.1	.....	5.6

The following discharge measurements were made during 1901 by Max Hall and others:

Jan. 23—Gage height, 3.60 feet; discharge, 6,454 second-feet.

April 5—Gage height, 9.90 feet; discharge, 16,692 second-feet.

June 22—Gage height, 3.70 feet; discharge, 6,030 second-feet.

Oct. 15—Gage height, 3.15 feet; discharge, 5,388 second-feet.

# WATER-POWERS OF ALABAMA.

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*Daily gage height of Coosa River at Rome, Georgia, for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1	7.4	6.4	3.0	8.8	4.0	10.6	3.6	1.8	6.4	2.6	1.2	1.3
2	6.4	5.8	3.0	8.6	3.8	7.6	8.0	1.8	5.8	2.6	1.2	1.3
3	5.2	5.6	3.0	13.0	3.8	5.6	3.0	1.8	3.7	3.2	1.2	1.3
4	4.2	15.8	3.0	13.0	3.8	6.4	2.6	1.6	3.4	3.0	1.2	1.5
5	4.0	18.5	3.0	10.0	3.6	5.0	2.4	1.6	3.0	2.8	1.2	2.0
6	3.8	13.8	3.0	7.9	3.5	4.0	2.2	2.6	2.9	2.2	1.2	1.8
7	3.5	9.5	3.0	6.4	3.5	7.0	5.2	5.3	2.6	2.0	1.2	1.8
8	3.2	6.5	3.0	5.6	3.4	7.5	4.8	5.9	2.2	1.9	1.1	1.8
9	3.0	9.6	2.8	5.2	3.3	5.4	3.3	3.0	2.0	1.9	1.1	1.8
10	2.8	12.5	5.5	4.5	3.1	4.3	2.6	2.6	2.0	1.8	1.1	1.8
11	8.8	10.5	7.8	4.3	3.0	4.0	2.4	2.5	2.0	1.7	1.0	2.6
12	23.5	7.6	8.0	4.2	2.9	3.8	2.3	3.4	2.0	1.6	1.0	2.6
13	27.0	6.5	6.7	4.3	2.8	3.8	2.0	3.0	1.8	1.8	1.0	2.1
14	23.8	5.6	4.8	10.4	2.8	4.0	2.0	2.3	2.0	2.0	1.0	3.2
15	21.4	5.0	4.0	10.1	2.7	4.3	2.0	4.5	3.0	3.2	1.0	16.4
16	19.8	4.8	3.6	7.7	2.6	6.9	1.9	7.2	2.4	2.6	1.0	17.6
17	17.4	4.2	3.2	5.8	2.6	6.0	1.7	10.5	6.0	2.4	1.0	14.7
18	8.9	4.2	3.0	5.2	2.5	5.0	5.5	9.8	11.2	2.4	1.0	11.0
19	5.0	4.2	3.0	9.0	2.5	4.8	3.0	10.8	11.1	2.0	1.0	13.0
20	4.0	4.0	3.0	18.6	3.0	4.0	3.0	12.5	7.0	1.8	1.0	5.6
21	3.8	3.8	3.0	17.2	10.0	3.8	2.4	10.8	3.9	1.8	1.6	3.0
22	3.8	3.7	3.0	15.5	23.6	3.6	2.4	14.5	3.7	1.6	1.2	3.0
23	3.8	3.6	3.0	14.6	26.4	3.6	2.4	20.8	3.3	1.6	1.4	2.0
24	3.8	3.6	3.6	12.7	21.8	3.6	2.0	23.2	2.8	1.6	1.4	3.6
25	6.7	3.5	3.6	6.8	18.9	2.7	2.0	18.3	2.6	1.6	1.3	4.0
26	6.6	3.2	22.0	5.6	16.5	2.7	1.9	13.1	2.5	1.4	1.3	3.7
27	5.4	3.2	27.0	4.8	11.1	3.2	1.7	6.6	2.3	1.3	1.3	5.7
28	5.2	3.0	24.5	4.4	5.5	3.0	2.8	8.8	2.0	1.3	1.3	6.0
29	5.0	.....	21.3	4.2	4.9	3.6	1.9	7.5	2.0	1.3	1.3	21.5
30	4.6	.....	19.2	4.1	4.7	3.6	1.9	6.2	2.5	1.3	1.3	29.8
31	6.8	.....	16.1	.....	5.4	....	1.6	5.6	.....	1.2	.....	32.6

*Rating table for Coosa River at Rome, Ga., for 1900 and 1901.*

Gage height.	Discharge.		Gage height.	Discharge.		Gage height.	Discharge.		Gage height.	Discharge.	
	Feet.	Second ft.		Feet.	Second ft.		Feet.	Second ft.		Feet.	Second ft.
0.8	1,930		1.6	2,850		2.4	4,000		3.2	5,230	
0.9	2,020		1.7	2,985		2.5	4,150		3.3	5,405	
1.0	2,110		1.8	3,120		2.6	4,300		3.4	5,580	
1.1	2,230		1.9	3,260		2.7	4,450		3.5	5,755	
1.2	2,350		2.0	3,400		2.8	4,600		*3.6	5,930	
1.3	2,475		2.1	3,550		2.9	4,750				
1.4	2,600		2.2	3,700		3.0	4,900				
1.5	2,725		2.3	3,850		3.1	5,065				

\*Above 3.6 ft. gage height, the rating for 1900-1901 is the same as for 1899.

*Estimated monthly discharge of Coosa River at Rome, Ga.*  
 [Drainage area, 4,006 square miles.]

Month.	Discharge in second-feet.			Total in acre-ft.	Run-off.	
	Maxi- mum.	Mini- mum.	Mean.		Depth in inches.	Second- ft. per sq. mile.
1897.						
January . . . . .	17,025	1,800	4,820	296,372	1.38	1.20
February . . . . .	20,925	4,220	10,100	560,926	2.62	2.52
March . . . . .	44,910	4,700	22,537	1,385,755	6.49	5.63
April . . . . .	35,150	4,860	12,304	732,137	3.43	3.07
May . . . . .	8,250	2,930	4,421	271,838	1.27	1.10
June . . . . .	4,540	1,900	2,884	171,610	0.80	0.72
July . . . . .	23,460	1,800	5,184	318,754	1.50	1.30
August . . . . .	4,860	1,360	2,256	138,717	0.64	0.56
September . . . . .	1,900	900	1,106	65,811	0.31	0.28
October . . . . .	2,570	1,010	1,518	93,339	0.44	0.38
November . . . . .	2,000	1,440	1,626	96,754	0.46	0.41
December . . . . .	9,810	1,900	4,086	251,240	1.18	1.02
1898.						
January . . . . .	26,970	2,220	7,272	447,138	2.10	1.82
February . . . . .	5,520	2,110	2,705	150,228	0.71	0.68
March . . . . .	20,730	2,110	4,384	269,563	1.27	1.10
April . . . . .	32,040	4,540	9,430	561,123	2.63	2.36
May . . . . .	4,220	2,330	2,778	170,814	0.79	0.69
June . . . . .	5,520	2,110	2,866	170,538	0.80	0.72
July . . . . .	6,690	1,900	3,670	225,661	10.59	9.17
August . . . . .	17,805	2,930	6,079	373,786	1.75	1.52
September . . . . .	45,885	3,060	12,114	720,832	2.26	3.03
October . . . . .	44,910	3,060	11,830	727,403	3.41	2.96
November . . . . .	12,150	3,060	5,213	310,194	1.45	1.30
December . . . . .	8,250	3,610	4,996	307,194	1.44	1.25
1899.						
January . . . . .	10,519	4,060	6,092	374,582	1.75	1.52
February . . . . .	54,538	8,710	22,536	1,251,586	5.85	5.62
March . . . . .	57,352	7,705	26,314	1,617,985	7.57	6.57
April . . . . .	28,810	6,700	13,333	793,369	3.72	3.33
May . . . . .	6,700	3,100	4,783	294,095	1.37	1.19
June . . . . .	6,700	2,740	3,489	207,610	0.97	0.87
July . . . . .	24,388	1,950	5,499	338,120	1.58	1.37
August . . . . .	3,900	1,790	2,595	159,560	0.75	0.65
September . . . . .	5,500	1,550	2,219	132,040	0.61	0.55
October . . . . .	2,030	1,470	1,684	103,545	0.48	0.42
November . . . . .	4,700	1,470	2,009	119,544	0.56	0.50
December . . . . .	13,735	1,870	4,314	265,258	1.25	1.08

*Estimated monthly discharge of Coosa River at Rome, Georgia.*

[Drainage area, 4,006 square miles.]

Month.	Discharge in second-feet.			Total in acre-feet.	Run-off.	
	Maximum.	Minimum.	Mean.		Depth in inches.	Second-ft. per sq. mile.
1900.						
January .....	21,373	2,725	6,854	421,436	1.97	1.71
February .....	53,332	2,850	14,736	818,396	3.83	3.68
March .....	33,835	6,300	14,714	904,728	4.33	3.67
April .....	25,996	5,755	12,050	717,025	3.36	3.01
May .....	11,122	3,850	5,129	315,370	1.48	1.28
June .....	35,242	4,000	14,154	842,222	3.94	3.53
July .....	19,765	4,000	7,589	466,629	2.18	1.89
August .....	5,580	2,725	3,488	214,469	1.00	0.87
September .....	20,971	1,930	3,960	235,636	1.10	0.99
October .....	10,519	2,010	3,408	209,550	0.98	0.85
November .....	21,775	2,600	5,438	323,583	1.52	1.36
December .....	14,740	3,400	7,096	436,316	2.04	1.77
The year .....	53,332	1,930	8,218	5,905,360	27.73	2.05

*Estimated monthly discharge of Coosa River at Rome, Ga.*

[Drainage area, 4,006 square miles.]

Month.	Discharge in second-feet.			Run-off.	
	Maximum.	Minimum.	Mean.	Depth in inches.	Second-feet per square mile.
1901.					
January .....	52,930	4,600	15,450	4.45	3.86
February .....	35,845	4,900	12,186	3.17	3.04
March .....	52,930	4,600	13,406	3.85	3.34
April .....	36,046	6,901	15,578	4.33	3.88
May .....	51,724	4,150	12,533	3.60	3.12
June .....	19,966	4,450	8,316	2.32	2.08
July .....	9,715	2,850	4,441	1.27	1.10
August .....	45,292	2,850	13,780	3.97	3.44
September .....	21,172	3,120	6,389	1.77	1.59
October .....	5,230	2,350	3,414	.98	.85
November .....	2,850	2,110	2,316	.65	.58
December .....	64,186	2,475	13,428	3.86	3.35
The year .....	64,186	2,110	10,103	34.22	2.52

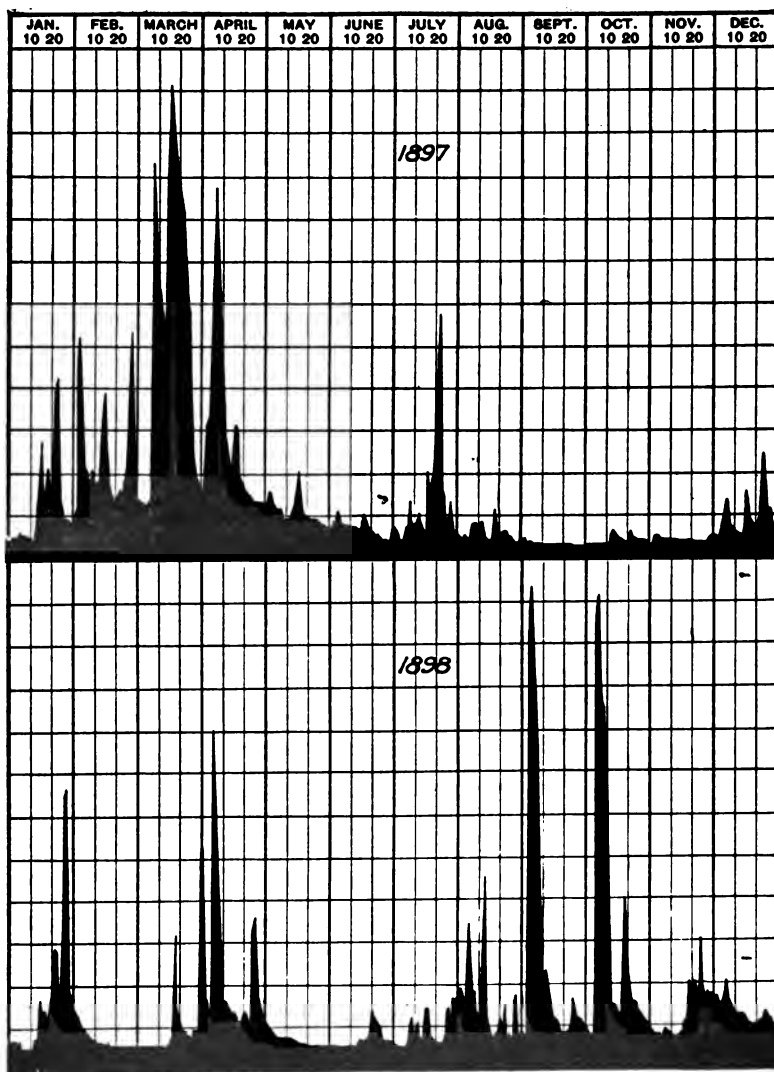


Fig. 12—Discharge of Coosa River at Rome, Ga., 1897 and 1898.

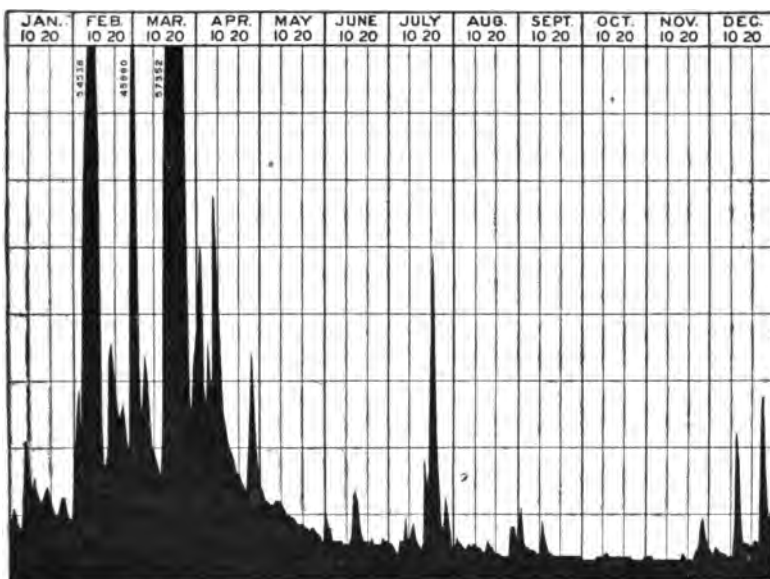


Fig. 13—Discharge of Coosa River at Rome, Ga., 1899.

*Minimum monthly discharge of Coosa River at Rome, Ga., with corresponding net horse-power per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

	1899			1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January ..	4,060	369	2	2,725	248	8	4,600	418	1
February ..	8,710	792	2	2,850	259	1	4,900	445	1
March .....	7,705	700	1	6,300	573	1	4,600	418	1
April .....	6,700	609	1	5,755	523	1	6,901	627	1
May .....	3,100	282	2	3,850	350	1	4,150	377	2
June .....	2,740	249	4	4,000	364	1	4,450	405	2
July .....	1,950	177	1	4,000	364	3	2,850	259	1
August ....	1,790	163	3	2,725	248	3	2,850	259	2
September ..	1,550	141	1	1,930	175	5	3,120	284	1
October ....	1,470	134	4	2,010	183	3	2,350	214	1
November ...	1,470	134	7	2,600	236	2	2,110	192	10
December ..	1,870	170	4	3,400	309	2	2,475	225	3

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table for that month.

## 3. TALLADEGA CREEK AT NOTTINGHAM, ALABAMA.

This station is located on the Southern railroad bridge a fourth of a mile from the depot at Nottingham, Ala., and one mile north of Alpine, Ala. The gage, which is graduated to feet and tenths and is 20 feet long, is fastened vertically to a tree on right bank about 50 feet above the bridge. The initial point of sounding is end of iron bridge right bank up stream. The bench mark is top rail on the upstream side of the bridge, and is 24.13 feet above gage datum. The station is a good one and is free from piers. The observer is R. M. McClatchy, station agent at Nottingham. During 1900 the following measurements were made by James R. Hall:

August 16—Gage height, 1.10 feet; discharge, 102 second-feet.

November 29—Gage height, 1.70 feet; discharge, 240 second-feet.

*Daily gage height in feet of Talladega Creek at Nottingham, Ala., for 1900.*

Day.	Aug.	Sept.	Oct.	Nov.	Dec.	Day.	Aug.	Sept.	Oct.	Nov.	Dec.
1.....		1.2	1.0	1.3	1.6	17.....	1.2	2.3	1.1	1.2	1.5
2.....		2.0	1.0	1.3	1.4	18.....	1.2	1.7	1.1	1.2	1.5
3.....		1.3	1.0	2.3	1.4	19.....	1.1	1.6	1.1	1.2	1.1
4.....		1.1	1.0	2.1	1.8	20.....	1.1	1.5	1.0	1.5	2.7
5.....		1.0	1.2	1.7	1.7	21.....	1.1	1.3	1.2	2.2	3.0
6.....		1.0	1.3	1.6	1.6	22.....	1.1	1.2	1.4	1.9	2.6
7.....		1.0	1.3	1.4	1.5	23.....	1.1	1.2	1.6	1.8	4.0
8.....		.9	1.4	1.4	1.5	24.....	1.0	1.2	2.9	1.6	2.5
9.....		11.0	1.3	1.3	1.4	25.....	.9	1.2	3.5	3.9	2.1
10.....		8.0	1.3	1.3	1.3	26.....	1.0	1.2	3.0	3.8	2.0
11.....		10.3	1.3	1.3	1.3	27.....	1.1	1.2	2.6	3.4	1.8
12.....		8.0	1.5	1.3	1.3	28.....	1.2	1.2	1.4	2.0	1.7
13.....		8.3	1.4	1.2	1.6	29.....	1.0	1.1	1.3	1.8	1.6
14.....		9.3	1.3	1.2	1.8	30.....	1.0	1.1	1.3	1.7	1.9
15.....		9.3	1.2	1.2	1.7	31.....	1.6		1.3		5.1
16.....	1.1	3.9	1.2	1.2	1.6						

The following discharge measurements were made during 1901 by Max Hall and others:

April 5—Gage height, 3.0 feet; discharge, 526 second-feet.

October 22—Gage height, 1.0 feet; discharge, 90 second-feet.



*Daily gage height in feet of Talladega Creek at Nottingham, Ala., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	3.2	2.3	2.0	3.4	2.2	2.0	1.5	1.1	1.2	1.1	1.0	1.0
2.....	2.8	2.1	2.0	5.9	2.2	2.0	1.4	1.0	1.2	2.8	1.0	1.4
3.....	3.2	3.8	2.0	4.5	2.1	2.0	1.3	1.0	1.2	2.5	1.0	1.3
4.....	3.7	8.0	2.0	3.4	2.0	1.8	1.3	1.0	1.1	1.5	1.0	1.2
5.....	2.4	3.9	1.8	3.0	2.0	1.7	1.3	1.0	1.1	1.3	1.0	1.2
6.....	2.2	3.2	1.8	2.9	1.9	1.7	1.4	1.0	1.1	1.2	1.0	1.3
7.....	2.1	2.8	1.8	2.6	1.9	3.0	1.4	1.0	1.1	1.1	1.0	1.1
8.....	2.1	2.8	1.8	2.5	1.9	1.9	1.3	1.0	1.1	1.1	1.0	1.0
9.....	2.0	3.3	1.8	2.3	1.9	1.7	1.3	1.0	1.0	1.1	1.0	1.0
10.....	1.9	2.8	2.0	2.2	1.9	1.6	1.2	1.0	1.0	1.0	1.0	1.1
11.....	5.8	2.6	1.9	2.1	1.9	1.5	1.2	1.0	1.0	1.0	1.0	1.1
12.....	3.8	2.7	1.8	2.1	1.9	1.6	1.2	1.1	1.0	1.0	1.0	1.0
13.....	4.7	2.5	1.8	2.7	2.0	1.6	1.2	1.0	1.0	1.1	1.0	1.0
14.....	3.4	2.3	1.8	3.7	1.9	1.8	1.2	1.0	1.0	1.0	1.0	2.4
15.....	2.9	2.2	1.8	2.7	1.9	1.7	1.2	1.0	1.0	1.0	1.0	3.5
16.....	2.6	2.2	1.7	2.5	1.9	1.6	1.2	2.3	1.0	1.0	1.0	1.7
17.....	2.7	2.2	1.7	2.3	1.8	1.5	1.2	2.2	3.4	1.0	1.0	1.5
18.....	2.4	2.1	1.7	2.4	1.7	1.4	2.0	1.9	2.8	1.0	1.0	1.3
19.....	2.2	2.1	1.7	11.2	1.9	1.4	1.5	1.7	1.6	1.0	1.0	1.2
20.....	2.2	2.0	1.8	6.3	2.5	1.4	1.3	1.5	1.5	1.0	1.0	1.1
21.....	2.1	2.0	2.1	4.1	3.9	1.4	1.3	1.4	1.4	1.0	1.0	1.1
22.....	2.2	2.0	2.0	3.4	2.5	1.3	1.2	1.3	1.3	1.0	1.1	1.1
23.....	2.2	2.0	1.8	3.1	2.0	1.3	1.2	1.2	1.3	1.0	1.2	1.1
24.....	2.2	2.1	2.6	2.8	1.8	1.3	1.2	1.2	1.2	1.0	1.3	1.1
25.....	2.2	2.1	2.8	2.7	1.7	1.3	1.2	1.5	1.2	1.0	1.2	1.3
26.....	2.3	2.1	8.9	2.6	1.7	1.3	1.2	1.3	1.1	1.0	1.1	1.2
27.....	2.2	2.0	4.6	2.5	1.7	1.3	1.2	1.4	1.1	1.0	1.1	1.1
28.....	2.2	2.0	3.3	2.4	1.7	1.3	1.2	1.5	1.1	1.0	1.0	1.2
29.....	2.2	.....	2.8	2.3	1.7	1.3	1.2	1.4	1.1	1.0	1.0	8.4
30.....	2.6	.....	2.9	2.2	1.7	1.4	1.2	1.3	1.1	1.0	1.0	7.5
31.....	2.5	.....	5.5	.....	2.2	...	1.2	1.2	.....	1.0	.....	3.5

*Rating table for Talladega Creek at Nottingham, Ala., for 1900 and 1901.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
1.0	90	2.1	328	3.2	570	4.3	812
1.1	109	2.2	350	3.3	592	4.4	834
1.2	130	2.3	372	3.4	614	4.5	856
1.3	152	2.4	394	3.5	636	4.6	878
1.4	174	2.5	416	3.6	658	4.7	900
1.5	196	2.6	438	3.7	680	4.8	922
1.6	218	2.7	460	3.8	702	4.9	944
1.7	240	2.8	482	3.9	724	5.0	966
1.8	262	2.9	504	4.0	746		
1.9	284	3.0	526	4.1	768		
2.0	306	3.1	548	4.2	790		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Talladega Creek at Nottingham, Ala.*

Month.	Discharge in second-feet.			Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Second- feet per square mile.	Depth in inches.
1900.					
August 16-31 .....			113	0.72	0.43
September .....	2,286	74	575	3.69	4.12
October .....	636	90	190	1.22	1.41
November .....	724	130	249	1.60	1.79
December .....	746	152	291	1.87	2.16
1901.					
January .....	1,802	284	485	3.11	3.59
February .....	1,626	306	449	2.88	3.00
March .....	1,824	240	405	2.60	3.00
April .....	2,330	328	591	3.79	4.23
May .....	724	240	306	1.96	2.26
June .....	526	152	218	1.40	1.56
July .....	196	130	149	.96	1.11
August .....	372	90	148	.95	1.10
September .....	614	90	148	.95	1.06
October .....	482	90	123	.79	.91
November .....	152	90	97	.62	.69
December .....	1,714	90	264	1.69	1.95
The year .....	2,330	90	282	1.81	24.46

*Minimum monthly discharge of Talladega Creek at Nottingham, Ala., with corresponding net horse-power per foot of fall on a water-wheel realizing 80 per cent. of the theoretical power.*

	1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January .....				284	26	1
February .....				306	28	6
March .....				240	22	4
April .....				328	30	2
May .....				240	22	7
June .....				152	14	8
July .....				130	12	18
August .....	66	6	1	90	8	13
September .....	74	6.7	1	90	8	8
October .....	90	8	5	90	8	21
November .....	130	12	7	90	8	24
December .....	152	14	3	90	8	5

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table for that month.

## 4. ALABAMA RIVER AT SELMA, ALABAMA.

This station was originally established by the United States Engineer Corps; readings are now taken by the United States Weather Bureau. The gage, which is attached to the iron highway bridge, the floor of which is about 60 feet above low water, is in two sections. The lower section, which reads from  $-0.3$  feet to  $+2.30$  feet, is secured to the pile on the lower side of the cofferdam on the draw pier; the upper section, which reads from  $2.30$  feet to  $48$  feet, is spiked to the highway bridge. The bench mark, which is an iron bolt driven into the face of a rock bluff  $182.3$  feet from the first bridge pier, on the road ascending to the city, is  $26$  feet above the zero of the gage and  $87.30$  feet above mean sea level. The top of the coping stone of the pivot pier at the highway bridge to which gage is attached is  $56$  feet above the zero of the gage, and  $117.30$  feet above mean sea level. Graduations extend from  $-3.0$  feet to  $+48$  feet. No measurements of discharge were made here during 1899.

*Daily gage height, in feet, of Alabama River at Selma, Ala., for 1899.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	6.2	10.8	35.8	23.8	13.9	4.5	2.5	11.1	3.7	-1.2	-0.2	4.8
2.....	6.2	17.0	36.8	24.3	11.5	4.0	2.5	9.9	4.8	-1.3	-.3	3.7
3.....	6.5	20.2	38.8	24.9	9.9	4.0	2.6	9.0	4.6	-1.3	-.5	3.0
4.....	6.2	24.0	37.7	24.1	9.8	4.0	2.6	6.7	4.8	-1.3	-.6	3.0
5.....	6.2	26.8	35.3	22.3	8.5	3.8	2.1	6.5	4.4	-1.1	-.7	2.8
6.....	5.8	27.2	32.6	20.9	8.1	3.9	1.6	5.8	4.4	-1.0	-.7	2.8
7.....	6.6	27.2	30.5	20.0	7.5	3.8	1.6	6.3	4.1	-1.6	-.8	1.7
8.....	8.3	29.8	27.5	19.8	7.4	3.7	1.5	4.7	3.9	-.5	-.8	1.7
9.....	9.7	32.2	23.4	23.3	7.3	3.3	1.4	4.5	3.6	-.4	-.8	1.3
10.....	11.6	33.9	19.7	25.6	7.3	3.1	1.3	3.7	.6	-.4	-.9	1.3
11.....	13.9	34.4	16.9	26.9	7.5	2.8	1.3	3.5	.4	-.5	-.9	1.4
12.....	14.8	33.9	15.0	26.6	7.2	2.3	1.2	3.0	.3	-.6	-1.0	4.0
13.....	21.2	32.0	13.9	25.1	6.8	2.6	1.5	2.9	.3	-.6	-1.1	10.4
14.....	21.9	30.0	16.2	22.6	6.5	2.4	1.2	2.7	.2	-.2	-1.2	16.6
15.....	19.8	28.0	16.8	19.5	6.3	2.4	1.1	2.6	.2	-.2	-1.2	17.8
16.....	18.0	26.5	19.4	16.3	6.2	2.3	1.0	2.6	.1	-.2	-1.3	16.3
17.....	17.5	26.8	21.4	14.0	6.1	2.8	1.0	2.6	.6	-.3	-1.3	13.4
18.....	17.2	24.0	27.7	13.0	5.6	3.4	.7	3.6	1.3	-.5	-1.3	9.9
19.....	17.0	22.3	31.6	12.0	5.4	3.9	.6	4.1	1.0	-.7	-1.2	8.4
20.....	15.8	19.9	33.5	11.2	5.0	3.9	.6	4.3	.9	-.6	-1.0	3.8
21.....	14.2	19.9	34.7	10.5	4.8	3.1	1.0	3.6	.6	-.7	-1.0	3.8
22.....	12.6	19.5	34.8	10.4	4.5	2.8	1.6	3.5	-.6	-.8	-.6	3.4
23.....	10.2	18.8	34.2	10.2	4.4	1.6	5.5	3.4	-.7	-.2	-.6	3.4
24.....	10.3	17.8	33.4	10.1	4.8	1.4	10.7	3.1	-.8	.0	-.3	3.8
25.....	9.4	17.3	31.1	12.4	5.0	1.4	14.8	3.6	-1.0	.0	.1	7.6
26.....	9.0	16.4	32.6	13.5	6.0	1.6	17.0	4.2	-1.0	-.5	.2	12.3
27.....	8.9	20.3	31.8	16.3	6.1	1.7	17.0	3.9	-1.0	-.5	.9	13.5
28.....	8.6	31.2	30.5	17.9	5.2	2.0	14.9	3.6	-1.0	-.6	3.2	13.7
29.....	8.6	.....	29.3	17.7	4.5	2.2	12.1	4.3	-1.1	-.6	1.6	12.3
30.....	9.0	.....	27.8	16.1	4.3	2.4	12.8	4.6	-1.1	-.6	4.8	11.4
31.....	9.6	.....	26.3	.....	4.2	...	11.9	3.8	.....	-.4	.....	8.3

During 1900 the following measurements were made:

April 14—Gage height, 23.60 feet; discharge 66,607 second-feet.

May 26—Gage height, 6.10 feet; discharge, 17,049 second-feet.

August 24—Gage height, 3.10 feet; discharge, 9,879 second-feet.

*Daily gage height, in feet, of Alabama River at Selma, Ala., for 1900.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	7.2	4.8	17.2	19.8	15.4	4.8	34.8	14.0	3.9	0.8	2.0	16.0
2.....	6.6	3.6	19.7	16.8	13.9	4.6	33.0	13.0	3.9	.7	2.0	14.0
3.....	4.3	3.5	22.2	14.0	13.0	4.4	23.8	11.0	3.8	.6	4.0	11.0
4.....	3.3	3.9	22.0	12.0	12.0	4.2	26.5	9.0	4.5	.6	9.0	9.0
5.....	3.0	4.7	20.6	10.9	11.8	5.6	23.5	7.0	5.4	.5	14.0	8.0
6.....	3.0	6.2	17.8	8.0	10.2	5.1	20.2	6.0	5.0	1.0	13.5	7.5
7.....	3.0	8.2	15.0	7.8	9.6	4.6	17.0	5.5	4.5	1.8	9.4	7.0
8.....	3.0	8.4	13.9	8.9	8.8	4.2	14.0	5.0	4.1	2.5	6.3	7.0
9.....	2.7	8.5	14.9	9.6	8.0	6.8	11.5	4.8	3.3	4.0	4.0	9.0
10.....	2.7	10.7	18.8	9.5	7.8	11.6	10.0	4.5	1.8	4.2	2.0	9.4
11.....	3.3	16.0	20.9	9.8	7.5	13.5	9.8	3.2	1.0	2.0	2.0	6.0
12.....	7.7	22.2	22.2	12.0	7.3	14.0	10.2	3.0	.7	2.0	1.9	5.0
13.....	12.4	29.9	22.0	17.7	7.0	13.9	10.0	2.8	.6	2.5	1.6	2.0
14.....	13.5	38.6	19.9	23.4	6.6	12.8	9.9	2.5	1.0	4.3	1.6	3.2
15.....	14.7	44.0	19.0	25.5	6.4	11.0	9.9	2.5	1.6	6.7	1.0	9.0
16.....	14.0	47.0	16.9	25.0	6.0	9.0	10.0	2.4	11.0	6.0	1.0	11.0
17.....	13.2	49.0	15.3	22.5	5.7	8.9	9.9	2.8	18.0	5.2	1.0	11.0
18.....	12.0	47.9	13.9	23.5	5.5	8.8	9.0	2.7	19.0	2.5	1.0	10.0
19.....	11.1	47.0	14.3	29.0	5.2	8.6	7.0	2.7	19.4	1.0	1.0	3.0
20.....	11.1	44.1	14.6	34.8	5.1	10.0	7.0	2.5	16.0	1.0	1.2	5.1
21.....	13.4	41.6	18.8	39.0	5.0	10.9	6.5	2.3	12.5	.9	1.0	9.0
22.....	16.9	36.9	23.0	39.8	5.0	12.0	6.5	2.6	10.0	1.0	1.6	14.5
23.....	18.5	33.2	25.5	41.0	4.8	12.9	6.3	3.6	6.0	1.5	6.0	17.0
24.....	18.3	22.6	29.0	40.0	5.5	14.0	6.0	3.8	3.0	6.0	9.0	17.2
25.....	17.0	22.6	30.2	38.5	6.1	17.6	5.8	4.0	1.9	11.5	9.8	17.6
26.....	14.7	21.1	32.7	35.8	6.2	24.5	5.0	3.5	1.6	12.0	9.9	18.0
27.....	13.0	19.0	33.3	32.7	6.6	29.0	4.5	3.5	1.0	11.5	13.0	17.0
28.....	11.2	16.9	32.5	28.5	6.8	32.0	4.4	3.4	1.0	12.3	16.0	14.5
29.....	8.4	.....	30.5	23.0	6.0	33.5	7.5	3.5	.9	13.0	16.8	12.9
30.....	6.5	.....	27.7	18.0	5.5	35.0	8.0	4.0	.8	11.0	17.0	11.2
31.....	4.8	.....	24.4	.....	5.0	.....	11.8	4.2	.....	5.0	.....	11.0

# WATER-POWERS OF ALABAMA.

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## *List of Discharge Measurements made on Alabama River at Selma, Alabama, in 1901.*

Date, 1901.	Hydrographer.	Gage height, ft	Dischg. sec.-ft.
March 14.	Max Hall .....	14.20	35,518
April 25..	J. R. Hall .....	34.00	90,332
August 9..	K. T. Thomas .....	4.35	12,519
October 30	Max Hall .....	1.10	7,710

## *Daily gage height, in feet, of Alabama River at Selma, Ala., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	16.0	13.0	11.3	35.6	12.0	19.0	1.0	2.8	17.0	4.3	1.4	1.2
2.....	21.0	13.0	9.8	36.5	10.4	17.0	2.0	2.6	13.8	4.3	1.4	1.8
3.....	24.0	13.6	9.5	37.4	10.0	16.5	2.4	2.6	10.6	5.0	1.4	1.9
4.....	24.6	17.0	9.3	38.5	9.6	18.5	6.6	2.8	8.8	7.4	1.5	2.2
5.....	24.0	24.9	9.6	38.4	8.2	19.0	6.0	3.0	8.0	7.9	1.5	2.3
6.....	23.0	30.1	9.6	37.2	7.0	19.8	5.6	2.8	7.4	6.4	1.4	2.3
7.....	18.0	33.0	9.4	35.5	7.0	18.5	5.6	6.6	6.0	5.8	1.6	2.4
8.....	15.0	35.1	8.0	33.0	6.8	17.4	5.5	4.6	5.2	5.0	1.6	2.1
9.....	13.3	35.6	7.7	28.0	6.4	16.1	5.5	4.2	4.4	4.3	1.5	2.0
10.....	10.0	35.7	7.9	22.6	6.0	14.8	5.5	3.4	4.4	3.4	1.5	2.2
11.....	8.1	33.0	9.0	17.4	5.0	12.0	5.2	4.0	3.6	2.9	1.5	2.4
12.....	16.5	31.4	10.2	14.0	5.0	11.0	5.2	5.3	3.4	2.8	1.5	2.4
13.....	28.0	31.2	12.0	12.0	4.8	9.5	5.1	6.2	3.2	2.6	1.4	2.4
14.....	34.0	27.0	14.1	11.8	4.5	8.0	4.4	4.4	3.7	2.5	1.4	2.6
15.....	38.0	26.0	15.4	12.0	4.0	7.6	3.7	3.8	4.3	2.2	1.4	5.0
16.....	39.5	20.6	15.0	15.0	3.8	7.0	3.7	6.0	4.0	2.5	1.3	10.0
17.....	40.0	16.9	14.8	16.5	3.4	6.1	3.5	7.4	5.0	2.5	1.3	18.0
18.....	39.0	14.6	12.0	17.3	3.2	6.0	4.3	11.0	5.0	2.4	1.3	21.6
19.....	37.5	13.1	11.1	22.0	3.0	5.0	6.3	12.0	5.5	2.0	1.4	22.0
20.....	35.0	12.6	10.5	28.6	3.0	4.1	7.3	16.0	9.5	2.3	1.6	21.5
21.....	32.4	12.0	11.0	35.0	3.5	3.4	6.0	17.6	11.4	2.5	1.8	18.7
22.....	29.0	11.8	11.9	38.0	4.7	3.0	5.5	18.8	11.5	2.5	1.8	15.0
23.....	24.0	11.7	12.2	39.0	9.4	3.0	5.4	20.0	11.2	2.2	1.8	14.2
24.....	22.0	11.6	13.0	38.0	17.0	2.6	5.4	20.9	10.0	2.0	1.8	12.0
25.....	14.0	11.2	14.7	35.8	19.0	2.2	4.4	22.8	7.5	2.0	1.9	11.0
26.....	12.8	11.5	17.0	31.9	20.0	2.0	4.0	24.6	6.0	2.0	1.9	7.1
27.....	12.8	11.4	22.5	28.0	20.9	1.5	4.0	24.8	4.4	1.8	2.0	6.2
28.....	12.7	11.3	27.6	24.2	22.0	1.3	3.6	22.9	4.0	1.6	2.0	6.0
29.....	12.7	.....	31.0	19.5	21.8	1.2	2.9	21.0	4.0	1.6	2.0	11.0
30.....	13.0	.....	33.0	15.0	20.7	1.2	2.9	20.6	4.2	1.3	1.8	23.0
31.....	13.0	.....	34.5	.....	19.5	...	2.8	19.5	.....	1.3	.....	35.0

*Rating table for Alabama River at Selma, Ala., for 1900 and 1901.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
0.0	6,700	7.6	21,116	15.2	41,332	22.8	61,548
0.1	6,770	7.7	21,382	15.3	41,598	22.9	61,814
0.2	6,845	7.8	21,648	15.4	41,864	23.0	62,080
0.3	6,925	7.9	21,914	15.5	42,130	23.1	62,346
0.4	7,010	8.0	22,180	15.6	42,396	23.2	62,612
0.5	7,100	8.1	22,446	15.7	42,662	23.3	62,878
0.6	7,184	8.2	22,712	15.8	42,928	23.4	63,144
0.7	7,282	8.3	22,978	15.9	43,194	23.5	63,410
0.8	7,384	8.4	23,244	16.0	43,460	23.6	63,676
0.9	7,488	8.5	23,510	16.1	43,726	23.7	63,942
1.0	7,596	8.6	23,776	16.2	43,992	23.8	64,208
1.1	7,706	8.7	24,042	16.3	44,258	23.9	64,474
1.2	7,818	8.8	24,308	16.4	44,524	24.0	64,740
1.3	7,931	8.9	24,574	16.5	44,790	24.1	65,006
1.4	8,045	9.0	24,840	16.6	44,056	24.2	65,272
1.5	8,160	9.0	25,106	16.7	45,322	24.3	65,538
1.6	8,270	9.2	25,372	16.8	45,588	24.4	65,804
1.7	8,393	9.3	25,638	16.9	45,854	24.5	66,070
1.8	8,511	9.4	25,904	17.0	46,120	24.6	66,336
1.9	8,630	9.5	26,170	17.1	46,386	24.7	66,602
2.0	8,750	9.6	26,436	17.2	46,652	24.8	66,868
2.1	8,872	9.7	26,702	17.3	46,918	24.9	67,134
2.2	8,996	9.8	26,968	17.4	47,184	25.0	67,400
2.3	9,124	9.9	27,234	17.5	47,450	25.1	67,666
2.4	9,256	10.0	27,500	17.6	47,716	25.2	67,932
2.5	9,392	10.1	27,766	17.7	47,982	25.3	68,198
2.6	9,532	10.2	28,032	17.8	48,248	25.4	68,464
2.7	9,676	10.3	28,298	17.9	48,514	25.5	68,730
2.8	9,822	10.4	28,564	18.0	48,780	25.6	68,996
2.9	9,970	10.5	28,830	18.1	49,046	25.7	69,262
3.0	10,120	10.6	29,096	18.2	49,312	25.8	69,528
3.1	10,272	10.7	29,362	18.3	49,578	25.9	69,794
3.2	10,428	10.8	29,628	18.4	49,844	26.0	70,060
3.3	10,588	10.9	29,894	18.5	50,110	26.1	70,326
3.4	10,752	11.0	30,160	18.6	50,376	26.2	70,592
3.5	10,920	11.1	30,426	18.7	50,642	26.3	70,858
3.6	11,092	11.2	30,692	18.8	50,908	26.4	71,124
3.7	11,268	11.3	30,958	18.9	51,174	26.5	71,390
3.8	11,448	11.4	31,224	19.0	51,440	26.6	71,656
3.9	11,632	11.5	31,490	19.1	51,706	26.7	71,922
4.0	11,820	11.6	31,756	19.2	51,972	26.8	72,188
4.1	12,015	11.7	32,022	19.3	52,238	26.9	72,454
4.2	12,220	11.8	32,288	19.4	52,504	27.0	72,720
4.3	12,435	11.9	32,556	19.5	52,770	27.1	72,986
4.4	12,660	12.0	32,820	19.6	53,036	27.2	73,252
4.5	12,900	12.1	33,086	19.7	53,302	27.3	73,518



# WATER-POWERS OF ALABAMA.

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*Rating table for Alabama River at Selma, Ala., for 1900 and 1901.*  
*Continued.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
<i>Feet.</i>	<i>Second ft.</i>	<i>Feet.</i>	<i>Second ft.</i>	<i>Feet.</i>	<i>Second ft.</i>	<i>Feet.</i>	<i>Second ft.</i>
4.6	13,150	12.2	33,352	19.8	53,568	27.4	73,784
4.7	13,405	12.3	33,618	19.9	53,884	27.5	74,050
4.8	13,668	12.4	33,884	20.0	54,100	27.6	74,316
4.9	13,934	12.5	34,150	20.1	54,366	27.7	74,582
5.0	14,200	12.6	34,416	20.2	54,632	27.8	74,848
5.1	14,466	127.	34,682	20.3	54,898	27.9	75,114
5.2	14,732	12.8	34,948	20.4	55,164	28.0	75,380
5.3	14,998	12.9	35,214	20.5	55,430	28.1	75,646
5.4	15,264	13.0	35,480	20.6	55,696	28.2	75,912
5.5	15,530	13.1	35,746	20.7	55,962	28.3	76,178
5.6	15,796	13.2	36,012	20.8	56,228	28.4	76,444
5.7	16,062	13.3	36,278	20.9	56,494	28.5	76,710
5.8	16,328	13.4	36,544	21.0	56,760	28.6	76,976
5.9	16,594	13.5	36,810	21.1	57,026	28.7	77,242
6.0	16,860	13.6	37,076	21.2	57,292	28.8	77,508
6.1	17,126	13.7	37,342	21.3	57,558	28.9	77,774
6.2	17,392	13.8	37,608	21.4	57,824	29.0	78,040
6.3	17,658	13.9	37,874	21.5	58,090	29.1	78,306
6.4	17,924	14.0	38,140	21.6	58,356	29.2	78,572
6.5	18,190	14.1	38,406	21.7	58,622	29.3	78,838
6.6	18,456	14.2	38,672	21.8	58,888	29.4	79,104
6.7	18,722	14.3	38,838	21.9	59,154	29.5	79,370
6.8	18,988	14.4	39,104	22.0	59,420	29.6	79,636
6.9	19,254	14.5	39,370	22.1	59,686	29.7	79,902
7.0	19,520	14.6	39,676	22.2	59,952	29.8	80,168
7.1	19,786	14.7	40,002	22.3	60,218	29.9	80,434
7.2	20,052	14.8	40,268	22.4	60,484	30.0	80,700
7.3	20,318	14.9	40,534	22.5	60,750		
7.4	20,584	15.0	40,800	22.6	61,016		
7.5	20,850	15.1	41,066	22.7	61,282		

NOTE—This table applied to the foregoing "daily gage heights" gives cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Alabama River at Selma, Ala.*  
 [Drainage area, 13,500 square miles.]

Month.	Discharge in second-feet			Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Depth in inches.	Second- feet per square mile.
1900.					
January .....	50,110	9,676	26,495	1.96	2.26
February .....	128,540	10,920	63,763	4.72	4.91
March .....	89,478	37,874	58,272	4.32	4.98
April .....	109,960	21,648	60,909	4.51	5.03
May .....	41,864	13,668	21,090	1.56	1.80
June .....	94,000	12,220	35,288	2.61	2.91
July .....	93,468	12,660	33,964	2.52	2.90
August .....	38,140	9,124	14,156	1.05	1.21
September .....	52,504	7,189	17,366	1.29	1.44
October .....	35,480	7,097	14,492	1.07	1.23
November .....	46,120	7,596	18,506	1.37	1.53
December .....	48,780	8,750	28,989	2.15	2.48
The year .....	128,540	7,097	33,772	2.34	32.68
1901.					
January .....	107,300	22,446	61,213	4.53	5.22
February .....	95,862	30,692	55,037	4.08	4.25
March .....	92,670	21,382	39,017	2.89	3.33
April .....	104,640	32,288	73,048	5.41	6.04
May .....	59,420	10,120	26,966	2.00	2.31
June .....	53,568	7,818	26,030	1.93	2.15
July .....	21,318	7,596	13,536	1.00	1.15
August .....	66,868	9,532	30,853	2.29	2.64
September .....	46,120	10,428	19,394	1.44	1.61
October .....	21,914	7,931	11,022	.82	.95
November .....	8,750	7,931	8,266	.61	.68
December .....	94,000	8,511	26,638	1.97	2.27
The year .....	107,300	7,596	32,585	2.47	32.60



*Minimum monthly discharge of the Alabama River at Selma, Ala., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

[Drainage area, 15,400 square miles.]

	1899.			1900.			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January ....	16,328	1,484	1	9,676	880	2	22,446	2,041	1
February ...	29,628	2,693	1	10,920	993	1	30,692	2,790	1
March .....	37,874	3,443	1	37,874	3,443	2	21,382	1,944	1
April .....	27,760	2,524	1	21,648	1,968	1	32,288	2,935	1
May .....	12,220	1,111	1	13,668	1,243	1	10,120	920	2
June .....	8,045	731	2	12,220	1,111	2	7,818	711	2
July .....	7,184	653	2	12,660	1,151	1	7,596	691	1
August .....	9,532	867	3	9,124	829	1	9,532	867	2
September ..	5,800	527	2	7,189	653	1	10,428	948	1
October ....	5,400	491	1	7,100	645	1	7,931	721	2
November ...	5,700	518	3	7,596	691	7	7,931	721	3
December ...	7,931	721	2	8,750	795	1	8,511	774	2

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table for that month.

#### 5. MISCELLANEOUS DISCHARGE MEASUREMENTS

Made by B. M. Hall, and Assistants, on Tributaries of Coosa River.

1898.

May 26—Choccolocco Creek, Eureka; discharge, 171 second-feet; low water.

1900.

March 15—Talladega Creek, Kymulga postoffice; discharge, 107 second-feet; medium.

March 16—Tallassahatchee Creek, in Talladega county, Childersburg; discharge, 102 second-feet.

March 17—Hatchet Creek, Goodwater; discharge, 84 second-feet.

April 5—Choccolocco Creek, L. & N. R. R. bridge, near Jenifer; discharge, 1,170 second-feet; high water.

Oct. 16—Big Wills Creek, Wesson's Mill, 2 miles north of Attalla; discharge, 107 second-feet; low water.

## 6. TRIBUTARIES OF THE COOSA RIVER FROM WETUMPKA UP.

From which side.	Name of Stream.	Point on stream.	Drainage area, sq. miles.	Estimated discharge cu. ft. per sec. low water 1900-1901.	Net H.P. per ft. of fall on 80 per cent. turbine.
Left.	Sofkahatchee Creek .....	Mouth of Creek.....	40	12	1.1
Left.	Wewoka Creek .....	Mouth of Creek.....	85	23	2.5
Right	Chestnut Creek .....	Mouth of Creek.....	90	30	2.7
Left.	Hatchet Creek .....	Mouth of Creek.....	500	185	15.0
Left.	Hatchet Creek .....	Goodwater, Ala.....	105	40	3.6
Left.	Pinthlocco Creek .....	Mouth of Creek.....	60	24	2.2
Right	Weogufka Creek .....	Mouth of Creek.....	120	48	4.3
Right	Waxahatchee Creek .....	Mouth of Creek.....	196	75	6.8
Right	Yellow Leaf Creek.....	Mouth of Creek.....	192	75	6.8
Left.	Tallassee-hatchee Creek .....	Mouth of Creek.....	172	70	6.3
Left.	Talladega Creek .....	Mouth of Creek.....	158	75	6.8
Left.	Talladega Creek .....	Nottingham, Ala.....	156	68	6.0
Right	Kelley's Creek .....	Mouth of Creek.....	218	88	8.0
Left.	Choccolocco Creek .....	Mouth of Creek.....	510	153	13.9
Left.	Choccolocco Creek .....	Jenifer, Ala.....	278	95	8.6
Left.	Blue Eye Creek .....	Mouth of Creek.....	26	7	0.6
Right	Broken Arrow Creek.....	Mouth of Creek.....	49	18	1.6
Right	Trout Creek .....	Mouth of Creek.....	23	10	0.9
Left.	Cane Creek .....	Mouth of Creek.....	94	36	3.2
Left.	Ohatchee Creek .....	Mouth of Creek.....	217	86	7.7
Left.	Ohatchee Creek .....	Above Tallassee-hatchee Creek.....	86	35	3.2
Left.	Tallassee-hatchee Creek .....	Mouth of Creek.....	125	50	4.5
Right	Shoal Creek .....	Mouth of Creek.....	31	12	1.1
Right	Beaver Creek .....	Mouth of Creek.....	33	12	1.1
Right	Big Canoe Creek.....	Mouth of Creek.....	248	90	8.2
Right	Big Canoe Creek .....	Above Little Canoe Creek .....	165	65	5.9
Right	Little Canoe Creek.....	Mouth of Creek.....	34	14	1.3
Right	Big Wills Creek .....	Mouth of Creek.....	354	160	14.4
Right	Big Wills Creek .....	Above Little Wills Creek .....	249	115	10.4
Right	Big Wills Creek .....	Above Wesson Mill.....	200	107	9.7
Left.	Black Creek .....	Mouth of Creek.....	59	25	2.3
Right	Little Wills Creek .....	Mouth of Creek.....	30	14	1.3
Left.	Ball Play Creek .....	Mouth of Creek.....	33	15	1.4
Left.	Terrapin Creek .....	Mouth of Creek.....	282	180	11.8
Right	Chattooga River .....	Above Little River..	394	170	15.4
Right	Chattooga River .....	Ala.-Ga. State Line	246	121	11.0
Right	Little River .....	Mouth of River.....	280	180	11.8
Right	Coosa River .....	Ala.-Ga. State Line	4340	2000	181.8

NOTE.—To find the net horsepower available at a shoal on one of these streams, near a point given, for low water 1900-1901, multiply the total fall of the shoal by the "net horsepower per foot fall" in this table for that point.

## 7. WATER POWERS ON TRIBUTARIES OF COOSA RIVER IN ALABAMA.

On the above named tributaries there are many important water powers, very few of which have been surveyed. The above list giving the drainage area, the discharge for low season, 1900-1901, and the corresponding net horse power per foot fall for each of the streams will be very useful in estimat-

ing the horse power available on any shoal, the fall of which may hereafter be surveyed, by the owners, or by parties contemplating development.

Talladega Creek, in the vicinity of Taylor's Mill, has a fall of 73 feet in one mile, where it emerges from the Crystalline rocks. Taking the flow at Nottingham, we say that during the low water of 1900 and 1901 this 73 feet of fall would have produced 438 net horse power without storage. This 73 feet is probably the most precipitous shoal on the large creek, but above it for four or five miles the creek has a number of rapids and shoals that will admit of good development.

The head waters of this stream in the neighborhood of the pyrites mines in Clay county have high falls on them.

Choccolocco Creek is a very large and constant stream, and has many rapids where good powers could be developed by dams. During a season such as low water of 1900 or 1901 a 10-foot dam near Jenifer would develop 86 net H. P. A 10-foot dam at any point near the mouth of the creek would develop 140 net H. P. during the given season.

Big Wills Creek, at the old Wesson mill, two miles north of Attalla, has a good site for a 25-foot dam. The flow at this point on October 16, 1901, was 107 second-feet, which with a fall of 25 feet, will give 242 net H. P. The fall on other tributaries named has not been ascertained.

#### 8. COOSA RIVER SURVEY.

The Coosa River has its beginning at the junction of the Etowah and Oastanaula Rivers, at Rome, Ga., a short distance west of the Alabama line.

From Rome down to Greensport, Ala., a distance of about 180 miles by river, navigation has been carried on for many years. The total fall in this section is only about 55 feet, and is so well distributed that it has not been necessary to construct locks at any point, though improvements have been made by the U. S. Government in the way of deepening channels, blasting out reefs, and building wing dams, etc.

This part of the river will, therefore, not be considered as having any water power value.

Below Greensport, Ala., the river has a large amount of fall, and although it is proposed to make the whole distance navigable by the construction of locks, there are many fine water power propositions which can be developed in connection with

the river improvements without interfering with navigation.

A complete survey has been made of this portion of the river by the U. S. engineers, and a system of locks planned.

The profile herein presented is reproduced from that survey, and shows in addition to the river profile the location of the proposed locks, and the lift of each.

It will be seen that the total distance between Greensport and Wetumpka, Ala., is 142 miles, and the number of locks proposed, 31, varying in lift from 5.83 feet to 15.0 feet. Of these only three have been completed; Nos. 1, 2, and 3. No. 4 is in process of construction.

The following table shows the lift or fall at each lock, the discharge of river in cubic feet per second, for the minimum low stage of water in 1897 and in 1900, and the equivalent net horse power for the fall shown.

The minimum low water is based on the exceptionally low stages occurring in 1896 and 1897, which represents the lowest stage of which there is any record; while the minimum for the year 1900 represents lowest water for average years.

In estimating the amount of horse power that will be available for use, it will be necessary to deduct the amount of water which will be necessary for lockage. This will depend upon the amount of traffic on the river, but will probably in no case amount to more than ten per cent. of the river discharge.

At most of these locks, and proposed locks, reservations have been made by the original owners of the river front of the privilege of utilizing for power the water not needed for lockage. By constructing a plant at the opposite end of the Government dam from the lock, the surplus water can be used for power without interfering with navigation. Such powers will be very valuable for running cotton mills, as the cotton furrows will run up to the front door of the factory, and water transportation will take the goods from the back door. Mobile, at the mouth of the river, is only a short distance from the proposed Isthmian canal.

2. TABLE OF DISCHARGE, AND NET H. P. AT THE 31 LOCKS AND PROPOSED LOCKS ON THE COOSA RIVER AT LOWEST WATER OF 1897 AND 1900.

[80 % of Theoretical H. P.]

Miles from Wetumpka, Ala.	Lock No.	Tide Water Elevation, top of lock.	Lift or Fall in feet at lock.	Minimum Low Water, 1897.		Minimum Low Water, 1900.	
				Cubic ft. per second.	Net H. P.	Cubic ft. per second.	Net H. P.
141.5	1	521.30	5.33	1,320	640	2,700	1,308
138.5	2	515.97	5.57	1,320	668	2,700	1,367
137.0	3	510.40	12.00	1,320	1,440	2,700	2,945
116.2	4	492.30	10.00	1,350	1,227	2,760	2,510
105.8	5	482.30	12.00	1,350	1,472	2,760	3,012
92.0	6	455.32	10.00	1,440	1,310	2,940	2,673
88.3	7	445.32	10.00	1,450	1,317	2,960	2,690
81.3	8	435.32	12.00	1,490	1,625	3,040	3,317
56.2	9	420.00	8.00	1,580	1,149	3,220	2,342
53.5	10	412.00	12.00	1,585	1,728	3,230	3,523
46.7	11	399.64	10.00	1,585	1,440	3,230	2,936
44.9	12	389.64	10.00	1,600	1,454	3,260	2,964
43.0	13	379.64	12.00	1,600	1,745	3,260	3,557
41.9	14	367.64	12.00	1,600	1,745	3,260	3,557
40.2	15	355.64	10.00	1,605	1,460	3,270	2,973
37.5	16	345.64	14.00	1,605	2,044	3,270	4,162
36.1	17	331.64	15.00	1,605	2,190	3,270	4,460
34.8	18	316.64	13.00	1,610	1,903	3,280	3,877
33.8	19	303.64	12.00	1,610	1,757	3,280	3,578
31.5	20	291.64	10.00	1,610	1,464	3,280	2,982
25.5	21	281.33	10.00	1,700	1,545	3,460	3,145
21.4	22	270.80	12.00	1,700	1,854	3,460	3,774
18.5	23	258.80	14.00	1,710	2,175	3,480	4,430
16.3	24	244.80	10.00	1,710	1,554	3,480	3,164
12.9	25	234.80	10.00	1,710	1,554	3,480	3,164
11.7	26	224.80	12.00	1,720	1,877	3,500	3,818
8.8	27	212.80	14.00	1,720	2,190	3,500	4,455
7.4	28	198.80	12.00	1,720	1,877	3,500	3,818
4.6	29	186.37	8.00	1,740	1,266	3,540	2,574
2.0	30	178.37	10.00	1,740	1,582	3,540	3,218
0.0	31	168.37	14.00	1,740	2,215	3,540	4,505
Total net H. P. ....				49,467			100,798

Locks and proposed locks on Coosa River are located as follows:

Lock No. 1 is one mile south of Greensport, Ala., and five miles north of Singleton, which is a station on the East & West Railroad of Alabama. Lock No. 1 is three miles above lock No. 2.

Lock No. 2 is one and a half miles above lock No. 3, and is located at the head of Woods Island., and two miles northeast of Singleton, Ala., which is a station on the East & West Railroad. This lock is situated at the head of Ten Island Shoal Canal.

Lock No. 3 is one and a half miles below lock No. 2, near the foot of Woods Island, and on Ten Island Shoal Canal. It is one mile east of Singleton, Ala., and 20.8 miles above lock No. 4.

Lock No. 4 is three and a half miles above the U. S. G. S. Hydrographic Station, Riverside, Ala., and three miles northwest of Lincoln, Ala. Lincoln and Riverside are on the Georgia Pacific division of the Southern Railway. Lock No. 4 has a lift of 12 feet, and is three-quarters of a mile below Denson's Island, and ten miles above proposed lock No. 5.

Proposed lock No. 5 is to be at the head of Ogletree Island, one mile above the mouth of Choccolocco Creek, and five miles northeast of Hamilton, on the Talladega & Coosa Valley Railroad. Has a lift of ten feet.

Proposed lock No. 6 is to be located one-fourth of a mile above the mouth of Upper Clear Creek, one and a half miles above Grissom's Ferry, and nine miles north-east of Vincent, which is a station on the Columbus & Western division of the Central of Georgia Railroad.

Proposed lock No. 7 is to be located two miles above Kelly Creek, and five and a half miles north-east of Vincent, Ala.

Proposed lock No. 8 is to be located at Myer's Ferry, at the mouth of Lower Clear Creek, six miles east of Harpersville, and three miles north-east of Creswell, which is a station on the Columbus & Western division of the Central of Georgia Railroad.

Proposed lock No. 9 is to be located at the mouth of Kelly Branch, at Fort Williams Shoals. It is to be thirteen and a half miles east of Columbiana, Ala., and eight miles east of Shelby, Ala.

Lock No. 10 is to be located a half mile above Peckerwood Creek, at the foot of Peckerwood Shoals, and is eight miles east of Shelby, Ala., and two miles west of Talladega Springs, Ala.

Lock No. 11 is to be located at the foot of Weduska Shoals, immediately above the narrows, two miles above Waxahatchee Creek, and six miles south-east of Shelby, Ala., which is a station on the Shelby Iron Works Railroad, connecting with the E. T., V. & G. R. R. at Columbiana, Ala.

Lock No. 12 is to be located 1.8 miles below lock No. 11, immediately below the mouth of Waxahatchee Creek, and eight miles south-east of Shelby, Ala.

Lock No. 13 is to be located 1.9 miles below lock No. 12, at a place known as Devil's Race, three miles above the mouth of Lower Yellow Leaf Creek, and sixteen miles north-east of Clanton, Ala., on the L. & N. R. R.

Lock No. 14 is to be located one mile below lock No. 13, two miles above Yellow Leaf Creek, and fourteen miles north-east of Clanton, Ala.

Lock No. 15 is to be located 1.7 miles below lock No. 14, three-tenths of a mile above Lower Yellow Leaf Creek, and twelve miles north-east of Clanton, Ala., on the L. & N. R. R.

Lock No. 16 is to be located 2.7 miles below lock No. 15, at Butting Ram Shoals, which is eleven miles north-east of Clanton, Ala.

Lock No. 17 is to be located 1.4 miles below lock No. 16, and is ten and a half miles north-east of Clanton, Ala.

Lock No. 18 is to be located 1.3 miles below lock No. 17, and eleven miles east of Clanton, Ala.

Lock No. 19 is to be located one mile below lock No. 18, about eleven miles east of Clanton, Ala.

Lock No. 20, 31.5 miles above Wetumpka, one-fourth mile above Zimmerman's Ferry, 1.2 miles above the mouth of Hatchet Creek.

Lock No. 21, 25.5 miles above Wetumpka, 1.6 miles below mouth of Blue Creek, 7 miles east of Cooper, Ala., on L. & N. R. R.

Lock No. 22, 21.4 miles above Wetumpka, three-fourths of a mile below the mouth of Proctors Creek, and 1.1 miles above the mouth of Pinchoulee Creek, and 7 miles east of Verbena, Ala., on the L. & N. R. R.

Lock No. 23, 18.5 miles from Wetumpka, 1.5 miles below the mouth of Pinchoulee Creek.

Lock No. 24, 16. miles above Wetumpka, 0.4 miles below the mouth of Welcree Creek, and seven and a half miles east of Mountain Creek Station, on the L. & N. R. R.

Lock No. 25, 12.9 miles above Wetumpka, 0.1 miles above the mouth of Shoal Creek, and about 8 miles east of Wadsworth, Ala., on the L. & N. R. R.

Lock No. 26, 11.7 miles above Wetumpka, at Staircase Falls, just above the mouth of Wewoka Creek.

Lock No. 27, 8.8 miles above Wetumpka, 0.6 miles above the mouth of Sofkahatchee Creek, and about nine miles east of Deatsville, Ala., on the L. & N. R. R.

Lock No. 28, 7.4 miles above Wetumpka.

Lock No. 29, 4.6 miles above Wetumpka.

Lock No. 30, 2 miles above Wetumpka.

Lock No. 31, at Wetumpka, Ala.



## CHAPTER IV.

### 1. CAHABA RIVER AT CENTERVILLE, ALABAMA.

Centerville Station, on Cahaba River, is at the Bibb county highway bridge, one-fourth of a mile west of the court house at Centerville, Ala. The bridge is a single span iron through bridge. The length of the span is about 175 feet. The floor of the bridge is 41½ feet above low water, and the stream is 130 feet wide at low water.

The initial point of sounding is at the end of the iron bridge, left bank, down stream. The gage is a wire gage, with rod fastened to the outside of down stream guard rail, and graded to feet and tenths. The gage pulley is at Station 100. Bench mark No. 1, down stream end of top of iron crossbeam under the bridge floor at Station 100, from initial point is 42.85 above gage datum.

Bench mark No. 2, top of bottom flange of same crossbeam, directly under B. M. No. 1, is 41.40 above datum of gage.

Banks are high, but overflow at time of high water.

The section is swift, and tolerably uniform, and the bottom appears to be rock.

The river observer is Mr. S. D. Hall, a merchant, who lives about a quarter of a mile from the gage.

The following discharge measurements have been made on Cahaba River at Centerville, Ala.:

1901.

April 25—Hydrographer, J. R. Hall; gage height, 5.50; discharge, 1,925 second-feet.

Aug. 7—Hydrographer, K. T. Thomas; gage height, 1.30; discharge, 399 second feet.

1902.

Jan. 25—Hydrographer, K. T. Thomas; gage height, 5.15; discharge, 1,707 second-feet.

#### *Daily gage height of Cahaba River at Centerville, Ala, for 1901.*

Day	Aug.	Sept.	Oct.	Nov.	Dec	Day	Aug.	Sept.	Oct.	Nov.	Dec
1.....		2.20	2.10	1.20	1.4	17.....	7.70	2.10	1.30	1.30	12.3
2.....		2.00	2.00	1.20	1.3	18.....	8.10	2.60	1.30	1.30	4.4
3.....		1.90	1.80	1.20	1.6	19.....	9.10	3.90	1.30	1.70	3.6
4.....		1.80	1.40	1.20	1.9	20.....	10.60	4.10	1.30	1.60	3.1
5.....		1.60	2.30	1.20	1.8	21.....	14.70	2.00	1.30	1.60	2.9
6.....		1.50	4.60	1.10	1.6	22.....	10.30	1.90	1.30	1.50	2.6
7.....	1.30	1.40	3.10	1.10	1.7	23.....	7.90	1.80	1.30	1.60	2.5
8.....	1.30	1.40	2.60	1.10	1.6	24.....	5.60	1.60	1.30	1.60	2.6
9.....	1.20	1.40	2.30	1.20	1.7	25.....	4.80	1.50	1.20	1.50	2.7
10.....	1.20	1.40	1.90	1.20	2.1	26.....	4.10	1.40	1.20	1.50	2.7
11.....	1.20	1.30	1.60	1.20	2.0	27.....	3.90	1.40	1.20	1.50	2.7
12.....	1.20	1.30	1.40	1.30	1.8	28.....	3.40	1.40	1.20	1.40	6.0
13.....	1.30	1.30	1.40	1.40	1.9	29.....	3.00	2.60	1.30	1.40	24.0
14.....	1.50	2.60	1.40	1.30	2.9	30.....	2.80	2.20	1.30	1.40	24.0
15.....	2.10	2.40	1.30	1.30	19.0	31.....	2.50		1.30		21.0
16.....	7.90	2.00	1.30	1.30	15.0						

Rating table for Cahaba River at Centerville, Alabama, for year 1901.

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
1.1	326	4.9	1,694	8.7	3,062	12.5	4,430
1.2	362	5.0	1,730	8.8	3,098	12.6	4,466
1.3	398	5.1	1,766	8.9	3,134	12.7	4,502
1.4	434	5.2	1,802	9.0	3,170	12.8	4,538
1.5	470	5.3	1,838	9.1	3,206	12.9	4,574
1.6	506	5.4	1,874	9.2	3,242	13.0	4,610
1.7	542	5.5	1,910	9.3	3,278	13.1	4,646
1.8	578	5.6	1,946	9.4	3,314	13.2	4,682
1.9	614	5.7	1,982	9.5	3,350	13.3	4,718
2.0	650	5.8	2,018	9.6	3,386	13.4	4,754
2.1	686	5.9	2,054	9.7	3,422	13.5	4,790
2.2	722	6.0	2,090	9.8	3,458	13.6	4,826
2.3	758	6.1	2,126	9.9	3,494	13.7	4,862
2.4	794	6.2	2,162	10.0	3,530	13.8	4,898
2.5	830	6.3	2,198	10.1	3,566	13.9	4,934
2.6	866	6.4	2,234	10.2	3,602	14.0	4,970
2.7	902	6.5	2,270	10.3	3,638	14.1	5,006
2.8	938	6.6	2,306	10.4	3,674	14.2	5,042
2.9	974	6.7	2,342	10.5	3,710	14.3	5,078
3.0	1,010	6.8	2,378	10.6	3,746	14.4	5,114
3.1	1,046	6.9	2,414	10.7	3,782	14.5	5,150
3.2	1,082	7.0	2,450	10.8	3,818	14.6	5,186
3.3	1,118	7.1	2,486	10.9	3,854	14.7	5,222
3.4	1,154	7.2	2,522	11.0	3,890	14.8	5,258
3.5	1,190	7.3	2,558	11.1	3,926	14.9	5,294
3.6	1,226	7.4	2,594	11.2	3,962	15.0	5,330
3.7	1,262	7.5	2,630	11.3	3,998	15.1	5,366
3.8	1,298	7.6	2,666	11.4	4,034	15.2	5,402
3.9	1,334	7.7	2,702	11.5	4,070	15.3	5,438
4.0	1,370	7.8	2,738	11.6	4,106	15.4	5,474
4.1	1,406	7.9	2,774	11.7	4,142	15.5	5,510
4.2	1,442	8.0	2,810	11.8	4,178	15.6	5,546
4.3	1,478	8.1	2,846	11.9	4,214	15.7	5,582
4.4	1,514	8.2	2,882	12.0	4,250	15.8	5,618
4.5	1,550	8.3	2,918	12.1	4,286	15.9	5,654
4.6	1,586	8.4	2,954	12.2	4,322	16.0	5,690
4.7	1,622	8.5	2,996	12.3	4,358		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Minimum monthly discharge of Cahaba River at Centerville, Ala., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

	1901.		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
August .....	362	33	4
September .....	398	36	3
October .....	362	33	5
November .....	326	30	3
December .....	398	36	1

NOTE.—To find the minimum net horse power available at a shoal on this stream near this station, for any month, multiply the total fall of the shoal by the "net H. P. per foot of fall" in this table for that month.

## 2. SURVEY OF CAHABA RIVER, ALABAMA.

The Cahaba River rises near Birmingham, Alabama, and flowing in a southerly direction, enters the Alabama River at a point just below Selma, Alabama.

From the published notes of a survey made by the Corps of Engineers, U. S. A., a profile has been made beginning at the southwestern boundary of Shelby county, and running down the river a distance of 110 miles to its mouth, in which distance there is a fall of 227 feet.

A United States Geological Survey hydrographic station has been established at Centerville, Ala., and measurements of discharge have also been made at Sydenton, near Birmingham, but the measurements and observations have not extended over a sufficient length of time to give accurate estimates of discharge at all seasons. The best that can be done at present is to form an estimate of the river discharge for ordinary low stage of water at Centerville, which is 344 second-feet, and add to or subtract from this amount for other points on the river. From measurements made on the same day at Centerville and at Harrell, near the mouth of the river, we estimate that the flow at Centerville is about one-third of that at the mouth of the river.

On the accompanying profile the stations are one mile apart, and are numbered from zero at the mouth of the river, up to 110 at the Shelby county line. In the following description of powers that can be developed these mile stations will be referred to as stations:

Power No. 1.—From the head of "Half Mile Rapids" at Station 108 there is a succession of shoals known as Half Mile, Long Island, Fish Trap, Ford, Reach, and Dry Creek Shoals, in which the aggregate fall is 30 feet in  $2\frac{1}{4}$  miles. There is also a fall of about 4 feet from the Shelby County line down to the head of Half Mile Shoal, making a total fall of 34 feet in four miles. This can be developed either by building a dam 34 feet high at the mouth of Dry Creek and backing the water to the Shelby County line, or by building a low dam near the head of the shoals, and a canal from it to a point opposite the mouth of Dry Creek. Such a development will give about 500 net horse-power, with an 80% turbine at ordinary low season. This power would be near Blocton, Ala.

Power No. 2.—By building a 15-foot dam at the head of "Baily Reach Rapids," near Station 101, and near the mouth of Big Ugly Creek, to back the water to the mouth of Persimmon branch near Station 104, and constructing from this point a canal along the river bank about four miles long, to a point opposite Station 97, at the mouth of Little Cahaba River, a practical head of 54 feet can be developed. This allows 8 feet for storage and grade, as the total fall is 62 feet. A 54-foot fall would produce about 800 net H. P.

The same power can be developed by building a high dam, lower down the river, and having the canal shorter. Or, the power can be divided into two separate powers. This power site is between River Bend and Cadle, in Bibb County.

Power No. 3.—From the mouth of Little Cahaba down to Station 88½ at the top of Centerville Shoals, there is a fall of ten feet in  $8\frac{1}{2}$  miles, and from the top of Centerville Shoals down to the foot of Centerville Shoals at Centerville, there is a fall of 13.6 feet in about  $1\frac{3}{4}$  miles. This power can be developed by a ten-foot dam at top of Centerville Shoals, and a canal from there to Centerville  $1\frac{1}{2}$  miles long. Allowing 2.6 feet for storage and canal grade, a head of 21 feet can be obtained which will give 650 net H. P.

It is probable that a much better method of development will be to erect a dam at Centerville 23.6 feet high to back the

water to the mouth of Little Cahaba. This will produce 732 net H. P., with storage. The incidental storage of such a dam would add largely to the amount and efficiency of the power. A plant running only 12 hours per day, and storing the water at night, could utilize 1,440 net H. P.

This power site is at Centerville, Alabama, on the M. & O. Railroad.

Power No. 4.—A 16-foot dam can be built at Shoal No. 9, Station 69½, in Perry County, just below the Bibb County line. This dam would back the water for 12 miles to Shoal No. 2, 4½ miles below Centerville. A 16-foot head will produce 670 H. P. without storage, or 1,340 H. P. by storing the water at night, and running only twelve hours per day. This dam site is about 17 miles below Centerville by river.

Power No. 5.—A 15-foot dam at "Blocks Cut-off," near Station 55, will back the water ten miles to the mouth of Taylor's Creek, and will produce 750 continuous, or 1,500 twelve-hour horse power.

Power No. 6.—At Shoal No. 24, Station 50, there is a fall of 9 feet in less than half a mile. A 14-foot dam at foot of this shoal, or a 5-foot dam at its head, and a short canal will develop a head—of 14 feet and realize 720 continuous, or 1,400 twelve-hour H. P.

This site is just above Burras Island, 8 or 10 miles northeast of Marion, Ala.

Power No. 7.—From Burras Island to Fikes Ferry there is a fall of 22 feet in a distance of 7 miles, 20 feet of which could probably be utilized by a dam at Fikes Ferry, producing 1,100 continuous, or 2,200 twelve-hour H. P. Fikes Ferry is near Marion, Ala.

In making the above statement of powers that can be developed, it has been assumed that there are suitable banks for dam sites. The system proposed, or some other system approximating to it, would not interfere with navigation improvements, as locks could be constructed at the dams.

## CHAPTER V.

### BLACK WARRIOR RIVER AND TRIBUTARIES

#### 1. TUSCALOOSA STATION ON BLACK WARRIOR RIVER.

This gage was placed in position by the United States Corps of Engineers in 1888. It is about three-fourths of a mile from the business center of Tuscaloosa, Alabama, and is reached by passing down Bridge street to the river, thence down the east bank 1,800 feet to the gage. It consists of an inclined timber, 2x6 inches, supported on posts and graduated by means of notches placed 1 foot vertically apart. The observer is W. S. Wyman, Jr., Tuscaloosa, Alabama. Mr. Wyman is observer for the Corps of Engineers, and has been kind enough to send weekly reports to this office. Observations are taken daily at 7 A. M. The area draining past this point is 4,900 square miles.

The bench marks are fixed, one on a willow 10 feet west of gage, 97.84 feet above Mobile datum, the other on a small hackberry 30 feet south of the upper end of the gage and 139.36 feet above Mobile datum. The current here is rather sluggish, being almost imperceptible at low stages. Both banks are of earth, and subject to overflow. Observations of gage heights have been obtained through the courtesy of Mr. R. C. McCalla, Jr., of the United States Engineers in charge of the Black Warrior River, from the time the gage was established until December 31, 1896. A measurement made by Mr. McCalla September 14, 1896, showed a gage height of -0.60 foot area, 1,022 square feet, mean velocity 0.16, discharge 164 second-feet.

The following list of measurements at the same place has been furnished by Mr. Horace Harding, C. E., United States assistant engineer, 2016 Quinlan avenue, Birmingham, Alabama. Velocities were obtained by means of rod floats reaching from the water surface to near the bottom. The highest flood occurred on April 8, 1892. The gage height was 62.5, the sectional area 33,600 square feet, and the estimated mean velocity 4.5 feet per second. This gave a discharge of 151,200 cubic feet per second. From this estimate and the following list of measurements a curve has been plotted and a rating table constructed, and this rating table applied to all gage heights observed. The estimates of discharge thus obtained are shown in diagrammatic form in Plate V. The highest discharges are merely approximations, but the discharges shown by the diagrams serve as a basis for comparison of the state of the river during the various years.

*List of Discharge Measurements made on Black Warrior River at  
Tuscaloosa, Alabama.*

No.	Date.	Gage height.	Discharge.	Remarks.
	1895.	<i>Feet.</i>	<i>Second-feet.</i>	
1	Dec. 17	1.10	617	Stationary.
2	Dec. 21	2.61	1,344	Do.
3	Dec. 24	3.60	1,733	Rising slowly.
	1896.			
4	Jan. 30	9.99	5,073	Falling 0.05 per hour.
5	Jan. 31	8.65	4,363	Do.
6	Feb. 26	8.25	4,360	Falling 0.01 per hour.
7	Feb. 28	7.27	3,657	Falling 0.02 per hour.
8	Feb. 29	6.92	3,522	Stationary.
9	Mar. 2	7.67	4,211	Do.
10	Mar. 3	7.28	3,632	Falling 0.03 per hour.
11	Mar. 6	6.94	4,558	Rising 0.15 per hour.
12	Mar. 24	24.85	13,550	Falling 0.12 per hour.
13	Apr. 10	9.71	5,331	Falling.
14	Apr. 11	8.89	4,755	Do.
15	Apr. 14	8.25	4,675	Rising.
16	Apr. 20	7.55	3,862	Falling.
17	Apr. 21	6.65	3,388	Do.
18	Apr. 22	5.96	2,940	Do.
19	Apr. 23	5.46	2,704	Do.
20	Apr. 24	5.88	3,158	Rising.
21	Apr. 27	5.68	3,049	Do.

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1889.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	15.00	23.50	18.80	8.50	8.80	2.30	2.40	3.70	0.75	2.40	0.90	6.00
2.....	19.50	20.50	16.50	9.80	14.50	2.90	3.10	3.60	7.30	1.95	1.05	5.65
3.....	18.00	17.50	25.00	10.30	13.20	2.75	4.40	3.00	6.40	1.55	1.85	5.30
4.....	16.00	14.80	31.50	11.60	11.00	2.50	5.10	2.95	5.30	1.30	1.90	4.80
5.....	25.00	13.00	29.40	10.50	8.50	2.30	6.25	2.90	6.20	1.15	3.85	4.50
6.....	33.00	11.50	26.50	9.50	7.00	2.10	6.85	3.70	9.30	1.05	3.65	4.20
7.....	33.50	10.40	23.00	8.40	5.80	1.90	6.45	3.45	14.90	.90	3.30	4.00
8.....	29.50	9.30	20.10	7.30	4.60	1.70	5.70	2.95	19.00	.70	3.00	3.80
9.....	26.80	8.30	17.50	6.80	4.30	2.00	4.75	2.60	14.00	.50	2.75	3.60
10.....	28.40	8.00	15.00	6.40	4.00	2.60	3.85	2.00	9.90	.40	2.55	3.40
11.....	29.00	7.80	12.80	6.30	3.80	2.85	3.05	1.75	5.50	.30	3.65	3.20
12.....	25.50	7.60	11.00	6.00	3.20	4.20	2.85	1.55	5.25	.30	4.30	2.95
13.....	22.50	7.30	10.00	5.60	3.00	3.40	2.45	3.85	4.15	.20	4.15	2.75
14.....	19.00	7.00	9.20	5.50	3.10	4.00	2.15	3.60	3.30	.15	3.75	2.70
15.....	16.00	7.30	8.80	8.00	3.20	3.45	1.95	3.35	2.70	.15	3.50	2.65
16.....	13.60	27.50	8.10	16.80	3.00	3.25	4.20	3.15	2.25	.15	4.20	2.60
17.....	29.00	49.00	7.20	16.70	2.80	3.05	7.35	3.80	2.00	.10	5.20	2.30
18.....	40.50	56.40	7.10	14.00	2.50	2.85	11.35	3.50	1.40	.10	12.45	2.20
19.....	38.50	56.60	11.00	12.00	2.35	2.65	11.50	3.20	1.40	.05	13.90	2.10
20.....	34.00	53.00	12.50	10.00	2.25	2.45	8.40	2.90	1.30	.05	16.90	2.05
21.....	30.30	47.00	12.30	8.90	2.05	2.25	4.60	2.40	1.20	.05	14.10	2.00
22.....	28.10	41.50	11.80	8.00	1.85	2.25	4.55	1.90	1.10	.00	11.70	2.00
23.....	26.00	36.50	10.80	7.00	1.65	2.45	4.50	1.50	1.00	.10	9.70	2.00
24.....	23.10	32.50	10.00	6.40	1.50	2.20	3.70	1.20	1.00	.30	7.90	2.00
25.....	21.00	28.50	9.80	6.30	1.30	2.00	3.15	.90	.95	.15	6.60	1.95
26.....	20.40	26.50	11.20	6.80	1.25	2.40	3.10	.75	2.25	.10	6.30	1.85
27.....	28.00	23.50	11.80	6.50	1.15	1.95	4.55	1.00	3.70	.30	6.10	1.95
28.....	33.50	21.40	12.30	6.20	1.05	1.55	4.05	.95	3.50	.35	5.90	1.80
29.....	33.80	11.00	11.00	5.80	.95	1.25	4.00	.90	3.25	.40	5.80	1.80
30.....	30.00	10.50	5.50	1.15	1.10	1.10	4.00	1.10	2.60	.90	6.30	3.00
31.....	27.00	9.80	1.10	1.10	1.10	1.10	3.80	1.05	1.00	1.00	.....	4.85

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1890.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	7.70	30.20	58.90	24.50	16.70	7.75	1.80	5.00	8.00	9.30	3.75	1.00
2.....	7.20	26.20	57.40	34.50	13.70	6.50	1.65	4.05	5.80	7.40	3.35	1.00
3.....	6.20	22.50	52.40	34.10	11.95	5.65	1.45	3.25	4.35	6.10	3.10	1.00
4.....	6.20	19.05	46.80	43.60	12.75	5.00	1.25	2.85	3.40	5.10	2.85	1.00
5.....	5.20	16.40	40.85	45.90	14.65	4.40	1.10	3.15	3.10	4.40	2.55	1.00
6.....	4.80	13.90	37.15	44.50	16.50	4.00	.80	3.35	3.85	3.80	2.40	1.00
7.....	4.50	12.10	35.40	38.70	15.00	5.20	.60	3.75	4.35	3.50	2.25	1.20
8.....	4.55	44.30	32.60	34.00	13.50	5.60	.50	4.55	4.80	3.45	2.15	10.05
9.....	5.10	53.95	30.50	29.95	11.95	5.40	.45	4.30	2.85	4.50	2.05	10.10
10.....	5.30	52.90	27.50	26.70	10.10	5.25	.30	4.30	3.20	4.45	2.00	8.95
11.....	5.60	47.50	25.00	23.45	9.30	5.00	.20	6.10	2.90	4.10	1.95	7.10
12.....	5.45	42.20	23.00	20.45	8.35	6.00	.15	6.40	3.45	3.80	1.75	5.80
13.....	5.30	37.20	20.75	17.45	8.35	6.20	.05	5.30	3.95	3.50	1.65	4.60
14.....	5.20	32.65	22.50	14.50	9.00	6.20	1.05	4.50	5.80	3.20	1.60	3.70
15.....	5.60	29.45	38.20	12.15	8.75	5.60	2.55	3.20	5.55	2.80	1.60	3.30
16.....	11.20	26.95	38.00	10.65	8.65	6.05	3.10	2.80	4.50	2.70	1.55	2.95
17.....	19.30	23.95	35.80	10.35	9.50	4.35	3.55	2.50	3.55	2.60	1.65	2.80
18.....	21.00	21.45	32.30	12.95	8.90	4.30	3.65	2.70	2.90	2.40	1.60	2.75
19.....	18.40	18.45	29.00	13.20	8.15	4.20	4.15	2.55	2.45	2.30	1.55	2.20
20.....	15.40	15.65	27.20	13.95	7.95	3.70	4.75	2.40	2.10	2.10	1.50	2.00
21.....	13.30	13.45	32.40	12.15	7.95	3.20	4.20	2.20	1.75	2.00	1.50	2.00
22.....	11.60	12.00	34.45	10.45	9.00	2.80	3.00	1.80	1.70	2.45	1.45	1.90
23.....	10.10	10.40	41.25	9.30	8.40	2.50	2.10	1.40	3.05	4.90	1.35	1.90
24.....	10.70	9.40	40.25	10.65	7.65	2.25	1.75	1.10	3.10	8.45	1.30	1.80
25.....	12.50	9.50	36.75	15.45	6.95	2.25	2.75	1.00	13.05	10.55	1.25	1.80
26.....	13.50	13.50	33.35	26.50	6.85	2.15	12.65	.95	23.90	9.60	1.20	2.30
27.....	12.80	35.20	29.70	28.45	9.60	2.55	13.40	.95	22.65	8.05	1.15	6.80
28.....	11.80	53.10	27.45	26.20	13.20	2.25	10.30	5.10	18.00	6.45	1.10	9.70
29.....	10.60	.....	24.55	23.35	13.55	2.05	9.30	9.55	14.70	5.30	1.05	10.20
30.....	17.60	.....	21.95	19.80	11.95	1.90	7.45	9.15	11.90	4.90	1.00	9.10
31.....	31.70	.....	18.80	.....	9.25	.....	6.50	10.45	.....	4.15	.....	7.30



*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1891.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	6.10	33.20	24.00	34.00	5.70	1.80	2.10	6.50	1.00	-0.40	-0.80	3.40
2.....	8.60	39.60	23.50	39.40	5.50	1.90	2.00	13.00	1.00	-.30	-.50	3.00
3.....	14.30	40.90	20.50	36.80	5.30	1.80	1.90	14.80	1.00	-.20	-.80	2.40
4.....	17.90	40.20	19.60	32.50	4.90	1.70	1.80	16.00	.80	-.20	-.80	6.20
5.....	15.20	39.30	17.20	28.50	4.50	1.50	1.50	13.00	.70	-.20	-.80	6.80
6.....	13.10	36.30	29.00	25.00	4.50	1.40	1.30	9.50	.60	& .20	-.80	21.50
7.....	11.10	37.00	53.00	22.20	4.20	1.40	1.40	7.40	.60	-.40	-.80	19.50
8.....	10.00	51.50	58.00	19.00	4.00	1.50	2.10	5.80	.60	-.50	-.80	20.00
9.....	9.50	51.50	60.40	16.00	3.90	2.10	7.60	5.40	.60	-.60	-.80	20.50
10.....	17.50	52.20	58.00	14.20	3.50	2.80	10.40	3.70	.60	-.70	+.70	17.00
11.....	20.90	53.50	54.00	17.20	3.20	3.50	8.20	3.10	.70	-.80	2.10	14.00
12.....	26.20	50.50	48.00	27.00	3.20	10.20	6.00	2.70	1.00	-.80	2.80	11.50
13.....	30.10	47.60	43.00	26.00	3.10	10.50	4.30	2.50	1.20	-.80	4.40	9.00
14.....	25.50	51.40	40.00	22.50	3.00	9.80	3.20	3.00	1.00	-.80	3.80	6.20
15.....	21.00	49.50	36.50	19.50	2.80	8.00	2.30	3.40	1.00	-.60	2.50	7.00
16.....	18.50	46.50	33.20	17.20	2.60	6.50	2.60	4.00	1.00	-.70	2.00	9.00
17.....	17.10	44.30	30.00	15.60	2.50	5.60	2.60	3.00	.60	-.70	1.80	11.00
18.....	17.70	41.00	28.00	16.40	2.50	6.30	2.90	2.60	.20	-.70	1.80	11.20
19.....	17.10	37.50	26.40	14.50	2.50	7.20	2.50	2.20	.10	-.60	1.50	10.00
20.....	15.60	35.00	24.00	13.40	2.50	7.00	2.20	2.00	.10	-.70	1.30	8.50
21.....	12.60	33.50	21.00	12.00	2.50	6.00	1.80	1.50	.10	-.70	1.40	7.00
22.....	20.20	39.50	19.90	11.00	2.70	5.50	1.60	1.50	.10	-.60	3.00	6.00
23.....	31.60	41.00	17.50	10.00	3.00	5.20	1.40	1.40	.10	-.60	8.00	6.00
24.....	31.80	39.00	15.00	9.00	3.00	4.80	1.80	1.30	.10	-.50	12.30	6.20
25.....	30.10	36.50	12.50	8.20	2.80	4.50	1.40	1.20	.00	-.50	13.30	7.50
26.....	30.70	33.00	12.10	7.60	2.60	4.20	2.60	1.10	.00	-.50	10.80	13.20
27.....	29.00	29.00	15.00	8.40	2.50	4.10	2.00	1.10	-.20	-.70	7.20	31.00
28.....	26.80	26.50	18.20	8.00	2.40	3.70	2.20	1.00	-.20	-.80	5.80	38.90
29.....	23.70	23.70	19.40	7.30	2.30	2.90	2.00	1.00	-.40	-.80	4.20	31.20
30.....	32.70	.....	18.00	6.50	2.10	2.50	2.10	1.10	-.50	-.80	3.80	27.00
31.....	33.00	.....	17.00	.....	2.00	.....	2.40	1.00	.....	-.80	.....	22.10

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1892.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	17.90	21.20	10.70	21.80	12.00	3.40	4.60	4.80	9.30	3.90	0.40	5.90
2.....	16.90	10.50	10.30	18.30	11.20	3.20	3.90	3.20	7.60	3.50	.40	5.60
3.....	18.50	9.40	9.70	15.50	9.90	3.50	3.40	7.00	6.50	3.30	1.20	5.30
4.....	13.70	8.50	8.80	15.20	8.80	3.70	3.00	7.40	5.60	3.10	1.60	4.90
5.....	14.40	7.80	8.00	12.00	7.20	2.50	2.90	6.60	4.90	2.80	1.60	6.30
6.....	14.60	7.40	7.80	11.60	6.50	4.70	3.50	5.40	4.90	2.50	1.60	7.50
7.....	13.00	7.80	8.00	56.30	5.90	5.00	5.20	4.80	4.80	2.40	2.00	7.50
8.....	11.60	7.50	16.50	63.20	5.80	4.90	11.00	3.90	6.40	2.20	3.90	12.20
9.....	10.80	9.00	26.80	62.20	5.20	4.10	26.70	3.60	6.20	2.10	4.60	15.00
10.....	10.50	16.00	28.50	58.00	4.90	4.00	43.50	3.50	5.40	2.00	8.90	13.20
11.....	25.70	13.00	26.70	52.30	4.90	3.80	46.20	3.00	6.10	2.00	10.90	11.00
12.....	34.80	11.30	22.00	45.40	4.80	3.50	41.40	3.00	3.90	2.00	8.30	9.40
13.....	53.00	10.80	18.00	40.70	4.70	3.30	38.30	3.30	3.40	2.00	7.60	8.00
14.....	57.40	9.50	15.80	36.50	4.60	2.80	37.50	3.10	3.30	1.90	5.80	7.40
15.....	55.90	9.00	13.80	32.80	4.50	2.50	34.80	3.90	4.00	1.90	5.00	8.60
16.....	51.70	10.00	13.30	29.50	4.00	2.40	32.80	3.50	5.10	1.90	4.50	10.30
17.....	45.00	11.80	11.20	27.00	3.70	2.00	41.40	3.50	4.90	1.80	4.00	13.10
18.....	40.10	11.00	12.90	24.50	3.60	1.80	38.00	6.00	4.30	1.70	4.20	26.70
19.....	36.60	9.90	24.00	22.40	4.30	2.10	33.00	9.50	5.00	1.00	4.40	28.40
20.....	41.50	10.50	25.50	20.20	4.60	3.90	29.00	11.50	18.30	1.00	4.70	28.40
21.....	41.00	13.50	22.90	18.10	6.50	7.40	28.30	9.50	23.90	.90	5.00	36.50
22.....	36.80	18.00	20.00	15.90	6.30	10.80	29.30	7.40	20.90	.90	4.80	35.80
23.....	34.40	23.90	18.30	13.80	6.00	10.70	25.10	10.20	17.00	.90	4.80	31.50
24.....	31.00	21.20	22.00	12.30	6.00	8.40	21.50	12.30	13.30	1.00	4.20	27.00
25.....	28.50	18.50	29.00	11.30	4.80	7.80	18.80	14.00	10.50	1.00	3.80	23.00
26.....	26.00	16.00	32.00	10.50	3.50	7.90	15.80	14.20	8.00	.90	3.50	19.50
27.....	23.80	14.00	35.80	8.80	4.70	7.20	13.00	13.20	6.10	.80	3.30	16.00
28.....	21.50	12.50	34.00	7.90	4.00	6.10	10.50	11.50	5.00	.70	3.20	13.60
29.....	19.00	11.50	30.50	7.70	3.80	5.20	7.80	12.00	4.90	.70	3.90	11.40
30.....	16.80	.....	26.80	10.50	3.50	5.40	6.20	11.50	4.30	.50	6.90	9.80
31.....	14.00	.....	23.90	.....	3.50	.....	5.50	10.50	.....	.50	.....	8.50

## Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1893.

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.	8.20	18.10	23.00	9.40	24.60	12.60	2.50	0.60	0.30	1.10	0.40	1.50
2.	8.60	15.90	21.70	9.30	21.20	33.50	2.30	.70	.20	1.00	.40	1.50
3.	9.80	14.00	19.70	9.00	30.00	49.60	2.80	1.00	.10	1.20	.40	1.80
4.	10.70	12.80	20.50	8.70	51.20	46.00	3.60	1.20	.00	1.40	.40	2.80
5.	10.10	12.50	23.00	8.00	52.20	39.00	3.60	1.30	.10	1.30	.40	3.50
6.	9.00	11.80	24.00	8.70	48.00	37.70	3.20	1.20	.10	1.20	.40	4.20
7.	8.40	11.20	22.60	9.30	42.90	39.90	2.70	1.30	.00	1.20	.40	4.10
8.	7.90	10.80	20.10	9.00	40.40	39.30	2.40	1.20	+	.20	1.20	.40
9.	7.00	10.60	20.00	8.30	37.40	34.00	2.10	1.00	1.40	1.00	.40	3.00
10.	6.70	10.20	22.00	7.60	34.30	29.20	1.80	.90	2.00	.90	.60	2.80
11.	6.20	12.00	21.80	7.00	30.80	25.10	1.60	.90	2.10	.80	.60	2.30
12.	6.50	23.90	20.60	6.40	27.60	21.20	1.60	.80	2.20	.80	.60	1.90
13.	7.80	28.30	19.30	6.10	24.40	17.40	2.20	.90	3.20	.80	.60	1.70
14.	9.00	27.00	18.10	5.80	21.90	14.70	2.60	3.60	4.30	.70	.60	1.60
15.	9.60	25.90	16.30	23.00	19.10	11.60	2.40	4.90	4.30	.70	.60	1.50
16.	11.40	52.20	14.70	27.10	16.20	9.60	2.60	5.10	3.90	.60	.60	1.70
17.	12.20	55.60	13.20	24.00	14.00	7.60	1.70	4.70	3.30	.50	.60	2.20
18.	11.80	64.70	12.20	20.00	11.90	6.70	1.40	3.70	2.50	.50	.60	2.40
19.	12.00	61.40	11.30	16.40	11.00	6.60	1.30	2.80	2.10	.50	.60	2.30
20.	12.40	46.50	10.70	13.90	8.40	6.90	1.20	2.20	1.80	.50	.60	2.10
21.	11.80	41.80	9.90	12.00	6.90	6.90	1.20	1.70	1.40	.50	.70	2.50
22.	10.90	37.90	9.30	11.20	6.20	6.60	1.30	1.50	1.30	.50	1.00	2.40
23.	11.20	34.50	8.80	10.70	5.60	6.40	1.60	1.30	1.20	.40	1.00	2.30
24.	12.90	31.30	12.30	9.90	5.10	5.70	1.70	1.10	1.00	.40	1.10	2.10
25.	15.90	28.40	22.20	8.90	4.80	4.90	1.50	1.00	1.30	.40	1.00	1.90
26.	19.20	25.90	22.60	7.00	4.50	4.40	1.30	.90	1.20	.40	.90	1.70
27.	22.00	23.80	20.00	10.50	4.30	4.00	1.20	.80	1.20	.40	1.00	1.60
28.	23.30	23.00	17.00	32.50	5.10	3.50	.90	.80	1.10	.40	1.70	1.50
29.	23.10	.....	14.40	33.90	6.10	3.10	.90	.60	1.10	.40	1.70	1.40
30.	22.10	.....	12.40	29.00	12.90	2.80	.80	.50	1.10	.40	1.60	3.70
31.	20.40	.....	10.90	.....	14.10	.....	.80	.20	.....	.40	.....	7.60

## Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1894.

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.	10.30	8.50	27.00	11.40	6.50	1.70	2.50	1.20	5.80	0.55	-0.05	-.05
2.	9.60	7.80	25.60	14.10	5.90	1.60	1.80	1.30	4.60	.45	-.10	-.05
3.	7.90	7.30	23.60	23.50	5.90	1.50	1.35	1.40	3.60	.40	-.20	-.00
4.	6.20	7.90	21.00	24.00	5.80	1.35	1.10	1.95	2.95	.32	-.25	.00
5.	5.40	16.90	18.20	22.20	5.60	1.33	.80	2.30	2.45	.25	-.30	.00
6.	6.40	22.60	15.90	20.60	5.20	1.20	.60	2.40	2.05	.20	-.30	.00
7.	16.80	20.40	14.80	18.90	5.10	1.20	1.00	2.00	1.85	.15	-.30	.00
8.	22.60	17.80	14.40	16.30	4.90	1.10	1.80	1.60	1.75	.00	.30	+.05
9.	19.70	17.30	13.80	14.70	4.50	.96	.70	1.80	1.65	-.10	.30	.05
10.	26.90	26.00	12.60	15.70	4.10	.90	.70	1.10	1.60	-.20	.30	.50
11.	34.50	27.80	11.60	25.00	3.70	.80	.60	.80	1.85	-.25	.30	2.10
12.	35.80	25.40	11.50	24.00	3.90	.80	.60	.70	3.40	-.30	.30	5.70
13.	30.90	29.90	16.50	22.70	4.30	.70	.50	.60	5.60	-.30	.30	10.40
14.	25.80	32.10	17.30	19.50	7.00	.60	.50	.50	7.50	-.30	.10	11.20
15.	22.20	29.70	15.80	16.30	6.20	.60	.60	.35	6.70	-.30	.00	8.90
16.	23.60	26.00	15.60	15.50	5.70	.60	.70	.60	4.20	-.30	+.05	6.40
17.	24.80	22.30	33.10	16.00	5.10	.55	.75	.55	3.25	-.30	.05	4.90
18.	22.40	19.10	36.70	17.00	5.00	.50	.70	.45	3.50	-.30	.05	3.90
19.	19.30	16.80	33.70	25.30	4.70	.60	.70	.40	3.25	-.30	.00	3.20
20.	15.90	16.00	29.50	24.80	4.20	.70	.60	.55	3.25	-.30	.10	2.90
21.	14.10	16.90	27.80	24.00	3.60	.85	.50	1.00	4.10	-.30	.10	2.60
22.	18.60	17.90	29.30	20.40	3.10	.80	.90	1.35	3.60	-.30	.10	2.20
23.	21.30	17.00	29.60	17.30	3.00	.95	1.30	3.80	2.90	-.30	.10	2.00
24.	19.90	16.10	28.00	14.20	2.60	1.10	1.35	5.30	2.20	-.35	.00	1.80
25.	17.60	17.30	27.80	12.20	2.40	1.00	1.60	9.20	1.75	-.40	.00	1.70
26.	15.50	29.30	25.30	10.50	2.30	1.05	1.50	16.00	1.45	-.40	.00	2.00
27.	14.00	31.00	22.10	9.80	2.10	1.40	1.40	20.40	1.20	-.45	.05	7.70
28.	12.40	28.80	19.00	8.70	2.10	1.70	1.30	16.00	1.05	-.45	.05	9.70
29.	11.10	.....	16.10	7.90	2.00	2.35	.20	11.80	.90	-.45	.05	8.40
30.	10.00	.....	14.30	7.00	1.90	2.60	1.10	8.30	.70	-.20	.05	7.20
31.	9.10	.....	12.70	.....	1.90	.....	1.20	6.80	.....	-.05	-.05	6.50

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1895.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	6.10	24.10	8.60	16.20	15.20	7.00	4.80	1.80	1.90	0.10	-0.04	0.75
2.....	6.40	21.50	13.20	14.50	13.00	5.90	4.50	1.50	1.80	.00	+ .07	.95
3.....	6.80	21.40	35.30	12.60	11.00	5.10	5.40	1.30	1.80	.07	.12	1.05
4.....	9.00	21.40	38.70	11.00	9.40	4.60	7.60	1.20	1.70	.10	.09	1.00
5.....	8.80	19.90	32.40	10.20	8.30	4.20	17.40	1.20	1.90	.10	.11	.90
6.....	8.00	17.90	27.80	8.40	7.40	6.70	17.70	1.20	2.60	.10	.25	1.30
7.....	7.40	16.50	23.80	7.90	7.00	5.70	18.40	1.10	3.05	.10	.31	1.50
8.....	35.00	16.80	20.60	20.50	8.20	5.10	15.70	1.00	3.70	.00	.30	1.45
9.....	50.60	16.90	20.10	24.00	13.50	4.30	13.10	.90	3.40	.03	.28	1.90
10.....	49.30	15.80	19.30	21.20	19.70	3.80	11.10	.97	2.90	.07	.70	1.80
11.....	46.10	14.60	17.50	18.00	23.30	3.10	9.80	1.40	2.40	.10	1.20	1.45
12.....	40.10	14.10	17.20	15.10	22.00	2.70	7.90	1.50	2.20	.14	1.10	1.40
13.....	36.00	13.90	18.40	12.60	18.50	2.40	6.50	1.40	2.10	.14	1.18	1.40
14.....	29.80	12.80	24.90	11.00	15.20	2.20	6.30	1.90	2.00	+ .06	1.35	1.37
15.....	25.70	11.60	37.50	9.50	12.20	2.00	5.40	1.10	1.80	.04	1.35	1.23
16.....	23.40	10.50	47.40	8.60	9.70	1.00	5.60	1.30	1.60	.06	1.28	1.23
17.....	31.20	9.40	62.00	10.00	8.00	2.70	6.90	1.40	1.40	.18	1.20	1.12
18.....	32.90	9.00	47.30	15.90	6.90	3.30	6.80	2.00	1.40	.27	1.05	1.05
19.....	29.20	9.00	42.10	15.80	6.30	4.20	6.10	4.00	1.30	.31	.88	1.02
20.....	25.80	9.50	38.80	14.00	6.90	4.90	4.60	4.30	1.00	.37	.82	1.87
21.....	23.10	10.20	48.70	12.00	5.30	4.70	4.00	3.60	1.20	.42	.70	2.55
22.....	21.60	10.90	51.30	10.40	4.80	4.30	3.60	2.80	1.50	.45	.60	2.60
23.....	21.60	11.40	47.60	8.90	4.30	4.10	3.40	2.40	1.30	.50	.50	3.08
24.....	19.80	11.30	42.10	8.00	4.10	3.70	3.30	4.30	1.10	.35	.75	3.65
25.....	17.40	10.90	37.30	7.20	4.00	3.40	3.60	4.90	.80	.33	.60	3.31
26.....	16.70	10.30	32.80	7.00	4.30	3.00	3.70	4.00	.60	.53	.38	4.50
27.....	20.00	9.70	29.10	7.10	5.60	4.40	3.30	3.20	.35	.70	.64	10.40
28.....	21.20	9.00	26.10	12.50	11.00	5.40	2.90	2.40	.10	.68	.65	21.02
29.....	22.00	13.30	23.60	15.00	13.30	6.00	2.40	2.30	.50	.67	.65	16.88
30.....	27.40	21.10	16.40	11.20	5.40	2.00	2.00	2.10	.30	.50	.60	13.10
31.....	27.10	18.60	8.90	8.90	2.10	2.00	2.10	2.00	.32	.32	.32	11.67

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1896.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	11.41	7.92	6.91	10.61	18.24	4.92	1.94	.53	.38	.52	.78	7.70
2.....	10.26	9.51	7.66	14.78	33.30	4.28	1.71	.80	.25	.62	.77	6.32
3.....	8.90	21.98	7.41	23.00	37.18	3.50	1.44	1.34	.15	.66	.42	4.63
4.....	7.60	33.12	6.74	28.50	30.88	5.44	1.48	.89	.05	.20	.39	3.40
5.....	6.55	30.02	6.14	19.85	26.40	5.24	1.80	.54	.06	.78	.33	2.60
6.....	5.74	30.75	6.64	16.70	21.93	4.30	1.15	.42	.15	.74	.19	2.05
7.....	5.09	35.92	10.18	14.76	17.88	3.62	1.32	.50	.23	.68	.19	1.70
8.....	5.24	35.08	12.04	12.55	13.95	3.27	2.20	.60	.30	.40	.17	1.47
9.....	5.76	36.21	14.89	11.20	10.80	6.60	8.40	.68	.37	.09	.18	1.21
10.....	6.46	36.52	13.69	10.02	8.60	8.18	6.98	.64	.44	.15	.21	1.10
11.....	6.58	33.65	12.45	9.04	7.11	18.39	5.37	.55	.44	.28	.30	.89
12.....	6.75	29.45	13.97	8.33	6.09	15.13	3.95	.45	.46	.28	.20	.84
13.....	5.79	25.97	16.15	7.62	5.34	10.77	2.99	.38	.50	.48	.22	.80
14.....	5.26	27.41	15.35	7.85	4.77	7.66	2.45	.35	.60	.49	.11	.75
15.....	4.80	33.25	13.56	11.65	6.13	5.60	2.16	.31	.59	.46	.10	1.20
16.....	4.87	31.02	13.86	13.90	5.45	4.33	2.13	.26	.59	.43	.01	1.25
17.....	6.03	27.30	22.30	12.35	4.65	4.74	2.15	.13	.60	.60	+ .26	1.25
18.....	8.61	23.65	27.75	10.57	3.94	4.20	2.42	.40	.60	.63	.46	1.60
19.....	9.14	20.09	29.70	9.03	3.44	6.18	2.45	.50	.60	.78	.52	1.90
20.....	8.53	17.00	37.68	7.81	3.05	6.00	2.94	.40	.61	.82	.45	1.70
21.....	7.87	14.45	37.92	6.88	2.91	5.32	2.16	.34	.64	.82	.39	1.50
22.....	8.85	12.20	33.55	6.15	2.77	4.91	1.90	.29	.64	.84	.38	1.33
23.....	22.47	10.39	29.12	5.56	2.87	4.46	1.71	.22	.36	.84	.38	1.20
24.....	29.26	9.13	25.85	5.67	2.90	4.19	2.06	1.06	.45	.78	.37	.88
25.....	26.52	8.60	23.54	5.76	2.87	4.04	2.53	.95	.61	.80	.87	.92
26.....	22.44	8.35	21.28	5.30	2.97	3.37	2.17	.79	.64	.82	.37	.77
27.....	18.55	7.90	18.79	5.26	3.65	2.85	1.70	.77	.71	.82	.98	.71
28.....	14.92	7.40	16.35	10.40	3.59	2.69	1.28	1.35	.76	.78	.09	.59
29.....	12.14	7.00	14.30	16.06	6.59	2.54	.94	1.30	.67	.80	.33	.62
30.....	10.18	12.69	14.18	6.50	2.21	.76	.95	.55	.80	1.00	.48	.48
31.....	8.88	11.40	5.78	5.78	.64	.60	.60	.60	.78	.78	.78	.48

The following discharge measurement was made by B. M. Hall in 1897, at Tuscaloosa, Ala.:

January 12, gage height, 1.70 feet; discharge, 829 second-feet.

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1897.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	0.34	3.90	11.14	13.90	9.51	1.83	-0.15	1.36	0.60	-1.65	-1.39	-1.28
2.....	.47	6.00	9.57	15.28	9.95	1.75	-.18	1.08	.96	-1.71	-1.30	-1.12
3.....	.40	11.50	8.58	22.20	9.22	1.70	-.20	.87	1.02	-1.72	-1.29	-.48
4.....	.90	12.60	8.72	21.11	7.66	2.01	-.18	.62	.92	-1.75	-1.31	+1.29
5.....	1.24	11.70	10.14	22.00	6.23	1.98	-.05	.50	.76	-1.79	-1.33	13.10
6.....	1.11	12.37	16.33	26.95	5.36	2.65	1.63	.40	.62	-1.86	-1.28	14.24
7.....	1.40	16.24	51.42	25.32	4.67	3.44	3.40	.25	.51	-1.79	-1.27	10.72
8.....	2.60	18.70	54.77	22.10	4.20	2.37	3.90	.24	.47	-1.85	-1.27	7.89
9.....	2.65	21.04	51.59	21.30	3.87	2.25	3.71	1.10	.42	-1.88	-1.17	5.12
10.....	2.30	19.80	44.69	29.27	3.50	1.95	3.05	2.10	.36	-1.90	-1.13	3.72
11.....	2.03	17.90	40.54	29.57	3.20	1.60	2.53	3.26	.29	-1.90	-1.10	8.05
12.....	1.76	23.42	42.53	25.48	3.64	1.39	2.42	3.22	.23	-1.88	-1.14	2.70
13.....	1.52	25.90	48.70	21.60	11.40	1.24	2.16	2.73	.14	-1.89	-1.17	2.56
14.....	1.33	23.84	50.96	18.10	20.36	1.11	1.86	2.27	.11	-1.92	-1.25	3.05
15.....	1.23	20.30	48.57	16.32	20.46	.95	1.46	1.63	.10	-1.92	-1.25	3.54
16.....	8.23	16.96	45.20	18.43	16.59	.85	1.16	1.28	.06	-1.92	-1.27	4.12
17.....	9.70	14.04	47.21	18.33	12.68	.75	.97	1.00	.01	-1.90	-1.33	4.10
18.....	13.10	11.72	46.72	15.92	9.77	.53	1.50	.73	-.06	-1.90	-1.35	3.78
19.....	19.35	9.97	42.90	13.66	7.73	1.30	3.50	.52	-.36	-1.90	-1.36	3.50
20.....	18.70	8.77	42.57	11.86	6.35	1.40	12.50	1.53	-.75	-1.88	-1.36	3.82
21.....	17.43	8.08	44.54	10.45	5.37	1.11	14.50	1.78	-.96	-1.88	-1.36	6.70
22.....	18.64	8.99	41.50	9.24	4.66	.80	11.30	1.58	-1.07	-1.77	-1.37	10.58
23.....	16.52	11.00	37.70	8.15	4.13	.55	8.42	1.33	-1.17	-1.64	-1.42	31.00
24.....	13.30	20.20	35.66	7.37	3.70	.33	6.64	1.08	-1.26	-1.58	-1.44	29.96
25.....	10.60	21.24	32.40	6.80	3.45	.23	4.77	1.27	-1.24	-1.63	-1.40	24.08
26.....	8.60	18.97	28.86	6.37	3.13	.16	3.46	.97	-1.36	-1.63	-1.36	18.97
27.....	7.20	16.18	25.60	5.90	2.90	.12	2.72	.80	-1.41	-1.61	-1.36	15.67
28.....	6.00	13.31	23.15	5.49	2.60	.03	2.92	.61	-1.44	-1.64	-1.28	13.10
29.....	5.10	.....	20.33	5.10	2.28	.00	2.15	.54	-1.50	-1.61	-1.29	11.00
30.....	4.42	.....	17.52	5.90	2.09	-.08	1.80	.46	-1.55	-1.63	-1.24	9.38
31.....	3.80	.....	14.98	.....	2.00	.....	1.62	.31	.....	-1.63	.....	8.00

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1898.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	7.10	21.68	3.67	25.50	11.34	0.23	0.66	2.45	1.20	-0.60	0.90	5.40
2.....	6.28	18.30	3.78	22.40	9.82	.08	.43	1.95	.82	-.70	.70	5.50
3.....	5.52	14.90	4.03	18.50	8.67	.23	.30	2.45	.70	-.80	.60	5.40
4.....	4.96	12.12	4.05	15.31	7.68	.27	.53	2.80	.30	-.90	.40	5.00
5.....	4.41	10.15	3.98	20.82	6.88	.27	.16	2.96	.00	-1.00	.00	4.70
6.....	4.17	9.42	3.90	38.70	6.20	.15	.10	3.90	-.10	-.90	.30	4.50
7.....	4.13	8.90	3.57	38.55	5.59	.02	-.18	3.40	-.30	-.90	.50	5.00
8.....	4.18	8.43	3.33	32.83	5.00	-.10	-.04	3.10	-.20	-.70	.40	5.00
9.....	4.17	7.84	3.12	27.70	4.50	-.29	-.04	3.98	.00	-.30	.40	4.70
10.....	3.97	7.30	3.00	23.04	4.20	-.41	+.10	6.20	.20	+2.00	.50	4.20
11.....	4.00	6.68	2.90	19.50	3.91	-.54	.08	12.30	.40	3.30	.50	3.70
12.....	4.18	6.30	2.83	17.03	3.50	-.62	-.07	14.10	.20	2.60	.40	3.30
13.....	6.70	6.20	2.80	15.01	3.21	-.71	-.10	10.60	.00	2.00	.50	3.00
14.....	9.70	6.08	3.00	12.93	2.88	-.62	-.07	7.30	.30	1.40	.80	2.80
15.....	11.97	5.86	4.80	11.65	2.58	.53	-.11	5.00	.30	1.00	1.10	2.60
16.....	11.71	5.40	8.00	10.78	2.30	.38	-.07	3.50	.40	.80	1.30	2.50
17.....	15.00	5.00	15.40	9.60	2.08	.48	-.14	2.62	.60	.40	1.50	2.40
18.....	15.53	4.90	14.10	8.63	1.97	.56	.00	1.91	.70	.80	1.60	2.10
19.....	14.32	4.83	11.92	8.15	1.64	.30	1.24	1.50	.70	.80	1.90	4.40
20.....	24.50	4.84	10.08	27.80	1.43	-.21	1.30	1.46	.80	1.20	2.20	18.80
21.....	33.54	4.70	8.82	33.11	1.23	.00	1.12	1.12	-.90	3.10	2.50	23.90
22.....	31.42	4.48	7.80	28.45	1.07	.07	.88	1.26	.50	3.80	4.00	21.30
23.....	28.50	4.30	6.96	23.77	1.00	.18	.47	1.20	.60	4.10	8.60	17.40
24.....	30.38	4.00	6.30	22.72	.93	.30	1.08	.90	.70	4.80	11.70	13.30
25.....	30.12	3.88	6.00	24.04	.78	.17	1.95	.50	.80	4.30	11.60	10.50
26.....	42.50	3.64	5.90	22.62	.57	.06	2.50	1.10	-.90	3.90	9.50	8.60
27.....	43.48	3.50	5.53	19.67	.43	1.30	2.00	1.02	-.90	2.90	7.40	7.30
28.....	39.41	3.72	5.12	17.12	.43	1.10	1.80	.98	-.80	2.30	5.90	6.20
29.....	33.80	.....	5.23	15.05	.52	.46	2.03	1.30	-.80	2.00	5.50	5.60
30.....	28.50	.....	13.40	13.03	.44	.68	2.55	1.52	-.60	1.40	5.30	5.00
31.....	24.90	.....	25.68	.....	.30	.....	2.86	1.56	.....	1.10	.....	4.70

The following measurements were made by B. M. Hall, and Prof. George S. Wilkins, of the Alabama University, in 1899:

*Measurements of Black Warrior River at Tuscaloosa, Ala.*

Date.	Gage height.	Discharge.	Date.	Gage height.	Discharge.
February 21.....	19.36	12,855	March 4.....	23.70	12,609
February 21.....	19.25	12,640	March 14.....	31.18	36,653
February 24.....	22.85	16,216	March 14.....	34.37	40,331
February 28.....	39.47	48,010	March 17.....	59.50	119,533
March 1.....	35.50	24,988	March 18.....	55.40	86,410
March 2.....	30.35	18,052	March 23.....	40.30	23,911

## Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1899.

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	4.40	26.10	37.70	30.30	7.90	2.00	0.10	6.50	1.20	-1.50	-1.03	4.33
2.....	4.60	29.50	32.00	29.20	7.00	1.50	.80	4.90	.70	-1.61	-1.02	3.50
3.....	4.60	27.00	27.70	24.80	6.30	1.20	.30	3.40	.62	-1.80	-1.03	2.77
4.....	4.70	29.00	24.10	22.00	5.80	1.00	.70	2.70	.23	-1.79	.99	2.03
5.....	4.70	45.50	21.06	23.20	5.30	1.00	.10	2.00	.02	-1.51	-1.00	1.50
6.....	11.20	50.60	20.80	22.60	4.80	.90	.30	1.90	.17	-1.48	-1.02	1.22
7.....	42.50	61.40	19.60	23.30	4.40	1.00	.20	1.80	.38	-1.46	-1.03	.98
8.....	49.30	61.70	16.90	33.90	4.40	.80	.30	1.30	.40	-1.34	-1.08	.88
9.....	46.60	48.60	14.40	34.00	7.60	.40	.40	1.20	.46	-1.30	-1.03	.58
10.....	40.40	43.10	12.90	39.80	6.50	.40	.40	1.10	.68	.96	-1.01	.60
11.....	33.70	37.80	11.90	27.00	5.10	.50	.40	2.00	.73	.78	-1.01	2.20
12.....	31.90	32.80	11.39	23.60	4.30	.50	.50	1.70	.79	.72	.98	23.50
13.....	28.00	28.80	10.00	20.10	4.20	.50	.60	1.30	.71	.70	.96	29.53
14.....	25.00	25.70	23.80	17.20	4.30	.50	.60	.90	.73	.83	.98	35.71
15.....	22.20	22.90	44.50	14.90	4.70	.50	.70	.60	.78	.94	.59	26.50
16.....	20.00	21.60	58.30	13.00	4.20	.70	.70	.40	.83	-1.03	-1.00	20.63
17.....	19.60	19.90	60.30	11.60	3.70	.60	.70	.30	.98	-1.07	.97	15.71
18.....	20.10	20.10	57.70	10.50	3.30	.40	.70	.30	.88	-1.12	.97	10.83
19.....	18.60	20.80	52.40	9.80	2.80	.30	.70	.60	.90	-1.16	.95	8.02
20.....	16.10	20.60	48.30	9.60	2.70	.20	.70	.60	.82	-1.12	.95	8.09
21.....	14.00	19.60	46.80	9.50	3.70	.10	.60	.70	.94	-1.10	.92	9.63
22.....	12.20	18.50	41.60	8.70	3.80	.40	.50	1.50	.99	.84	.89	10.80
23.....	11.00	22.70	36.80	8.60	3.30	.10	.20	2.60	-1.03	.73	.38	10.63
24.....	11.00	23.10	33.00	11.80	3.10	.10	.60	2.50	-1.06	.71	.11	22.01
25.....	20.30	20.90	29.50	13.60	2.70	.60	4.90	2.60	-1.04	.88	.88	29.04
26.....	29.30	18.50	26.50	13.20	2.30	.20	7.60	2.30	-1.05	.94	2.60	25.91
27.....	26.20	23.50	24.20	12.60	2.00	.20	7.40	2.10	-1.04	-1.02	4.50	20.98
28.....	22.50	39.00	22.30	11.40	1.75	.20	7.90	1.90	-1.18	.60	10.40	17.09
29.....	18.60	.....	21.10	10.00	1.50	.20	9.50	1.60	-1.28	.73	9.40	15.00
30.....	15.70	.....	19.30	8.90	1.60	.10	9.30	1.50	-1.38	.96	6.67	14.62
31.....	14.80	.....	18.10	.....	2.50	.....	8.50	1.10	.....	-1.03	.....	12.51

## Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1900.

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	10.90	6.92	18.14	21.55	21.23	4.24	41.00	9.85	5.50	0.76	4.20	12.50
2.....	9.48	6.40	25.44	18.60	18.95	5.30	33.88	8.00	5.00	.80	5.10	9.65
3.....	8.32	5.00	26.00	15.80	16.33	13.95	32.27	6.50	4.07	.60	5.10	8.50
4.....	7.10	5.71	23.30	13.60	13.82	15.16	32.13	5.15	4.30	.25	6.55	7.90
5.....	6.18	6.40	20.29	12.27	11.60	20.98	30.96	4.40	3.58	.40	5.05	8.20
6.....	5.53	9.58	17.43	12.17	9.60	21.50	27.31	3.80	2.80	.40	4.46	10.20
7.....	5.12	11.50	15.34	11.44	8.20	20.92	23.70	3.35	2.30	.80	3.90	10.45
8.....	4.84	10.53	27.58	10.29	7.28	21.75	20.80	3.00	1.90	1.80	3.50	9.65
9.....	4.60	12.23	39.00	9.97	6.80	28.50	18.45	2.50	1.60	3.90	3.10	8.5
10.....	4.50	20.60	36.03	10.58	6.50	31.70	15.96	2.30	1.50	6.05	2.90	8.15
11.....	7.12	23.00	31.63	26.35	6.12	24.73	12.96	2.10	1.10	6.10	.70	7.60
12.....	31.63	20.64	27.34	52.79	5.90	19.60	11.05	2.00	.90	14.30	2.60	6.90
13.....	31.80	41.37	23.78	53.40	5.48	15.95	10.60	1.75	.80	22.50	2.40	6.45
14.....	28.18	47.96	20.43	48.69	4.98	15.60	12.90	1.50	1.40	21.85	2.25	6.40
15.....	24.09	45.73	17.40	42.30	4.50	19.23	10.35	1.40	5.45	16.40	2.20	6.65
16.....	20.12	40.23	16.63	37.10	3.85	29.15	8.50	1.50	10.65	11.60	2.10	6.45
17.....	16.50	34.74	21.18	63.00	3.65	28.85	7.00	1.48	8.95	8.20	2.05	6.10
18.....	14.28	29.75	20.28	64.05	3.50	25.33	6.00	1.40	6.34	6.13	2.00	5.75
19.....	15.80	25.83	18.80	62.17	3.52	24.51	5.60	3.00	4.44	5.10	1.95	5.30
20.....	25.00	22.43	45.10	59.35	3.39	30.10	6.35	4.40	3.35	4.30	3.90	5.45
21.....	32.60	19.90	51.00	56.10	3.65	27.80	8.50	3.75	2.60	3.60	8.40	6.30
22.....	29.44	23.58	47.98	51.71	3.50	25.38	8.00	3.00	2.20	3.15	10.25	8.35
23.....	24.54	26.50	42.40	46.20	3.35	24.65	8.10	2.50	1.87	4.60	12.60	11.90
24.....	20.43	24.20	38.24	41.88	4.40	60.00	8.70	2.55	1.65	7.30	11.00	17.00
25.....	17.12	22.38	35.41	37.94	7.55	58.25	7.40	2.10	1.60	10.40	9.45	18.70
26.....	14.12	21.00	36.78	33.94	7.68	56.35	6.80	4.48	1.45	8.55	15.90	17.00
27.....	12.14	18.63	35.33	30.89	6.70	52.90	9.20	4.30	1.30	6.75	22.20	14.0
28.....	10.47	16.52	31.80	28.15	6.30	49.05	16.20	3.25	1.05	5.40	21.00	12.50
29.....	9.10	.....	28.85	25.73	4.25	48.77	13.50	2.40	.90	4.70	17.35	10.95
30.....	8.27	.....	26.25	23.50	3.60	44.77	11.83	2.10	.75	4.10	14.00	10.15
31.....	7.60	.....	24.20	.....	3.62	.....	11.05	2.50	.....	3.70	.....	14.20

During 1901 the following discharge measurements were made on Black Warrior River at Tuscaloosa, Ala.:

1901.

Feb. 1—Hydrographer, K. T. Thomas; gage height, 15.10; discharge, 9,300 second-feet.

March 15—Hydrographer, K. T. Thomas; gage height, 18.72; discharge, 9,461 second-feet.

June 27—Hydrographer, K. T. Thomas; gage height, 1.77; discharge, 828 second-feet.

*Daily gage height of Black Warrior River at Tuscaloosa, Ala., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	19.50	15.10	7.50	28.00	9.40	6.50	1.70	0.70	6.11	5.40	0.75	1.60
2.....	19.00	14.80	7.30	26.50	8.50	12.75	1.73	0.60	5.10	6.91	0.80	1.50
3.....	17.90	15.00	8.40	32.60	7.90	16.65	2.00	0.85	4.06	9.00	0.75	1.40
4.....	14.70	35.20	9.65	35.10	7.30	14.80	2.35	1.00	3.60	6.72	1.00	1.40
5.....	13.00	42.00	9.70	31.50	7.00	12.00	2.50	0.80	3.05	5.30	1.10	1.31
6.....	11.50	38.35	9.50	27.50	6.35	11.20	2.30	0.75	2.81	4.50	1.10	1.95
7.....	10.25	32.15	9.00	24.30	6.15	11.10	2.50	0.60	2.35	3.45	1.05	1.90
8.....	9.50	29.15	8.50	20.90	6.00	11.50	2.40	0.50	2.04	3.02	1.00	2.58
9.....	8.75	28.05	8.05	17.90	5.70	9.40	2.25	0.35	1.90	2.50	0.95	2.60
10.....	8.30	30.97	8.70	14.50	5.60	7.60	2.10	0.25	1.70	2.31	0.90	3.80
11.....	17.40	29.75	29.50	12.50	5.00	6.50	2.00	0.20	1.50	2.10	0.90	5.00
12.....	52.70	27.45	34.00	11.00	4.60	6.20	1.70	0.65	1.41	1.95	0.85	5.80
13.....	56.50	25.15	28.50	10.00	5.00	6.40	1.30	0.65	1.30	3.41	1.00	5.70
14.....	53.25	22.50	23.50	11.35	6.60	5.50	0.90	0.80	2.52	3.30	1.05	7.40
15.....	47.25	19.60	19.70	12.80	8.70	5.05	0.70	1.20	6.70	3.97	1.00	21.00
16.....	41.45	17.45	16.00	13.00	8.30	4.80	0.60	5.70	8.00	3.80	0.98	40.75
17.....	36.30	15.70	13.00	11.70	6.80	4.50	0.43	17.00	6.10	3.51	0.90	35.00
18.....	31.85	13.70	11.05	12.80	5.50	4.10	2.60	26.30	12.21	3.00	0.93	27.00
19.....	28.15	12.50	10.00	25.70	4.70	3.80	4.00	22.70	16.00	2.70	1.10	21.50
20.....	25.15	11.90	9.60	39.80	4.90	3.75	5.70	25.70	14.50	2.30	1.30	16.41
21.....	22.50	10.90	13.00	42.60	8.30	3.00	7.33	32.10	10.60	2.00	1.40	12.10
22.....	19.85	9.95	16.50	38.00	17.30	2.75	6.00	32.50	7.50	1.81	1.70	9.95
23.....	17.55	9.20	15.70	32.80	19.80	2.30	4.50	31.97	5.70	1.61	2.00	7.00
24.....	15.35	8.75	14.00	28.41	17.50	2.20	3.20	26.80	4.51	1.45	2.05	6.80
25.....	14.60	8.65	13.50	24.40	14.10	2.00	2.50	22.40	3.90	1.31	2.00	7.30
26.....	18.00	8.50	28.50	21.00	11.30	1.97	2.00	18.30	3.40	1.20	1.95	8.34
27.....	17.20	8.20	37.25	17.90	9.30	1.85	1.50	14.10	2.91	1.05	1.95	9.00
28.....	15.50	7.90	34.50	14.90	8.15	1.70	1.05	9.85	2.60	0.81	2.00	10.96
29.....	14.80	.....	29.00	12.50	7.00	1.65	0.90	7.90	2.91	0.57	1.95	76.30
30.....	15.10	.....	24.30	10.90	6.90	1.65	0.80	7.41	4.80	0.90	1.91	40.00
31.....	14.60	.....	24.85	.....	6.35	.....	0.60	7.10	.....	0.90	.....	49.00

*Rating table for Black Warrior River at Tuscaloosa, Ala.*

[This table is applicable from Jan. 1, 1895, to Dec. 31, 1901.]

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
—1.0	90	6.0	3,110	13.0	6,995	20.0	10,880
—0.9	105	6.1	3,160	13.1	7,045	20.1	10,950
—0.8	120	6.2	3,210	13.2	7,095	20.2	11,020
—0.7	140	6.3	3,260	13.3	7,145	20.3	11,090
—0.6	160	6.4	3,310	13.4	7,195	20.4	11,160
—0.5	180	6.5	3,370	13.5	7,255	20.5	11,230
—0.4	200	6.6	3,430	13.6	7,315	20.6	11,305
—0.3	220	6.7	3,490	13.7	7,375	20.7	11,380
—0.2	240	6.8	3,550	13.8	7,435	20.8	11,455
—0.1	260	6.9	3,610	13.9	7,495	20.9	11,530
0.0	280	7.0	3,665	14.0	7,550	21.0	11,600
0.1	310	7.1	3,715	14.1	7,600	21.1	11,690
0.2	340	7.2	3,765	14.2	7,650	21.2	11,780
0.3	370	7.3	3,815	14.3	7,700	21.3	11,870
0.4	400	7.4	3,865	14.4	7,750	21.4	11,960
0.5	430	7.5	3,925	14.5	7,810	21.5	12,050
0.6	460	7.6	3,985	14.6	7,870	21.6	12,140
0.7	490	7.7	4,045	14.7	7,930	21.7	12,230
0.8	530	7.8	4,105	14.8	7,990	21.8	12,320
0.9	565	7.9	4,165	14.9	8,050	21.9	12,410
1.0	600	8.0	4,220	15.0	8,105	22.0	12,500
1.1	635	8.1	4,270	15.1	8,155	22.1	12,600
1.2	670	8.2	4,320	15.2	8,205	22.2	12,700
1.3	710	8.3	4,370	15.3	8,255	22.3	12,800
1.4	750	8.4	4,420	15.4	8,305	22.4	12,900
1.5	790	8.5	4,480	15.5	8,365	22.5	13,000
1.6	830	8.6	4,540	15.6	8,425	22.6	13,100
1.7	870	8.7	4,600	15.7	8,485	22.7	13,200
1.8	910	8.8	4,660	15.8	8,545	22.8	13,300
1.9	955	8.9	4,720	15.9	8,605	22.9	13,400
2.0	1,000	9.0	4,775	16.0	8,660	23.0	13,500
2.1	1,045	9.1	4,825	16.1	8,710	23.1	13,620
2.2	1,090	9.2	4,875	16.2	8,760	23.2	13,740
2.3	1,135	9.3	4,925	16.3	8,810	23.3	13,860
2.4	1,180	9.4	4,975	16.4	8,860	23.4	13,980
2.5	1,225	9.5	5,035	16.5	8,920	23.5	14,100
2.6	1,270	9.6	5,095	16.6	8,980	23.6	14,220
2.7	1,320	9.7	5,155	16.7	9,040	23.7	14,340
2.8	1,370	9.8	5,215	16.8	9,100	23.8	14,460
2.9	1,420	9.9	5,275	16.9	9,160	23.9	14,580
3.0	1,470	10.0	5,330	17.0	9,215	24.0	14,700
3.1	1,520	10.1	5,380	17.1	9,265	24.1	14,830
3.2	1,570	10.2	5,430	17.2	9,315	24.2	14,960
3.3	1,620	10.3	5,480	17.3	9,365	24.3	15,090
3.4	1,670	10.4	5,530	17.4	9,415	24.4	15,200



Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
3.5	1,725	10.5	5,590	17.6	9,475	24.5	15,350
3.6	1,780	10.6	5,650	17.6	9,535	24.6	15,480
3.7	1,835	10.7	5,710	17.7	9,595	24.7	15,610
3.8	1,890	10.8	5,770	17.8	9,655	24.8	15,740
3.9	1,945	10.9	5,830	17.9	9,715	24.9	15,870
4.0	2,000	11.0	5,885	18.0	9,770	25.0	16,000
4.1	2,055	11.1	5,935	18.1	9,820	26.0	17,600
4.2	2,111	11.2	5,985	18.2	9,870	28.0	21,500
4.3	2,166	11.3	6,035	18.3	9,920	30.0	26,500
4.4	2,222	11.4	6,085	18.4	9,970	32.0	31,700
4.5	2,277	11.5	6,145	18.5	10,030	34.0	38,000
4.6	2,333	11.6	6,205	18.6	10,090	36.0	45,000
4.7	2,388	11.7	6,265	18.7	10,150	38.0	53,000
4.8	2,444	11.8	6,325	18.8	10,210	40.0	61,000
4.9	2,500	11.9	6,385	18.9	10,270	42.0	69,000
5.0	2,555	12.0	6,440	19.0	10,325	44.0	77,000
5.1	2,610	12.1	6,490	19.1	10,375	46.0	85,000
5.2	2,666	12.2	6,540	19.2	10,425	48.0	93,000
5.3	2,721	12.3	6,590	19.3	10,475	50.0	101,000
5.4	2,777	12.4	6,640	19.4	10,525	52.0	109,000
5.5	2,832	12.5	6,700	19.5	10,585	54.0	117,000
5.6	2,888	12.6	6,760	19.6	10,645	55.0	121,000
5.7	2,943	12.7	6,820	19.7	10,705		
5.8	3,000	12.8	6,880	19.8	10,765		
5.9	3,054	12.9	6,940	19.9	10,825		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Black Warrior River at Tuscaloosa, Alabama.*

[Drainage area, 4,900 square miles.]

Month.	Discharge in second-feet.			Run-off.		
	Maxi- mum.	Mini- mum.	Mean	Total in acre- feet.	Depth in inches.	Second- ft. per sq. mile.
1895.						
January .....	103,400	2,777	25,464	1,565,720	6.00	5.20
February .....	14,830	4,775	7,603	422,249	1.61	1.55
March .....	109,000	4,540	39,977	2,458,106	9.42	8.16
April .....	14,700	3,665	6,895	410,280	1.57	1.41
May .....	13,860	2,000	5,511	338,860	1.29	1.12
June .....	3,665	1,000	2,133	126,922	0.49	0.44
July .....	9,970	1,000	3,581	220,189	0.84	0.73
August .....	2,500	565	1,098	67,514	0.25	0.22
September .....	1,835	310	883	52,542	0.20	0.18
October .....	310	140	233	14,327	0.06	0.05
November .....	750	280	488	29,038	0.11	0.10
December .....	11,600	530	2,021	124,267	0.47	0.41
1896.						
January .....	24,610	2,444	5,981	367,757	1.41	1.22
February .....	47,000	3,665	19,161	1,102,153	4.22	3.91
March .....	52,600	3,160	12,996	799,093	3.06	2.65
April .....	14,100	2,721	6,072	361,309	1.38	1.24
May .....	49,800	1,370	7,420	456,238	1.74	1.51
June .....	9,970	1,090	2,910	173,157	0.65	0.59
July .....	4,420	460	1,232	75,753	0.29	0.25
August .....	750	310	478	29,391	0.12	0.10
September .....	400	120	201	11,960	0.04	0.04
October .....	260	120	157	9,654	0.03	0.03
November .....	600	120	307	18,268	0.07	0.06
December .....	4,045	430	955	58,721	0.22	0.19
1897.						
January .....	10,500	385	3,493	214,775	0.82	0.71
February .....	17,440	1,945	8,409	467,010	1.79	1.72
March .....	120,080	4,540	52,883	3,251,650	12.44	10.79
April .....	25,285	2,610	9,657	574,630	2.20	1.97
May .....	11,195	1,000	3,600	221,355	0.84	0.73
June .....	1,697	260	715	42,545	0.17	0.15
July .....	7,810	240	1,809	111,230	0.43	0.37
August .....	1,595	355	701	43,100	0.16	0.14
September .....	600	102	295	17,555	0.07	0.06
October .....	102	90	93	5,718	0.02	0.02
November .....	125	107	115	6,843	0.03	0.02
December .....	29,000	115	5,549	341,195	1.30	1.13
1898.						
January .....	75,000	1,972	16,577	1,019,287	3.90	3.38
February .....	12,230	1,752	3,902	216,706	0.83	0.80

*Estimated monthly discharge of Black Warrior River at Tuscaloosa, Alabama.*

[Drainage area, 4,900 square miles.]

Month.	Discharge in second-feet.			Total in acre-ft.	Run-off.	
	Maxi-mum.	Mini-mum.	Mean.		Depth in inches.	Second-ft. per sq. mile.
March . . . . .	17,120	1,370	3,626	222,955	0.85	0.74
April . . . . .	55,800	4,295	15,620	929,452	3.56	3.19
May . . . . .	6,060	370	1,766	108,589	0.41	0.36
June . . . . .	710	160	303	18,030	0.07	0.06
July . . . . .	1,395	250	549	33,757	0.13	0.11
August . . . . .	7,600	430	1,785	109,756	0.41	0.36
September . . . . .	670	140	252	14,995	0.06	0.05
October . . . . .	2,444	130	880	54,109	0.21	0.18
November . . . . .	6,265	280	1,626	96,754	0.37	0.33
December . . . . .	14,580	1,045	3,763	231,379	0.89	0.77
1899.						
January . . . . .	81,375	2,222	18,118	1,114,033	4.27	3.70
February . . . . .	90,375	10,030	30,923	1,717,376	6.57	6.31
March . . . . .	122,625	5,330	35,308	2,171,004	8.31	7.21
April . . . . .	32,800	4,540	11,901	708,158	2.71	2.43
May . . . . .	4,165	790	2,092	128,632	0.49	0.43
June . . . . .	1,000	175	448	26,658	0.10	0.09
July . . . . .	5,035	160	1,111	68,313	0.26	0.23
August . . . . .	3,370	370	963	59,213	0.23	0.20
September . . . . .	670	110	200	11,901	0.04	0.04
October . . . . .	175	92	130	7,993	0.03	0.03
November . . . . .	5,590	127	721	42,902	0.17	0.15
December . . . . .	47,650	460	8,880	546,010	2.09	1.81
1900.						
January . . . . .	29,760	2,277	9,857	606,083	2.01	2.32
February . . . . .	76,312	2,555	18,356	1,019,440	3.75	3.90
March . . . . .	87,750	8,280	27,105	1,666,623	5.53	6.37
April . . . . .	136,687	5,302	48,426	2,881,547	9.88	11.02
May . . . . .	11,825	1,645	3,702	227,627	0.76	0.88
June . . . . .	115,312	2,138	32,614	1,940,668	6.66	7.43
July . . . . .	52,000	2,888	10,952	673,412	2.24	2.59
August . . . . .	5,245	750	1,674	102,920	0.34	0.39
September . . . . .	5,680	512	1,580	94,017	0.32	0.36
October . . . . .	13,000	355	3,382	207,951	0.69	0.80
November . . . . .	12,700	977	3,701	220,225	0.76	0.85
December . . . . .	10,150	2,721	5,119	314,755	1.05	1.21
The year . . . . .	136,687	355	13,872	9,955,278	2.83	38.12

*Estimated monthly discharge of Black Warrior River at Tuscaloosa, Alabama.*

[Drainage area, 4,900 square miles.]

Month.	Discharge in second-feet.			Run-off.	
	Maximum.	Minimum.	Mean.	Second-feet per square mile.	Depth in inches.
1901.					
January .....	108,375	4,370	22,938	4.68	5.39
February .....	55,100	4,165	15,094	3.08	3.21
March .....	40,900	3,815	11,947	2.44	2.81
April .....	57,020	5,330	17,370	3.55	3.96
May .....	10,765	2,333	4,355	.89	1.03
June .....	9,010	850	3,217	.66	.74
July .....	3,815	415	1,210	.25	.29
August .....	29,550	340	7,117	1.45	1.67
September .....	8,660	710	2,626	.54	.60
October .....	4,775	445	1,536	.31	.36
November .....	1,022	512	712	.15	.17
December .....	80,250	710	13,293	2.71	3.12
The year .....	108,375	340	8,454	1.73	23.35

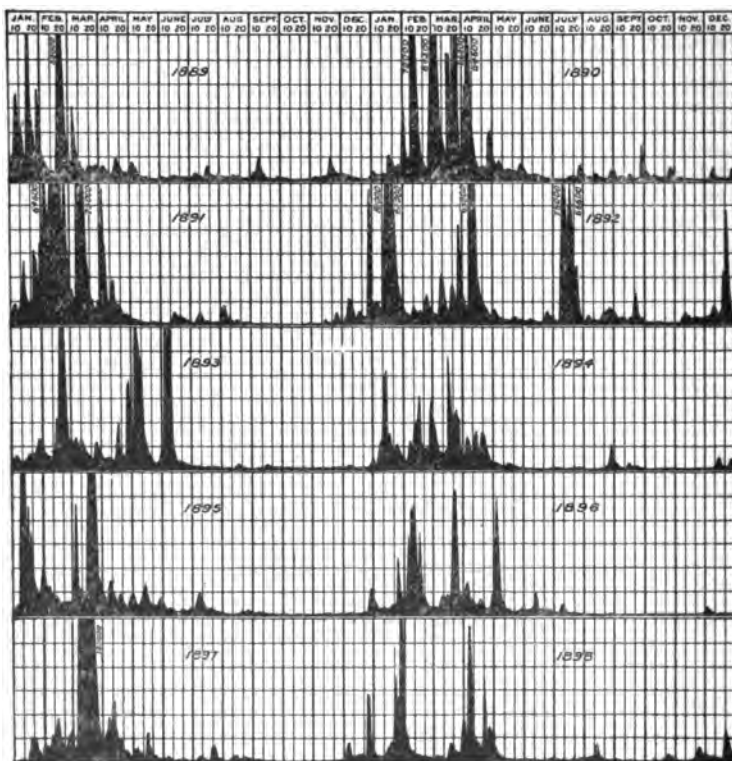


FIG. 14.—Discharge of Black Warrior River at Tuscaloosa, Alabama, 1889-1898.

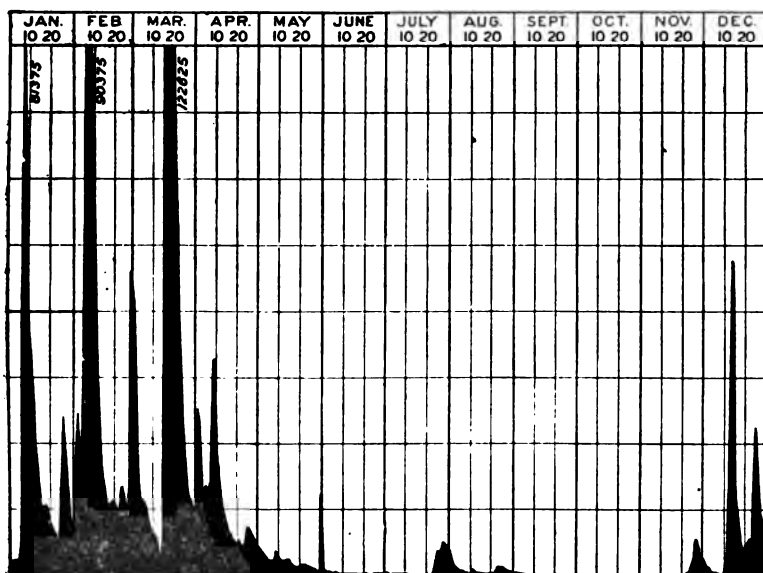


FIG. 15.—Discharge of Black Warrior River at Tuscaloosa, Alabama, 1899.

*Minimum monthly discharge of Black Warrior River at Tuscaloosa, Ala., with corresponding net horse power per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

[Drainage area, 4,900 square miles.]

	1899			1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January .....	2,222	202	1	2,277	207	1	4,370	398	1
February .....	10,030	912	2	2,555	232	1	4,165	378	1
March .....	5,330	485	1	8,282	753	1	3,815	347	1
April .....	4,539	413	1	5,302	483	1	5,330	485	1
May .....	790	72	1	1,645	150	1	2,333	212	1
June .....	175	16	1	2,138	194	1	850	77	2
July .....	160	15	7	2,888	262	1	415	38	1
August .....	370	34	2	750	68	2	340	31	1
September ...	110	10	1	512	47	1	710	65	1
October .....	92	9	1	355	32	1	445	40	1
November .....	127	12	5	977	89	1	512	47	1
December ....	460	42	1	2,721	247	1	710	65	1

## 2. BLACK WARRIOR RIVER NEAR CORDOVA, ALABAMA.

This station is located at the Kansas City, Memphis & Birmingham Railroad bridge, three-fourths of a mile from Cordova, Alabama. The gage was established by the United States Weather Bureau, but the records were discontinued by that bureau some time ago. From 12 to 55 feet the gage is a vertical timber bolted to the inside of the bridge pier on the left bank of the river. Below 12 feet the gage was sloping, but it was out of position, and could not be used when the station was established by the Geological Survey on May 21, 1900, so a short new section was put in at that time. This section is a 2-inch by 10-inch plank, graduated to feet and tenths, marked with nails from -1.5 feet to +12.5 feet, and spiked to a willow tree on the right bank of the river about 200 feet below the bridge. The bench mark is the top of the stone pier on the left bank, and is 54.95 feet above the zero of the gage. Measurements are made from the railroad bridge, which is a single-span, iron, through bridge 300 feet long. The section is a good one. The observer is A. B. Logan, who lives on the right bank of the river, only a few hundred feet from the end of the bridge. During 1900 measurement was made by Max Hall as follows:

*Daily gage height, in feet, of Black Warrior River near Cordova, Alabama, for 1900.*

Day	May.	June	July	Aug.	Sept.	Oct.	Nov.	Dec
1		0.6	8.1	0.9	0.5	-0.8	-0.1	1.4
2		2.8	5.2	.5	.2	-.8	+.5	1.0
3		5.3	7.1	.4	.1	-.9	.8	1.8
4		7.1	6.5	.2	.0	-.9	.4	1.0
5		7.5	5.3	.1	-.1	-1.0	.3	1.9
6		7.6	4.6	.0	-.2	-1.0	.3	1.8
7		6.5	3.5	.1	-.3	-1.1	.2	1.4
8		7.3	2.1	.2	-.4	+.2	.2	1.2
9		6.5	2.5	.3	-.5	.9	.1	1.0
10		4.5	2.7	.3	-.5	.9	.0	.8
11		3.2	1.0	.4	-.6	.1	-.1	.7
12		3.0	.8	.5	-.6	.9	-.2	.7
13		4.6	1.0	.5	-.7	.9	-.2	.8
14		8.6	.8	.5	-.6	3.2	-.3	.8
15		9.8	.5	.6	1.5	2.3	-.3	.8
16		8.0	.4	.6	.9	1.7	-.3	.6
17		8.2	.4	.6	.5	1.0	-.4	.5
18		7.2	.3	.6	.1	.6	+.3	.4
19		13.4	.3	.1	.0	.3	.2	.4
20		10.0	.2	.0	-.1	.2	.1	.5
21		0.1	.6	.7	.2	.2	.1	.8
22		.1	6.5	1.0	.3	.3	1.1	3.0
23		.1	15.2	.8	.4	.3	1.0	.8
24		1.5	22.9	.5	.8	.4	.7	.8
25		1.5	33.8	.9	1.7	.4	.5	.9
26		1.0	31.3	.5	.8	.5	.3	5.1
27		.6	22.1	1.6	.2	.5	.2	4.9
28		.7	23.9	2.4	.0	.6	.1	3.6
29		.8	22.4	1.3	.1	.7	.0	2.8
30		.9	16.5	1.2	.9	-.7	-.1	1.9
31		.9	....	1.5	1.0	.....	-.1	.....

During 1901 the following discharge measurements were made on Black Warrior River at Cordova, Alabama: 1901.

Jan. 8—Hydrographer, Max Hall; gage height, 1.30; discharge, 1,781 second-feet.

Feb. 18—Hydrographer, K. T. Thomas; gage height, 2.40; discharge, 2,863 second-feet.

March 12—Hydrographer, K. T. Thomas; gage height, 9.45; discharge, 13,279 second-feet.

April 17—Hydrographer, K. T. Thomas; gage height, 1.70; discharge, 2,024 second-feet.

June 20—Hydrographer, K. T. Thomas; gage height, 0.00; discharge, 644 second-feet.

Oct. 26—Hydrographer, K. T. Thomas; gage height, -0.40; discharge, 385 second-feet.

*Daily gage height, in feet, of Black Warrior River near Cordova, Alabama, for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	3.7	4.00	1.00	5.40	1.7	1.6	+0.1	-0.6	0.7	1.2	-0.4	-0.2
2.....	3.1	3.60	1.60	9.20	1.5	4.5	0.3	-0.6	0.5	0.8	-0.5	-0.2
3.....	2.8	4.50	2.30	14.50	1.3	2.8	0.1	-0.65	0.4	0.5	-0.5	-0.1
4.....	2.5	17.60	1.90	10.90	1.1	2.1	0.0	-0.65	0.2	0.4	-0.4	0.0
5.....	2.3	18.00	1.60	7.00	1.0	2.0	-0.2	-0.70	0.1	0.3	-0.4	0.1
6.....	2.0	9.10	1.80	5.80	0.8	2.6	-0.2	-0.70	0.0	0.2	-0.4	0.1
7.....	1.7	5.60	1.50	4.90	0.7	2.7	-0.3	-0.70	-0.1	0.1	-0.3	0.1
8.....	1.5	5.20	1.30	4.20	0.6	2.1	-0.3	-0.65	-0.2	0.0	-0.2	0.1
9.....	1.3	7.10	1.50	3.50	0.5	1.5	-0.4	-0.60	-0.3	-0.1	-0.4	0.1
10.....	2.6	8.10	17.50	2.80	0.4	1.1	-0.5	-0.60	-0.4	-0.2	-0.4	0.7
11.....	19.9	6.20	20.50	2.30	0.3	2.2	-0.6	-0.60	-0.5	-0.2	-0.4	1.2
12.....	32.5	5.40	10.80	2.10	0.4	1.5	-0.6	-0.30	-0.3	-0.2	-0.3	1.0
13.....	29.85	4.80	6.20	1.80	0.6	1.1	-0.6	+ .40	-0.3	0.0	-0.3	0.8
14.....	17.85	4.20	5.30	2.50	3.1	1.0	-0.7	.10	+0.5	+1.1	-0.3	6.2
15.....	9.25	3.80	4.20	2.80	1.8	0.9	-0.7	.20	0.7	0.6	-0.2	2.4
16.....	6.80	3.30	3.50	2.30	1.3	0.7	-0.7	5.00	0.5	0.3	-0.4	1.52
17.....	5.50	2.90	2.80	1.80	0.9	0.6	-0.6	12.10	3.8	0.1	-0.4	8.1
18.....	4.20	2.60	2.20	2.20	0.7	0.5	0.0	6.80	5.0	0.0	-0.4	5.3
19.....	3.80	2.20	1.90	9.40	0.9	0.3	+1.1	10.80	3.5	-0.1	-0.2	3.5
20.....	3.10	2.00	2.40	16.10	1.8	0.2	2.8	8.70	1.8	-0.2	0.0	2.6
21.....	2.80	1.80	5.00	11.50	5.8	0.0	1.6	8.50	1.1	-0.2	-0.1	2.0
22.....	2.60	1.60	4.20	6.80	7.3	-0.1	0.5	11.20	0.8	-0.3	-0.1	1.5
23.....	2.40	1.30	3.50	5.40	5.5	-0.2	0.0	7.30	0.5	-0.3	0.0	1.5
24.....	2.60	1.40	3.50	4.50	3.6	-0.3	-0.2	5.20	0.4	-0.4	-0.1	2.0
25.....	5.40	1.20	4.95	3.60	2.8	-0.3	-0.3	4.00	0.3	-0.4	-0.2	2.5
26.....	5.00	1.20	11.20	3.00	2.1	-0.3	-0.4	2.80	0.2	-0.4	-0.2	2.1
27.....	3.80	1.10	6.90	2.60	1.6	-0.4	-0.5	2.00	0.1	-0.4	-0.1	2.6
28.....	4.20	1.00	5.30	2.20	1.3	-0.4	-0.5	1.50	0.1	-0.4	-0.1	5.0
29.....	3.80	.....	4.60	2.00	1.1	-0.4	-0.5	1.40	1.0	-0.5	-0.2	18.5
30.....	3.60	.....	4.00	1.90	1.0	-0.5	-0.5	1.20	2.3	-0.5	-0.2	22.0
31.....	4.20	.....	6.20	.....	1.0	.....	-0.6	1.00	.....	-0.5	.....	13.5

Rating table for Black Warrior River at Cordova, Alabama, for 1900 and 1901.

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Sec. ft.	Feet.	Second ft.	Feet.	Second ft.
-0.7	205	3.3	4,144	7.3	10,064	11.3	15,984
-0.6	260	3.4	4,292	7.4	10,212	11.4	16,132
-0.5	320	3.5	4,440	7.5	10,360	11.5	16,280
-0.4	384	3.6	4,588	7.6	10,508	11.6	16,428
-0.3	450	3.7	4,736	7.7	10,656	11.7	16,576
-0.2	518	3.8	4,884	7.8	10,804	11.8	16,724
-0.1	588	3.9	5,032	7.9	10,952	11.9	16,872
0.0	660	4.0	5,180	8.0	11,100	12.0	17,020
0.1	734	4.1	5,328	8.1	11,248	12.1	17,168
0.2	810	4.2	5,476	8.2	11,396	12.2	17,316
0.3	888	4.3	5,624	8.3	11,544	12.3	17,464
0.4	968	4.4	5,772	8.4	11,692	12.4	17,612
0.5	1,050	4.5	5,920	8.5	11,840	12.5	17,760
0.6	1,134	4.6	6,068	8.6	11,988	12.6	17,908
0.7	1,220	4.7	6,216	8.7	12,136	12.7	18,056
0.8	1,307	4.8	6,364	8.8	12,284	12.8	18,204
0.9	1,396	4.9	6,512	8.9	12,432	12.9	18,352
1.0	1,486	5.0	6,660	9.0	12,580	13.0	18,500
1.1	1,577	5.1	6,808	9.1	12,728	13.1	18,648
1.2	1,669	5.2	6,956	9.2	12,876	13.2	18,796
1.3	1,762	5.3	7,104	9.3	13,024	13.3	18,944
1.4	1,856	5.4	7,252	9.4	13,172	13.4	19,092
1.5	1,951	5.5	7,400	9.5	13,320	13.5	19,240
1.6	2,047	5.6	7,548	9.6	13,468	13.6	19,388
1.7	2,144	5.7	7,696	9.7	13,616	13.7	19,536
1.8	2,242	5.8	7,844	9.8	13,764	13.8	19,684
1.9	2,342	5.9	7,992	9.9	13,912	13.9	19,832
2.0	2,444	6.0	8,140	10.0	14,060	14.0	19,980
2.1	2,547	6.1	8,288	10.1	14,208	14.1	20,128
2.2	2,652	6.2	8,436	10.2	14,356	14.2	20,276
2.3	2,758	6.3	8,584	10.3	14,504	14.3	20,424
2.4	2,868	6.4	8,732	10.4	14,652	14.4	20,572
2.5	2,988	6.5	8,880	10.5	14,800	14.5	20,720
2.6	3,118	6.6	9,028	10.6	14,948	14.6	20,868
2.7	3,258	6.7	9,176	10.7	15,096	14.7	21,016
2.8	3,404	6.8	9,324	10.8	15,244	14.8	21,164
2.9	3,552	6.9	9,472	10.9	15,392	14.9	21,312
3.0	3,700	7.0	9,620	11.0	15,540	15.0	21,460
3.1	3,848	7.1	9,768	11.1	15,688		
3.2	3,996	7.2	9,916	11.2	15,836		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.



*Estimated monthly discharge of Mulberry Fork of Black Warrior River, near Cordova, Alabama.*

[Drainage area, 237 square miles.]

Month.	Discharge in second-feet.			Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Second- feet per square mile.	Depth in inches.
1900.					
June .....	49,284	1,134	16,185	8.52	9.51
July .....	11,248	810	2,975	1.57	1.81
August .....	2,144	660	1,016	.53	.61
September .....	1,951	205	556	.29	.32
October .....	13,098	60	1,732	.91	1.05
November .....	6,808	384	1,487	.78	.87
December .....	6,660	968	2,154	1.13	1.30
1901.					
January .....	51,800	1,762	8,713	4.59	5.29
February .....	31,820	1,486	6,616	3.48	3.62
March .....	29,600	1,486	6,637	3.49	4.02
April .....	23,088	2,242	6,967	3.67	4.10
May .....	10,064	968	2,539	1.34	1.54
June .....	5,920	320	1,582	.83	.93
July .....	3,404	205	631	.33	.38
August .....	17,168	205	4,155	2.19	2.53
September .....	6,660	320	1,415	.74	.83
October .....	1,669	320	687	.36	.42
November .....	660	320	468	.25	.28
December .....	31,820	518	4,923	2.59	2.99
The year .....	51,800	205	3,778	1.99	26.93

*Minimum monthly discharge of Black Warrior River at Cordova, Ala., with corresponding net horse power per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

[Drainage area, 237 square miles.]

	1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January .....				1,762	160	1
February .....				1,486	135	1
March .....				1,486	135	1
April .....				2,242	204	2
May .....	784	67	3	968	88	1
June .....	1,134	103	1	320	29	1
July .....	810	74	1	205	19	3
August .....	660	60	3	205	19	3
September .....	205	19	3	320	29	1
October .....	60	55	1	320	29	3
November .....	384	35	1	320	29	2
December .....	968	88	2	518	47	2

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table for that month.

### 3. SURVEY OF BLACK WARRIOR RIVER, ALABAMA.

The Black Warrior River is formed by the junction of the Mulberry and Sipsey forks of Black Warrior at Old Warrior Town in Walker County, Alabama, and runs in a southwesterly direction past Tuscaloosa to Demopolis, Ala., at which point it enters the Tombigbee River. Above Tuscaloosa it is known as the Black Warrior River, and below Tuscaloosa as the Warrior River.

The accompanying profile is made from the surveys of the Corps of Engineers, U. S. A.

A great deal of work is being done by the Government on this river in order to make it navigable as an outlet to important coal fields above.

In the 92 miles from Old Warrior Town to Tuscaloosa, there is a fall of 158 feet. The distribution of this fall is shown by the following table, giving distances in miles above Tuscaloosa and elevations of water surface above sea level.

Miles above Tuscaloosa.	Elevation above sea level.	
92.4	249.7	Fork of Sipsey and Mulberry.
85.3	248.8	Black Water Creek.
84.6	242.3	Foot of Sanders Shoals.
79.7	242.1	Big Cane Creek.
76.2	240.8	Above Paynes Mill.
76.2	237.1	Below Paynes Mill.
74.4	231.7	Foot of Bee Shoals.
71.2	231.6	Birch Shoals.
71.2	230.6	Birch Shoals.
69.0	230.0	Top of Tuggles Shoals.
68.0	225.6	Foot of Tuggles Shoals.
55.0	216.6	Mouth of Lost Creek.
48.0	215.1	Mouth of Little Warrior River.
47.6	206.1	Foot of Fork Shoals.
43.3	296.1	Above Knight's Mill.
43.3	202.5	Below Knight's Mill.
38.0	202.2	Above Black Rock.
37.7	193.0	Below Black Rock.
30.3	192.3	Top of Squaw Shoals.
25.8	151.3	Foot of Squaw Shoals.
21.7	139.8	Foot of Fair Shoals.
19.3	132.3	Foot of Rose Shoals.
8.7	132.30	Top of Lock 4, at Lower Yellow Creek.
2.0	120.16	Top of Lock 3, Tuscaloosa, Ala.
1.3	109.66	Top of Lock 2, Tuscaloosa, Ala.
0.7	101.16	Top of Lock 1, Tuscaloosa, Ala.
	91.30	Foot of Lock 1, Tuscaloosa, Ala.

There are gage stations, both at Tuscaloosa and Cordova, Ala., where systematic discharge measurements have been made, the results of which are given in the foregoing pages. Comparative measurements at the two stations at same stage, which was same stage as low water in November, 1901, shows a discharge of 825 second-feet at Tuscaloosa, and 285 second-feet at Cordova. At minimum stage of dryest years the water gets considerably lower, as is shown by the records referred to, but the figures named are safe for low season in all ordinary years, and will be used in this discussion, for determining the power available at different sites along the river.

The locks and proposed locks on this section of the river begin with No. 1, at Tuscaloosa, and are numbered up the river. Locks 1, 2, 3 and 4 are about completed, and others are projected, but the locations of the latter in the following list are approximated. However, the exact location of each is immaterial in showing the power available. The following is a table showing positions of locks and lock sites in miles above Tuscaloosa, the sea level elevation of water below each, the lift at each and the net horsepower that can be developed at each day on an 80 per cent. turbine during dry season in ordinary years, like 1900, after deducting 100 second-feet for lockage.

No. of lock or site.	Miles from Tuscaloosa.	Sea-level elevation of water below lock.	Lift.	Sec.-ft. after deducting 100 for lockage.	Net H. P. on 80 per cent. turbine without storage.	Location.
1	0.7	91.30	9.85	725	650	Bottom University Shoal, Tuscaloosa.
2	1.3	101.16	8.50	725	560	On University Shoal, Tuscaloosa.
3	2.0	109.66	10.50	725	690	On University Shoal, Tuscaloosa.
4	8.7	120.16	12.14	704	777	Near mouth of Yellow Creek.
5	19.3	132.30	10.00	660	600	Foot of Rose Shoals.
6	21.7	142.30	9.00	660	540	Foot of Fair Shoals.
7	25.8	151.30	14.00	660	840	Foot of Squaw Shoals.
8	26.3	165.30	14.00	660	840	On Squaw Shoals.
9	27.8	179.30	14.00	660	840	On Squaw Shoals.
10	37.7	193.3	14.00	550	700	Below Black Rock.
11	47.6	207.3	14.00	550	700	Mouth of Little Warrior River, or Locust Fork.
12	63.4	221.3	14.00	374	476	
13	75.0	235.3	14.00	285	364	

The best power on the river is at Squaw Shoals, 26 miles above Tuscaloosa, covered on the above table by locks Nos. 7, 8 and 9, each having a lift of 14 feet, and making a total fall on Squaw Shoals of 42 feet. This can be developed to best advantage by constructing a canal from the top of proposed dam at Lock No. 9, along the river bank, two miles in length, to a point opposite the foot of Squaw Shoals, below Lock No. 7. This canal taking the river water not need for lockage, and allowing two feet for grade and storage, will utilize a net head of 40 feet, and produce 2,400 net horse power continuously, or 4,800 net horse power for a 12-hour run per day, storing the water above Lock No. 9 during the 12 idle hours.

It is to be remembered that the above estimates of power are for low season during ordinary years. There will be exceptional periods of minimum water in extremely dry years in which the entire flow of the river will be as low as 100 second-feet, and will, therefore, barely suffice for lockage during a busy season of boating on the river. See Nineteenth Annual Report, United States Geological Survey, Part IV, page 251. But such seasons are rare, and the facilities for water transportation should compensate for them to a great extent. It is admitted that the cheapness of coal along this river would naturally make the water powers less valuable, but the cheapness of development in connection with Government dams would partly offset the cheapness of coal. It is believed that the proposed development at Squaw Shoals could be made at a very moderate cost, and that such an investment would pay handsomely.

## APPENDIX TO BLACK WARRIOR REPORT.

The following additional information concerning the Warrior and Black Warrior River is from Mr. R. C. McCalla, U. S. Assistant Engineer, Tuscaloosa, Ala., who is in charge of the improvements on that river.

Tuscaloosa is 361 miles by river above Mobile, and above here the river is called the Black Warrior, and below it is called the Warrior. The locks on the two parts of the stream are numbered as two separate systems, the lowest lock in each system being No. 1, and the numbers running up stream. The following table gives the lift and location of the locks in both systems:

No. of lock.	Lift in feet.	Miles above Mobile.	
1	10.00	230.5	0.5 miles below mouth of Warrior; located but not begun.
2	10.00	246.2	Located but not begun.
3	10.00	266.7	Located but not begun.
4	10.00	282.3	Under construction.
5	10.00	298.3	Under construction.
6	10.00	315.2	Under construction.
1	9.88	361.9	In operation.
2	8.50	362.3	In operation.
3	10.50	363.1	In operation.
4	12.14	370.1	Under construction.

Between Lock No. 4 and the junction of Mulberry and Locust Forks, 407.8 miles above Mobile, there are projected seven locks at 14 feet lift each, but none of these are yet located. The following table gives the location, etc., of gages now established and read daily at 7 A. M.:

Name of Gauge.	No. of gauges.	Miles above Mobile.	Elevation of zero above mean low tide, Mobile.	Remarks.
Demopolis ....	1	229.7	28.07	Zero, about 1½ ft. above mean low water.
Millwood ....	1	259.8	45.97	Zero about mean low water.
Lock 4.....	1	282.3	64.50	Zero top of lower mitre sill.
A. G. S. bidge	1	288.0	61.28	Zero about mean low water.
Lock 5.....	1	298.3	64.50	Zero top of lower mitre sill.
Lock 6.....	1	315.2	74.50	Zero top of lower mitre sill.
Grays Landg.	1	319.5	80.41	Zero about mean low water.
Tuscaloosa ...	1	361.1	86.86	Zero about 1 ft. above mean low water.
Lock 1.....	2	361.9	84.36	Zero top of lower mitre sill.
Lock 2.....	2	362.3	94.36	Zero top of lower mitre sill.
Lock 3.....	2	363.1	102.86	Zero top of lower mitre sill.
Lock 4.....	1	370.1	113.36	Zero top of lower mitre sill.
Cordova .....	1	445.0	237.65	Zero about mean low water.

## 5. BLACK WARRIOR RIVER TRIBUTARIES.

At Clear Creek Falls, in Winston County, within a distance of half a mile, there is a fall of over 100 feet, distributed as follows:

Rapids above Upper Falls in 100 yards.....	6 feet fall
Upper Falls, about .....	45 feet fall
Still pool for 275 yards .....	00 feet fall
Lower Falls .....	27 feet fall
Rapids below Lower Falls .....	30 feet fall

No discharge measurements have ever been made on this stream. It is thought best not to attempt to approximate its flow by any water-shed rule, as the stream originates from big springs. Actual discharge measurements will be made during 1902.

Little Warrior, or Locust Fork, is an important stream on which no surveys have been made. On December 1, 1901, a hydrographic station was established on this stream at Palos, Ala., by Mr. R. C. McCalla, U. S. Assistant Engineer, and will be maintained by him.

Mr. B. M. Hall and his assistants will make a series of discharge measurements at this station during 1902. The only measurement so far was on January 18, 1902. The gage stood at 0.85, and the discharge was 849 second-feet.

There are other tributaries having shoals that can be developed, but they have not been examined.

## CHAPTER VI.

### 1. TOMBIGBEE RIVER AT COLUMBUS, MISSISSIPPI.

This station is located about 1,000 feet below the highway bridge,  $1\frac{1}{2}$  miles from the Southern Railway depot at Columbus. The rod, which is in three sections, is fastened vertically to the rock bluff on the left bank. It is a 3-inch by 10-inch pine timber 45 feet long, marked with brass figures and copper nails, the graduation extending from  $-5.0$  feet to  $+40.0$  feet. The initial point of sounding is the end of the iron bridge, right bank, downstream side. Bench mark No. 1 is 250 feet from the initial point of sounding. The bridge floor is 40.85 feet above the zero of the rod, and the top of the iron girder under the floor timbers is 39.85 feet above the zero. Bench mark No. 2 is the top of the rail at the depot of the Southern Railway, and is 55.2 feet above gage datum and 190.9 feet above mean sea level. The width of the river at low water is 160 feet. The maximum record height of the river was on April 8, 1892, when the gage registered 42 feet. The lowest recorded height was on October 26, 1893, when the gage reading was  $-3.9$  feet. The danger line is at 33 feet. No measurements of discharge were made during 1900.

*Daily gage height in feet of Tombigbee River at Columbus, Miss., for 1900.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	3.5	0.4	7.1	4.5	8.0	1.0	19.7	5.8	-1.2	-2.7	+2.6	+3.4
2.....	2.2	.3	6.8	3.8	6.5	8.0	18.3	4.0	-1.0	-2.8	+4.9	+2.5
3.....	1.8	.2	5.6	2.7	5.9	10.0	17.6	2.8	-1.0	-3.0	+5.5	+1.9
4.....	1.6	.4	4.4	1.9	4.5	13.4	16.2	1.9	-7	-3.1	+5.1	+1.0
5.....	1.4	1.8	3.8	1.5	2.8	15.3	15.9	1.0	-6	-3.3	+4.4	+1
6.....	1.2	3.5	3.4	1.3	1.5	17.0	15.4	.0	-9	-3.5	+3.1	+2
7.....	1.1	3.5	7.6	1.3	.8	20.7	14.5	.5	1.3	-3.5	+1.9	+3
8.....	1.0	3.3	14.4	1.1	.5	23.6	13.5	.9	1.9	-2.2	+9	+3
9.....	.9	4.2	15.1	.7	1.4	25.5	10.0	1.4	2.1	-1.0	.1	.0
10.....	.9	8.4	13.8	.5	2.3	25.0	6.8	1.8	2.3	.6	.5	.3
11.....	2.0	7.8	11.3	11.7	3.3	23.6	5.5	2.2	2.4	.4	.9	+5
12.....	6.6	7.6	9.9	16.2	3.6	21.6	4.0	-2.6	2.5	+2.6	1.1	.6
13.....	8.3	10.2	6.7	17.4	2.8	20.0	3.5	-2.6	2.6	+4.8	1.3	.9
14.....	7.1	9.8	4.3	19.3	2.2	18.5	2.0	-2.7	2.7	+5.2	1.3	.9
15.....	5.6	8.1	4.8	20.8	1.6	17.8	1.9	-2.2	2.7	+5.6	1.4	.9
16.....	4.6	5.8	5.6	20.3	.7	17.1	1.5	-2.2	2.8	+5.4	1.5	.9
17.....	2.8	4.6	5.2	22.9	.0	17.3	1.0	-2.3	2.9	+2.4	1.5	.9
18.....	2.4	3.8	4.6	26.9	.4	17.8	.5	-2.3	3.0	.4	1.6	.9
19.....	2.2	3.2	9.4	27.6	-1.0	18.0	.0	-1.8	3.0	.4	1.6	1.0
20.....	2.1	2.8	15.6	27.5	.8	16.8	.4	-1.4	3.1	.9	1.6	1.0
21.....	1.9	3.5	18.2	27.1	.5	15.2	1.3	-1.1	3.0	-1.1	1.6	1.0
22.....	1.8	5.6	19.0	25.5	.5	13.5	2.0	-1.6	2.9	-1.3	1.2	+3.8
23.....	1.6	5.4	19.2	23.3	.1	13.8	1.9	-1.9	2.7	-1.0	.0	+5.0
24.....	1.4	4.5	18.1	21.3	.2	18.5	1.4	-2.0	1.3	.2	+1.7	+6.0
25.....	1.2	5.0	15.2	19.4	.3	21.5	.7	-1	1.7	.8	+2.3	+6.6
26.....	1.1	5.0	11.4	17.3	.3	24.1	.1	-1.2	2.0	-1.0	+2.8	+6.8
27.....	.9	3.9	7.8	14.8	2.4	25.0	1.8	.6	2.2	+5.1	+3.3	+6.1
28.....	.7	4.2	4.6	11.8	2.2	24.8	5.0	.3	2.4	+6.6	+3.8	+5.0
29.....	.6		3.9	9.5	1.6	23.5	7.2	-1.1		+3.0		+4.0
30.....	.5		3.1	9.4	1.0	21.7	5.9	-1.0	2.5	+4.6	+3.9	+4.2
31.....	.4		3.3		.8		6.7	-1.4	2.5	+4.0	+3.8	+4.8

The following discharge measurements were made during 1901 by K. T. Thomas:

March 11—Gage height, 12.33 feet; discharge, 19,425 second-feet.

April 16—Gage height, 1.10 feet; discharge, 3,926 second-feet.

June 25—Gage height, —2.50; discharge, 698 second-feet.

Oct. 30—Gage height, —3.00; discharge, 657 second-feet.

*Daily gage height in feet of Tombigbee River at Columbus, Miss., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1	4.0	6.0	0.9	2.6	0.9	1.3	— 1.6	— 3.2	— 0.4	— 2.3	— 3.0	— 2.2
2	3.7	5.9	0.8	6.4	0.5	3.1	— 2.4	— 3.4	— 0.9	— 2.0	— 3.0	— 2.2
3	3.2	6.8	2.2	8.0	0.2	3.3	— 2.4	— 3.5	— 1.3	— 2.2	— 3.0	— 2.3
4	2.6	12.3	2.4	7.1	0.0	3.9	— 2.1	— 3.4	— 1.6	— 2.2	— 2.9	— 2.3
5	1.8	13.2	2.1	6.8	— 0.3	3.5	— 2.4	— 3.3	— 1.6	— 2.5	— 2.9	— 2.2
6	1.0	13.9	1.8	6.2	— 0.5	3.1	— 1.6	— 3.2	— 1.9	— 2.5	— 2.9	— 2.2
7	0.7	15.1	1.4	5.3	— 0.6	2.8	— 2.6	— 3.0	— 2.0	— 2.6	— 3.0	— 2.2
8	0.1	15.9	1.2	4.0	— 0.9	1.9	— 2.6	— 2.8	— 2.2	— 2.7	— 2.8	— 2.1
9	0.0	15.6	1.0	2.9	— 1.0	1.4	— 2.7	— 2.8	— 2.3	— 2.7	— 2.8	— 2.0
10	0.3	14.5	8.8	2.0	— 1.1	1.0	— 2.8	— 2.9	— 2.4	— 2.8	— 2.8	— 1.8
11	10.9	13.0	12.1	1.4	— 1.2	— 0.1	— 2.8	— 2.9	— 2.5	— 2.8	— 2.7	— 1.1
12	16.9	12.5	14.0	0.9	— 1.2	— 0.7	— 2.9	— 3.1	— 2.5	— 2.8	— 2.8	— 0.5
13	19.4	12.4	17.5	0.7	— 0.4	— 1.0	— 3.0	— 3.0	— 2.4	— 2.6	— 2.8	— 0.1
14	21.7	11.0	19.4	0.7	— 3.3	— 0.9	— 3.1	— 3.2	— 2.1	— 2.5	— 2.8	— 4.5
15	22.7	8.9	19.0	0.9	— 4.4	— 0.8	— 3.1	— 1.5	— 1.8	— 2.2	— 2.8	— 9.5
16	22.3	6.0	17.1	1.2	— 4.4	— 0.8	— 3.2	— 4.0	— 1.0	— 2.0	— 2.7	— 9.8
17	20.9	4.4	13.8	1.5	— 3.6	— 0.8	— 3.2	— 11.5	— 0.2	— 2.0	— 2.7	— 9.2
18	18.8	3.3	10.8	6.0	— 2.7	— 1.0	— 3.0	— 12.4	— 2.5	— 2.2	— 2.1	— 9.8
19	16.0	2.7	8.0	11.8	— 2.1	— 1.2	— 3.0	— 12.4	— 3.4	— 2.3	— 2.6	— 10.0
20	13.6	2.2	6.3	12.1	— 2.1	— 1.7	— 3.0	— 14.0	— 3.5	— 2.4	— 2.5	— 8.8
21	9.4	1.8	4.6	12.7	— 6.3	— 2.0	— 2.0	— 15.6	— 2.6	— 2.5	— 2.4	— 5.9
22	6.2	1.5	3.5	13.5	— 6.7	— 2.2	— 1.9	— 16.9	— 1.4	— 2.6	— 2.2	— 4.8
23	3.8	1.2	3.2	13.5	— 6.3	— 2.4	— 2.3	— 14.8	— 0.4	— 2.7	— 2.3	— 2.4
24	3.0	1.0	3.1	11.8	— 5.3	— 2.5	— 2.5	— 12.1	— 0.6	— 2.8	— 2.1	— 1.4
25	5.9	1.0	2.8	8.2	— 4.0	— 2.5	— 2.6	— 8.9	— 1.2	— 2.9	— 2.0	— 0.8
26	6.5	1.1	2.5	5.1	— 2.2	— 2.6	— 2.9	— 6.5	— 1.6	— 2.9	— 1.9	— 0.7
27	6.3	1.2	2.2	3.4	— 1.5	— 2.6	— 3.0	— 4.1	— 1.9	— 2.9	— 1.8	— 1.1
28	6.0	1.0	1.9	2.5	— 1.8	— 2.6	— 3.1	— 2.0	— 2.1	— 2.9	— 1.8	— 1.0
29	5.8	.....	1.6	1.8	— 1.7	— 2.7	— 3.2	— 1.6	— 2.2	— 3.0	— 2.0	— 9.0
30	5.7	.....	1.4	1.3	— 1.2	— 2.6	— 3.3	— 1.1	— 2.3	— 3.0	— 2.1	— 11.0
31	5.5	.....	2.3	.....	— 0.9	.....	— 3.4	— 0.3	.....	— 3.0	.....	— 9.6



Rating table for Tombigbee River at Columbus, Miss., for 1900 and 1901.

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
-3.0	650	1.0	3,790	5.0	9,310	9.0	14,830
-2.9	668	1.1	3,928	5.1	9,448	9.1	14,968
-2.8	688	1.2	4,066	5.2	9,586	9.2	15,106
-2.7	712	1.3	4,204	5.3	9,724	9.3	15,244
-2.6	736	1.4	4,342	5.4	9,862	9.4	15,382
-2.5	752	1.5	4,480	5.5	10,000	9.5	15,520
-2.4	780	1.6	4,618	5.6	10,138	9.6	15,658
-2.3	810	1.7	4,756	5.7	10,276	9.7	15,796
-2.2	842	1.8	4,894	5.8	10,414	9.8	15,934
-2.1	877	1.9	5,032	5.9	10,552	9.9	16,072
-2.0	915	2.0	5,170	6.0	10,690	10.0	16,210
-1.9	956	2.1	5,308	6.1	10,828	10.1	16,348
-1.8	1,000	2.2	5,446	6.2	10,966	10.2	16,486
-1.7	1,047	2.3	5,584	6.3	11,104	10.3	16,624
-1.6	1,097	2.4	5,722	6.4	11,242	10.4	16,762
-1.5	1,150	2.5	5,860	6.5	11,380	10.5	16,900
-1.4	1,206	2.6	5,998	6.6	11,518	10.6	17,038
-1.3	1,265	2.7	6,136	6.7	11,656	10.7	17,176
-1.2	1,328	2.8	6,274	6.8	11,794	10.8	17,314
-1.1	1,394	2.9	6,412	6.9	11,932	10.9	17,452
-1.0	1,464	3.0	6,550	7.0	12,070	11.0	17,590
-0.9	1,537	3.1	6,688	7.1	12,208	11.5	18,280
-0.8	1,613	3.2	6,826	7.2	12,346	12.0	18,970
-0.7	1,692	3.3	6,964	7.3	12,484	12.5	19,660
-0.6	1,775	3.4	7,102	7.4	12,622	13.0	20,350
-0.5	1,863	3.5	7,240	7.5	12,760	13.5	21,040
-0.4	1,957	3.6	7,378	7.6	12,898	14.0	21,730
-0.3	2,057	3.7	7,516	7.7	13,036	14.5	22,420
-0.2	2,165	3.8	7,654	7.8	13,174	15.0	23,110
-0.1	2,283	3.9	7,792	7.9	13,312	15.5	23,800
0.0	2,410	4.0	7,930	8.0	13,450	16.0	24,490
0.1	2,548	4.1	8,068	8.1	13,588	16.5	25,180
0.2	2,686	4.2	8,206	8.2	13,726	17.0	25,870
0.3	2,824	4.3	8,344	8.3	13,864	17.5	26,560
0.4	2,962	4.4	8,482	8.4	14,002	18.0	27,250
0.5	3,100	4.5	8,620	8.5	14,140	18.5	27,940
0.6	3,238	4.6	8,758	8.6	14,278	19.0	28,630
0.7	3,376	4.7	8,896	8.7	14,416	19.5	29,320
0.8	3,514	4.8	9,034	8.8	14,554	20.0	30,010
0.9	3,652	4.9	9,172	8.9	14,692		

*Estimated monthly discharge of Tombigbee River at Columbus, Miss.*  
 [Drainage area, 4,440 square miles.]

Month	Discharge in second-feet.			Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Depth in inches.	Second feet per square mile.
1900.					
January .....	13,864	2,962	5,588	1.26	1.45
February .....	16,486	2,686	8,659	1.95	2.03
March .....	23,938	6,688	15,285	3.42	3.85
April .....	40,498	3,100	21,265	4.79	5.34
May .....	13,450	1,464	4,944	1.11	1.28
June .....	37,600	37,90	27,692	6.24	6.96
July .....	29,596	2,410	11,411	2.57	2.97
August .....	10,414	707	2,257	.51	.59
September .....	1,775	632	950	.21	.23
October .....	10,138	566	3,989	.90	1.04
November .....	10,000	1,097	4,304	.97	1.08
December .....	11,794	1,464	5,239	1.18	1.36
The year .....	40,498	566	9,299	2.09	28.18
1901.					
January .....	33,736	2,410	14,193	3.20	3.69
February .....	24,352	3,790	12,533	2.83	2.95
March .....	29,182	3,514	10,884	2.45	2.33
April .....	21,040	3,376	9,890	2.23	2.49
May .....	11,656	1,328	4,949	1.11	1.28
June .....	7,792	707	2,767	.62	.69
July .....	10,97	582	730	.16	.18
August .....	24,352	582	7,673	1.73	1.99
September .....	7,240	753	2,008	.45	.50
October .....	915	650	748	.17	.20
November .....	1,000	650	756	.17	.19
December .....	17,590	810	6,730	1.52	1.75
The year .....	33,736	582	6,155	1.39	18.74

*Minimum monthly discharge of Tombigbee River at Columbus, Miss., with corresponding net horse power per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

[Drainage area, 4,440 square miles.]

	1900			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot fall.	No. of days duration of minimum.
January .....	2,962	269	1	2,410	219	1
February .....	2,686	244	1	3,790	345	3
March .....	6,688	608	1	3,514	319	1
April .....	3,100	282	1	3,376	307	2
May .....	1,464	133	1	1,328	121	2
June .....	3,790	345	1	707	65	1
July .....	2,410	219	1	582	53	1
August .....	707	65	1	582	53	1
September .....	632	57	1	753	68	2
October .....	566	51	2	650	59	3
November .....	1,097	100	4	650	59	4
December .....	1,464	133	4	810	74	2

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "Net H. P. per foot of fall" in this table for that month.

## 2. TOMBIGBEE RIVER NEAR EPES, ALABAMA.

A record of gage heights has been kept at this station for the last ten years by the Alabama Great Southern Railway Company. The gage is painted on the center brick pier of the railway bridge of that company across the Tombigbee a half mile east of Epes, and is referred to two bench marks, the first, the top of the iron girder at the third cross-beam at the station, 80 feet from the right-bank end of the iron bridge, is 64.70 feet above datum of gage; the second, the top of the cross-tie or the base of the rail at the station, 80 feet from the right-bank end of the iron bridge, is 65.50 feet above datum of gage. The west bank of the river is a solid wall of limestone, the east bank is flat and is subject to overflow. The trestle at the east end of the bridge is seven-eighths of a mile long. The section is good, though the water is very deep and rather swift.

*Daily gage height in feet of Tombigbee River near Epes, Ala., for 1900.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	8.5	3.0	18.0	24.5	43.0	6.0	44.5	14.5	2.0	0.5	7.0	8.0
2.....	7.5	3.0	19.0	19.5	41.0	8.0	44.5	12.0	2.5	.5	8.0	7.5
3.....	6.5	3.0	18.0	16.0	39.0	15.0	44.5	10.5	2.0	.5	9.0	7.0
4.....	6.0	3.0	17.0	12.0	33.5	21.0	44.0	8.0	2.0	.5	8.5	6.0
5.....	5.5	5.0	15.0	10.0	28.0	24.5	42.5	6.0	2.0	.5	8.5	5.0
6.....	5.0	6.0	14.0	8.0	20.5	27.0	42.0	5.0	2.0	.5	7.5	4.5
7.....	4.5	7.0	13.0	7.5	16.0	29.5	41.0	5.0	2.0	.5	7.0	4.0
8.....	4.0	7.5	18.0	7.5	10.0	32.0	40.0	4.0	2.0	.5	5.5	4.0
9.....	3.5	11.5	21.0	7.0	7.0	34.5	39.0	3.0	2.0	.5	4.5	4.0
10.....	3.5	13.5	23.0	6.5	7.0	37.0	38.0	2.0	2.0	2.0	4.0	3.5
11.....	13.0	15.0	24.0	20.5	7.0	38.5	34.0	2.0	1.5	4.0	4.0	3.0
12.....	20.0	20.5	24.0	26.0	8.0	39.5	26.0	2.0	1.5	6.0	3.0	3.0
13.....	23.0	26.0	23.0	29.0	8.0	40.5	23.0	2.0	1.5	7.5	3.0	3.5
14.....	23.5	28.0	20.0	30.0	7.0	41.0	15.5	2.0	2.0	8.5	2.5	3.5
15.....	22.0	28.0	17.5	31.0	6.0	41.5	13.0	2.0	1.5	10.0	2.0	3.5
16.....	21.0	26.0	17.0	38.0	6.0	42.0	8.0	1.5	1.0	10.5	2.0	3.0
17.....	18.5	24.0	18.0	46.0	5.0	42.0	7.0	1.5	1.0	10.0	2.0	3.0
18.....	15.0	22.0	18.0	48.5	4.0	42.0	6.0	1.5	1.0	7.5	2.0	3.0
19.....	11.5	18.5	18.5	51.0	3.5	41.5	5.0	1.5	1.0	5.0	2.5	3.0
20.....	10.0	16.0	26.0	51.5	3.5	41.5	6.5	1.5	.5	4.0	2.5	6.0
21.....	8.5	14.0	30.0	52.0	3.5	41.5	7.0	1.0	.5	3.5	3.0	5.0
22.....	8.0	15.0	32.0	52.0	3.5	41.0	8.0	1.0	.5	3.0	3.5	5.0
23.....	8.0	17.5	34.0	51.5	3.5	41.0	9.0	2.0	.5	2.0	3.5	8.5
24.....	7.0	18.0	36.5	51.0	4.0	42.5	8.0	1.5	.5	2.0	3.5	10.0
25.....	6.5	.....	37.5	49.5	4.5	42.5	6.0	1.5	.5	3.0	6.5	11.5
26.....	6.0	17.0	38.0	47.5	5.0	43.5	5.5	1.5	.5	4.0	8.5	11.5
27.....	5.0	16.0	39.0	47.0	6.0	43.5	5.0	5.0	.5	4.5	7.5	12.5
28.....	3.0	17.0	38.5	46.5	7.0	43.5	5.5	4.5	.5	7.5	7.5	12.0
29.....	3.0	.....	35.0	46.0	7.0	44.0	14.0	4.0	.5	9.0	8.0	10.0
30.....	3.0	.....	33.0	44.5	6.0	44.5	14.5	3.0	.5	8.0	8.5	10.0
31.....	3.0	.....	30.0	.....	5.0	.....	14.5	2.0	.....	7.5	.....	10.5

The following discharge measurements were made during 1901 by K. T. Thomas:

Jan. 31—Gage height, 12.70 feet; discharge, 13,738 second-feet.

March 14—Gage height, 21.10 feet; discharge, 23,824 second-feet.

June 28—Gage height, 1.00 feet; discharge, 1,496 second-feet.

Nov. 13—Gage height, 0.70 feet; discharge, 1,290 second-feet.

*Daily gage height in feet of Tombigbee River near Epes, Ala., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	10.0	12.0	6.5	13.0	7.5	5.5	1.0	.....	5.5	1.5	0.7	1.7
2.....	10.0	10.5	6.5	13.5	7.0	6.0	1.0	.....	4.0	6.0	0.7	1.7
3.....	9.0	16.8	9.0	15.5	6.0	7.0	1.0	.....	3.5	2.5	0.7	1.7
4.....	8.0	21.5	9.0	18.0	5.0	9.0	1.0	.....	2.5	1.5	0.0	3.5
5.....	7.0	25.5	8.5	17.0	4.5	9.0	1.0	.....	2.0	1.5	0.0	2.0
6.....	6.0	28.5	7.0	16.0	5.0	11.0	1.0	.....	2.0	1.5	0.0	2.0
7.....	5.5	27.5	6.5	15.0	4.0	12.0	1.0	.....	1.5	1.5	0.0	2.0
8.....	5.0	29.0	6.5	14.0	3.5	10.0	1.0	.....	1.5	1.0	0.0	3.5
9.....	4.5	30.5	6.5	11.0	3.5	8.0	1.0	.....	1.5	1.0	0.7	5.9
10.....	4.0	31.0	13.0	9.5	3.0	6.5	1.0	.....	1.5	1.0	0.7	6.5
11.....	18.0	31.0	18.0	8.0	3.0	6.5	0.5	.....	1.5	1.0	0.7	5.5
12.....	29.5	31.0	19.5	7.5	3.5	4.5	0.5	.....	1.5	1.0	0.7	4.7
13.....	32.0	30.0	20.0	9.0	4.0	4.0	0.5	.....	1.0	1.0	0.7	4.2
14.....	35.0	29.5	24.5	8.0	7.0	3.5	0.5	.....	1.0	1.0	0.7	13.0
15.....	36.0	28.5	25.5	7.0	8.0	3.5	0.5	.....	1.0	1.0	0.7	19.5
16.....	38.0	26.0	26.5	7.0	8.5	3.5	0.5	8.5	1.5	1.0	0.7	22.0
17.....	39.0	23.0	26.5	6.5	8.0	3.0	0.5	15.0	2.0	1.0	1.0	23.0
18.....	39.5	16.0	26.0	20.0	7.0	3.0	0.5	20.0	2.5	1.0	1.7	22.5
19.....	40.0	12.0	25.0	28.0	6.5	2.5	0.5	22.5	6.0	1.0	1.7	22.0
20.....	40.5	1.00	29.0	29.5	9.0	2.0	1.0	23.0	.....	1.0	1.5	20.0
21.....	39.0	8.5	20.0	30.0	12.0	2.0	1.0	24.0	7.5	1.0	1.5	18.0
22.....	38.0	8.0	17.0	29.5	12.0	2.0	1.0	24.5	7.5	1.0	1.7	14.0
23.....	34.5	7.5	12.5	28.5	11.5	1.5	1.0	26.0	7.0	1.0	1.7	12.0
24.....	29.0	7.0	1.0	28.0	11.0	1.2	1.0	26.5	5.5	0.5	1.8	9.0
25.....	24.5	6.5	1.0	27.0	10.0	1.0	1.0	26.0	4.0	0.5	1.8	8.0
26.....	20.0	6.5	13.5	24.0	8.0	1.0	1.0	25.0	3.0	0.5	2.0	7.0
27.....	16.0	6.5	13.5	18.0	6.0	1.0	0.5	23.0	2.0	0.5	2.0	7.0
28.....	15.0	6.5	11.5	13.0	6.5	1.0	0.5	19.0	1.5	0.5	2.0	12.0
29.....	14.0	.....	10.0	11.0	6.5	1.0	0.2	12.0	1.5	0.5	1.9	20.0
30.....	13.0	.....	9.0	8.0	6.5	1.0	0.2	6.5	1.5	0.5	1.8	26.0
31.....	12.7	.....	13.5	.....	7.0	.....	0.2	5.5	.....	0.5	.....	27.0

*Rating table for Tombigbee River at Epes, Ala., for 1900-1901..*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second-ft.	Feet.	Second-ft.	Feet.	Second -ft.	Feet.	Second ft.
0.2	810	5.4	5,208	11.0	11,700	16.6	18,420
0.1	840	5.5	5,308	11.1	11,820	16.7	18,540
0.0	880	5.6	5,409	11.2	11,940	16.8	18,660
0.1	830	5.7	5,511	11.3	12,060	16.9	18,780
0.2	985	5.8	5,613	11.4	12,180	17.0	18,900
0.3	1,043	5.9	5,716	11.5	12,300	17.1	19,020
0.4	1,103	6.0	5,820	11.6	12,420	17.2	19,140
0.5	1,164	6.1	5,925	11.7	12,540	17.3	19,260
0.6	1,226	6.2	6,030	11.8	12,660	17.4	19,380
0.7	1,289	6.3	6,136	11.9	12,780	17.5	19,500
0.8	1,353	6.4	6,243	12.0	12,900	17.6	19,620
0.9	1,418	6.5	6,350	12.1	13,020	17.7	19,740
1.0	1,484	6.6	6,458	12.2	13,140	17.8	19,860
1.1	1,551	6.7	6,566	12.3	13,260	17.9	19,980
1.2	1,619	6.8	6,675	12.4	13,380	18.0	20,100
1.3	1,688	6.9	6,785	12.5	13,500	18.1	20,220
1.4	1,758	7.0	6,900	12.6	13,620	18.2	20,340
1.5	1,829	7.1	7,020	12.7	13,740	18.3	20,460
1.6	1,903	7.2	7,140	12.8	13,860	18.4	20,580
1.7	1,976	7.3	7,260	12.9	13,980	18.5	20,700
1.8	2,050	7.4	7,380	13.0	14,100	18.6	20,820
1.9	2,125	7.5	7,500	13.1	14,220	18.7	20,940
2.0	2,200	7.6	7,620	13.2	14,340	18.8	21,060
2.1	2,276	7.7	7,740	13.3	14,460	18.9	21,180
2.2	2,353	7.8	7,860	13.4	14,580	19.0	21,300
2.3	2,431	7.9	7,980	13.5	14,700	19.1	21,420
2.4	2,510	8.0	8,100	13.6	14,820	19.2	21,540
2.5	2,590	8.1	8,220	13.7	14,940	19.3	21,660
2.6	2,671	8.2	8,340	13.8	15,060	19.4	21,780
2.7	2,753	8.3	8,460	13.9	15,180	19.5	21,900
2.8	2,835	8.4	8,580	14.0	15,300	19.6	22,020
2.9	2,918	8.5	8,700	14.1	15,420	19.7	22,140
3.0	3,002	8.6	8,820	14.2	15,540	19.8	22,260
3.1	3,087	8.7	8,940	14.3	15,660	19.9	22,380
3.2	3,172	8.8	9,060	14.4	15,780	20.0	22,500
3.3	3,258	8.9	9,180	14.5	15,900	20.1	22,620
3.4	3,345	9.0	9,300	14.6	16,020	20.2	22,740
3.5	3,432	9.1	9,420	14.7	16,140	20.3	22,860
3.6	3,520	9.2	9,540	14.8	16,260	20.4	22,980
3.7	3,609	9.3	9,660	14.9	16,380	20.5	23,100
3.8	3,698	9.4	9,780	15.0	16,500	20.6	23,220
3.9	3,788	9.5	9,900	15.1	16,620	20.7	23,340
4.0	3,878	9.6	10,020	15.2	16,740	20.8	23,460
4.1	3,969	9.7	10,140	15.3	16,860	20.9	23,580
4.2	4,060	9.8	10,260	15.4	16,980	21.0	23,700

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second-ft.	Feet.	Second-ft.	Feet.	Second-ft.	Feet.	Second-feet.
4.3	4,152	9.9	10,380	15.5	17,100	21.1	23,820
4.4	4,245	10.0	10,500	15.6	17,220	21.2	23,940
4.5	4,338	10.1	10,620	15.7	17,340	21.3	24,060
4.6	4,432	10.2	10,740	15.8	17,460	21.4	24,180
4.7	4,527	10.3	10,860	15.9	17,580	21.5	24,300
4.8	4,622	10.4	10,980	16.0	17,700	21.6	24,420
4.9	4,718	10.5	11,100	16.1	17,820	21.7	24,540
5.0	4,815	10.6	11,220	16.2	17,940	21.8	24,660
5.1	4,912	10.7	11,340	16.3	18,060	21.9	24,780
5.2	5,010	10.8	11,460	16.4	18,180	22.0	24,900
5.3	5,109	10.9	11,580	16.5	18,300		

NOTE.—This table applied to the foregoing "Daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Tombigbee River at Epes, Ala.*

[Drainage area, 8,830 square miles.]

Month.	Discharge in second-feet.			Run-off.	
	Maxi-mum.	Mini-mum.	Mean.	Second-feet per square mile.	Depth in inches.
1901.					
January .....	47,100	3,878	25,579	2.90	3.34
February .....	35,700	6,350	20,999	2.38	2.48
March .....	30,300	6,350	16,198	1.83	2.11
April .....	34,500	6,350	18,102	2.05	2.29
May .....	12,900	3,002	6,880	.78	.90
June .....	12,900	1,484	4,585	.52	.58
July .....	1,484	810	1,295	.15	.17
August 16-31 .....			21,541	2.44	1.41
September .....	7,500	1,484	3,205	.36	.40
October .....	5,820	1,164	1,633	.18	.21
November .....	2,200	880	1,550	.18	.20
December .....	30,900	1,960	12,249	1.39	1.60

*Minimum monthly discharge of Tombigbee River at Epes, Ala., with corresponding net horse power per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

[Drainage area, 8,830 square miles.]

	1900.			1901.		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot fall.	No. of days duration of minimum.
January .....	3,002	273	4	3,878	353	1
February .....	3,002	273	4	6,350	577	4
March .....	14,100	1,282	1	6,350	577	5
April .....	6,350	577	1	6,350	577	1
May .....	3,432	312	5	3,002	273	2
June .....	5,820	529	1	1,484	135	6
July .....	4,815	438	2	810	74	3
August .....	1,484	135	2	.....	.....	.....
September .....	1,164	106	11	1,484	135	3
October .....	1,164	106	9	1,164	106	8
November .....	2,200	200	4	880	80	5
December .....	3,002	273	5	1,960	178	3

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "net H. P. per foot of fall" in this table for that month.

### 3. TRIBUTARIES.

There are several large creeks in Marion and Lamar Counties that flow into Mississippi, and enter the Tombigbee River near Columbus. One of these, the Buttahatchee Creek, in Marion County, has numerous rapids, especially near the crossing of the Military Road.

Luxapalila Creek, in Lamar County, has two prongs that are both good power streams. They come together before the creek enters Mississippi, making Big Luxapalila Creek, which enters the Tombigbee at Columbus, Miss. The following measurements have been made on this stream at Columbus, Miss.:

1901.

March 11—Gage height, 8.20 feet; discharge, 2,459 second-feet.

April 16—Gage height, 4.45 feet; discharge, 873 second-feet.

June 26—Gage height, 1.90 feet; discharge, 109 second-feet.

Oct. 31—Gage height, 2.00 feet; discharge, 126 second-feet.



## CHAPTER VII.

### 1. TENNESSEE RIVER AT CHATTANOOGA, TENNESSEE.

This river, after passing Chattanooga, enters Alabama. It then makes a bend to the west and later to the north, returning to Tennessee. Flowing through this State and Kentucky, it empties into the Ohio 50 miles above Cairo. In 1879 a gage was established at Chattanooga, Tennessee, at the foot of Look-out street, just below Chattanooga Island, by the Signal Corps of the United States Army, which has been in charge of the Weather Bureau since July 1, 1891. The drainage area above this station is 21,382 square miles, and is mapped on Morristown, Greenville, Roan Mountain, London, Knoxville, Mount Guyot, Asheville, Murphy, Briceville, Standingstone, Wartburg, Pikeville, Maynardville, Cumberland Gap, Jonesville, Estillville, Bristol, Whitesburg, Grundy, Abington, Tazewell, Pocahontas, Wytheville, Cranberry, Morganton, Mount Mitchell, Saluda, Pisgah, Como, Nantahala, Walhalla, Dahlonega, Ellijay, Dalton, Cleveand, Ringgold, Kingston, and Chattanooga atlas sheets. The gage is on an incline railroad iron for about 20 feet of its lower portion. Above this it is a vertical rod, bolted to the rock bluff forming the river bank. The zero of the gage is 630.4 feet above sea level. Measurements are made from the Hamilton County steel highway bridge at the foot of Walnut street, a short distance below the gage. Gage heights are obtained from L. M. Pindell, United States Weather Bureau observer. During the year 1900 a new gage on the same datum was established. It is a vertical rod bolted to the south side of the third stone pier from the south end of the bridge.

*Daily gage height of Tennessee River at Chattanooga, Tenn, for 1890.*

[Furnished by L. M. Pindell, observer in charge, United States Weather Bureau.]

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	4.9	8.0	40.2	10.0	7.5	6.1	3.2	5.8	7.6	9.4	6.8	2.4
2.....	5.2	7.3	42.5	9.8	7.1	6.6	2.9	5.2	5.6	8.1	6.6	2.4
3.....	5.1	7.3	41.0	9.4	6.7	5.3	2.9	4.7	5.7	7.7	5.7	2.3
4.....	5.0	7.4	34.4	9.7	6.8	5.3	3.0	4.7	4.0	7.2	5.3	2.3
5.....	5.0	7.2	23.0	12.2	7.0	6.2	3.0	4.8	3.6	6.4	5.0	2.3
6.....	4.8	7.8	15.1	14.0	7.5	5.3	3.0	4.5	3.2	5.7	4.6	2.5
7.....	4.7	8.3	14.2	13.6	8.9	5.2	3.0	5.2	3.6	5.2	4.4	2.8
8.....	4.6	11.6	14.4	13.4	9.1	4.9	2.9	5.7	3.2	5.2	4.3	2.9
9.....	4.7	19.3	12.8	11.9	8.7	4.7	2.8	6.6	2.8	5.5	4.0	7.1
10.....	4.9	20.4	11.2	10.5	8.7	4.5	2.7	7.6	2.8	5.3	3.9	8.1
11.....	4.7	17.8	10.0	9.6	8.1	4.5	2.7	7.2	3.0	4.8	3.8	8.2
12.....	4.6	14.7	9.2	8.7	7.7	4.4	3.0	6.3	3.4	4.6	3.6	7.7
13.....	4.6	12.0	8.6	8.0	7.2	4.0	2.7	5.8	3.6	4.4	3.5	7.4
14.....	4.9	10.0	8.7	7.5	6.7	3.9	2.5	5.2	4.6	4.2	3.4	6.4
15.....	4.6	9.8	9.7	7.1	6.6	4.0	2.3	4.8	4.0	3.9	3.3	3.7
16.....	5.4	9.6	13.7	6.9	7.8	4.1	2.1	4.2	4.0	3.7	3.2	4.2
17.....	7.2	9.0	15.1	7.1	9.3	4.0	2.0	3.8	3.7	3.7	3.2	3.9
18.....	9.2	9.3	14.9	9.4	8.6	3.9	2.2	3.7	4.0	3.7	3.0	3.9
19.....	8.2	8.5	13.0	16.6	7.9	3.7	4.7	3.3	5.3	4.2	3.2	4.0
20.....	7.5	7.8	11.7	20.4	8.8	3.6	4.7	3.1	3.7	4.0	3.1	4.0
21.....	7.0	7.3	12.4	18.2	10.7	3.7	4.1	2.6	3.7	4.0	3.1	3.9
22.....	9.6	7.1	14.0	14.3	11.9	3.8	3.5	2.5	5.8	3.8	3.1	3.9
23.....	13.0	7.2	20.0	11.3	11.9	3.7	3.2	3.8	3.5	3.3	2.9	3.9
24.....	12.3	7.4	26.5	9.6	11.6	3.8	3.2	4.0	3.3	7.2	2.8	3.9
25.....	11.7	12.1	27.2	8.7	9.2	4.0	4.1	3.3	3.4	8.8	2.8	4.3
26.....	10.0	18.7	26.0	8.4	7.8	4.0	5.9	3.8	4.0	9.2	2.7	4.6
27.....	8.3	26.4	21.4	8.4	7.4	3.9	7.5	3.6	4.3	9.5	2.6	9.4
28.....	7.3	34.8	15.4	8.5	8.0	3.9	7.7	3.0	4.0	8.6	2.6	12.5
29.....	6.6	.....	13.0	8.4	8.1	3.5	7.0	3.8	3.8	7.6	2.5	12.9
30.....	7.7	.....	11.9	8.1	7.4	3.1	6.3	5.4	5.3	7.1	2.4	12.4
31.....	8.3	.....	10.7	.....	6.8	.....	6.2	6.5	.....	7.0	.....	9.3

*Daily gage height of Tennessee River at Chattanooga, Tenn, for 1891.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	7.7	9.8	18.6	15.4	5.9	5.8	3.9	8.3	5.4	1.8	1.3	3.7
2.....	7.8	13.2	17.5	16.3	5.7	5.6	3.8	10.3	4.7	1.9	1.3	3.2
3.....	9.9	16.1	15.6	16.3	5.6	5.3	3.6	15.1	4.3	1.9	1.3	2.9
4.....	14.1	19.8	13.3	15.7	5.5	4.8	3.6	16.4	4.2	1.8	1.2	2.8
5.....	15.5	22.6	15.4	15.1	5.2	4.4	3.6	12.0	5.1	1.8	1.2	5.6
6.....	15.2	21.6	20.0	12.6	5.1	4.1	3.6	8.7	5.1	1.7	1.2	6.1
7.....	10.4	18.3	23.6	11.6	4.9	3.9	3.4	6.9	5.2	1.7	1.2	6.6
8.....	8.2	16.9	29.1	10.8	4.7	4.1	3.4	6.8	5.2	1.6	1.2	8.6
9.....	7.1	14.5	34.5	9.8	4.6	4.6	3.3	5.1	5.8	1.7	1.3	10.8
10.....	6.3	21.0	37.5	9.6	4.5	4.7	4.5	4.6	4.9	1.7	1.2	10.3
11.....	6.9	27.8	38.9	9.8	4.4	5.5	5.1	4.4	4.4	1.8	1.5	10.3
12.....	8.9	34.3	37.6	9.9	4.3	7.0	4.4	4.0	3.9	1.8	1.7	8.5
13.....	10.7	36.5	33.5	10.6	4.2	6.5	3.9	4.0	3.6	1.7	2.7	6.8
14.....	10.0	37.5	27.0	11.3	4.1	5.7	3.5	3.9	3.5	1.7	3.6	5.7
15.....	9.2	35.5	22.2	12.2	4.2	5.5	3.1	3.8	3.5	1.7	4.1	5.1
16.....	7.3	29.0	19.8	10.8	4.2	5.7	2.9	3.6	3.5	1.6	3.5	5.0
17.....	7.8	21.1	18.1	9.4	4.5	5.5	2.8	3.5	3.5	1.6	2.9	6.2
18.....	7.5	19.7	15.3	8.4	4.7	6.1	2.7	3.4	3.2	1.5	2.5	5.3
19.....	7.5	18.2	13.5	8.2	4.7	6.8	4.1	3.0	2.9	1.5	2.4	4.8
20.....	7.6	16.5	12.3	7.9	4.6	7.3	5.0	3.0	2.7	1.5	2.5	4.5
21.....	7.5	15.5	11.3	7.9	4.5	6.8	4.5	3.4	2.6	1.5	2.3	4.2
22.....	8.2	18.8	10.8	7.6	4.3	6.8	4.0	4.0	2.5	1.5	2.4	4.0
23.....	12.5	24.0	10.7	7.4	4.1	6.5	3.8	4.6	2.4	1.5	3.0	3.8
24.....	15.3	27.7	10.8	7.4	4.0	7.1	3.6	5.5	2.3	1.5	4.6	3.7
25.....	14.0	29.0	10.6	7.5	3.8	7.4	3.5	5.6	2.2	1.5	6.2	4.1
26.....	13.6	26.7	10.4	7.5	3.6	7.6	3.5	7.7	2.2	1.5	6.7	4.9
27.....	11.2	20.6	10.5	7.4	4.0	6.2	3.5	8.2	2.1	1.5	6.3	8.1
28.....	9.7	19.0	14.1	7.2	4.1	4.9	3.6	8.1	2.0	1.5	5.6	10.2
29.....	7.9	.....	13.6	6.5	4.1	4.3	3.7	7.1	1.9	1.4	4.7	9.6
30.....	7.9	.....	13.0	6.2	4.7	4.1	3.8	6.4	1.9	1.4	4.0	8.4
31.....	8.9	.....	13.1	.....	5.3	.....	5.7	6.1	.....	1.4	.....	7.8



# WATER-POWERS OF ALABAMA.

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*Daily gage height of Tennessee River at Chattanooga, Tenn, for 1892.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	6.6	6.5	5.7	9.1	8.7	5.6	6.6	4.2	2.1	2.2	1.1	4.1
2.....	6.6	6.2	5.6	8.3	8.2	5.4	5.6	4.1	1.8	2.0	1.2	4.0
3.....	8.1	6.0	5.5	7.4	7.6	5.5	5.5	4.2	1.7	1.9	1.2	3.8
4.....	8.8	5.8	5.3	6.8	7.3	5.8	5.4	4.0	1.5	1.8	1.6	3.7
5.....	8.7	5.5	5.1	6.5	7.4	8.8	6.6	4.1	1.3	1.7	2.0	3.6
6.....	8.4	5.4	5.0	8.5	7.0	9.2	8.9	4.9	2.2	1.6	2.3	3.6
7.....	9.0	5.3	4.9	21.7	6.6	9.3	11.2	4.2	2.1	1.6	2.4	3.3
8.....	9.8	5.8	5.0	31.6	6.3	8.7	11.8	3.8	2.0	1.5	2.4	3.2
9.....	10.0	8.1	6.0	34.2	6.2	8.6	10.1	3.5	2.0	1.5	2.9	3.2
10.....	9.3	11.6	7.1	34.3	5.9	8.3	8.6	3.3	1.9	1.5	4.4	3.0
11.....	8.1	11.3	8.0	31.0	5.5	7.8	9.0	3.3	2.0	1.5	5.9	2.9
12.....	8.3	10.5	7.9	26.6	5.7	8.0	9.5	3.1	1.8	1.5	6.6	2.7
13.....	11.2	8.9	7.6	18.0	5.7	8.1	9.4	3.5	2.1	1.4	7.0	2.6
14.....	22.9	7.7	7.6	12.9	5.5	7.8	8.9	3.6	2.1	1.4	4.4	2.6
15.....	32.9	7.2	6.8	11.7	5.3	6.0	8.7	3.7	2.1	1.4	4.2	3.3
16.....	37.1	7.4	6.2	10.9	5.2	5.3	8.5	3.5	3.1	1.4	4.8	3.4
17.....	37.9	8.0	5.9	10.0	5.1	4.8	8.4	3.1	4.5	1.4	5.6	5.3
18.....	35.2	7.9	6.5	9.4	4.8	4.4	8.1	2.9	4.1	1.3	6.2	8.1
19.....	26.3	7.5	7.5	8.8	5.1	4.4	7.5	2.8	3.5	1.3	6.4	8.7
20.....	18.7	7.7	8.2	12.3	5.5	4.3	7.2	2.8	3.0	1.2	6.2	8.4
21.....	19.0	7.9	8.2	16.2	6.1	7.0	6.9	3.0	2.8	1.2	5.5	9.1
22.....	19.0	8.9	7.8	16.3	6.5	7.8	6.2	2.9	2.8	1.2	4.8	9.8
23.....	17.4	8.9	7.6	15.5	6.9	7.4	5.6	2.9	2.6	1.2	4.4	8.9
24.....	14.9	8.4	8.4	14.8	7.1	7.1	5.4	3.0	2.3	1.2	4.2	7.8
25.....	12.2	7.9	9.6	13.5	7.6	6.9	5.2	2.7	2.3	1.2	3.9	6.7
26.....	10.5	7.5	9.5	13.7	7.8	6.8	5.0	2.8	2.6	1.1	3.4	5.8
27.....	9.7	6.7	10.0	13.6	6.7	6.7	4.6	3.2	2.6	1.1	3.1	5.2
28.....	8.5	6.4	10.6	10.4	6.2	7.3	4.5	3.6	3.1	1.1	3.0	4.8
29.....	7.7	5.9	10.3	9.1	5.8	7.5	4.3	4.4	2.6	1.1	3.0	4.3
30.....	6.9	.....	9.7	8.8	5.7	7.2	4.1	3.8	2.4	1.1	3.6	3.9
31.....	6.8	.....	9.4	.....	5.6	.....	3.8	3.3	.....	1.1	.....	3.3

*List of discharge measurements made on Tennessee River at Chattanooga, Tennessee.\**

No.	Date.	Hydrographer.	Meter No	Gage height, feet.	Area of section, (square feet.)	Mean velocity (ft. per sec.)	Discharge (second-feet.)
1	March 15	L. M. Pindell ...	2	10.3	17,971	.....	63,039
2	March 16	do .....	2	9.2	16,847	.....	58,310
3	April 3	do .....	2	5.1	12,427	.....	32,628
4	April 4	do .....	2	5.1	12,427	.....	32,643
5	May 5	do .....	2	26.0	36,648	3.93	156,187
6	May 8	do .....	2	26.0	36,825	3.73	151,660
7	May 9	do .....	2	16.0	24,913	3.75	96,979
8	May 17	do .....	2	9.9	17,971	3.39	65,867
9	May 18	do .....	2	10.4	18,539	3.29	67,883

\*Meter submerged at one-half depth.

*Daily gage height of Tennessee River at Chattanooga, Tenn., for 1893.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	3.4	10.4	8.4	5.5	10.2	7.4	3.9	2.6	8.4	2.6	3.1	2.5
2.....	3.8	12.1	9.1	5.3	9.6	8.0	3.8	2.5	6.8	2.5	3.0	3.9
3.....	4.7	10.6	8.9	5.1	11.0	10.0	4.2	3.1	5.0	2.4	2.8	3.7
4.....	5.7	8.6	8.9	5.1	18.4	8.1	5.2	3.5	6.2	2.4	2.7	4.1
5.....	5.6	8.0	8.7	5.1	24.5	6.6	4.8	3.3	6.0	2.6	2.6	4.8
6.....	5.3	7.7	9.0	5.2	28.2	8.3	3.9	3.2	4.9	2.5	2.5	4.7
7.....	5.2	7.0	9.1	5.4	30.0	16.0	3.8	4.9	4.3	2.9	2.4	4.7
8.....	4.7	6.5	8.8	5.4	28.2	20.7	3.5	5.0	3.5	3.1	2.4	4.5
9.....	4.0	6.1	8.8	5.1	18.0	19.1	3.4	4.1	3.2	2.9	2.4	4.1
10.....	3.8	6.2	9.4	5.1	12.8	15.2	3.6	3.8	2.8	2.9	2.4	4.0
11.....	3.4	8.5	11.1	5.1	11.7	11.8	3.4	3.3	2.8	2.6	2.5	3.7
12.....	2.9	14.7	11.7	5.0	10.4	8.9	3.4	3.0	3.7	2.5	4.8	3.2
13.....	2.9	21.8	11.5	4.8	9.4	7.8	3.3	2.7	5.8	2.4	3.8	3.1
14.....	*	22.6	12.0	10.2	8.8	6.8	3.2	2.5	10.9	2.5	3.6	3.6
15.....	*	22.6	10.6	12.1	8.1	6.5	3.0	2.8	12.7	2.0	3.5	2.9
16.....	*	21.3	9.5	10.4	7.8	6.2	2.8	2.9	9.6	1.7	3.0	3.0
17.....	*	22.6	8.4	8.6	9.4	5.6	2.7	3.8	8.0	9.6	2.8	3.2
18.....	*	22.4	7.6	7.4	10.4	5.4	2.8	5.2	7.0	6.4	2.7	3.5
19.....	*	32.4	7.0	6.5	8.9	5.3	3.0	4.9	6.1	5.7	2.5	3.8
20.....	*	32.4	6.7	6.4	7.7	5.2	3.2	4.0	5.1	5.2	2.6	3.9
21.....	*	32.4	6.3	7.2	7.4	5.4	3.5	2.9	4.2	4.1	2.6	3.9
22.....	*	23.5	6.0	7.2	6.7	5.6	3.6	2.6	3.6	3.4	2.6	3.5
23.....	2.9	18.2	5.8	7.1	6.1	5.7	4.6	2.4	3.4	3.1	2.5	3.3
24.....	3.1	12.3	5.7	6.8	5.7	5.4	5.2	2.3	3.2	2.8	2.5	3.1
25.....	3.1	10.4	6.4	6.7	5.4	5.9	5.5	2.2	3.0	3.1	2.4	2.9
26.....	3.4	9.3	6.8	6.0	5.2	5.6	3.7	1.9	2.9	3.3	2.5	2.8
27.....	3.7	8.4	6.8	5.7	5.0	5.3	3.4	1.8	2.7	4.9	2.6	2.7
28.....	3.8	8.2	6.3	7.0	4.6	5.1	2.9	1.6	2.6	4.6	2.8	2.6
29.....	4.4	.....	5.9	9.5	5.4	4.7	3.7	1.6	2.4	4.0	2.7	2.6
30.....	5.3	.....	5.8	10.4	6.5	4.1	2.6	1.7	2.5	3.5	2.5	2.7
31.....	7.1	.....	5.7	.....	7.4	.....	2.6	1.6	.....	3.2	.....	3.1

\*Frozen at gage.

*Daily gage height of Tennessee River at Chattanooga, Tenn., for 1894.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	2.9	5.1	7.7	5.0	3.9	3.8	4.4	2.9	4.0	0.9	1.4	1.0
2.....	3.4	5.0	8.2	4.8	3.8	3.6	4.0	2.9	3.8	.9	1.7	1.0
3.....	3.8	4.9	9.4	5.4	3.7	3.5	3.7	2.9	3.0	1.0	2.3	.9
4.....	3.9	5.5	9.7	5.3	3.7	3.4	4.4	2.8	2.6	1.5	1.6	.9
5.....	3.5	21.9	9.5	6.8	3.6	3.3	4.2	2.9	2.1	1.8	1.4	.9
6.....	3.1	25.5	9.3	6.9	3.5	3.2	3.7	3.0	2.0	1.8	1.5	.9
7.....	4.9	23.9	8.5	7.2	3.4	2.9	3.2	2.9	1.8	1.5	1.5	.9
8.....	6.1	19.7	8.2	7.4	3.3	2.8	3.3	2.9	1.7	1.3	1.3	.9
9.....	9.3	16.1	7.9	6.6	3.4	2.6	3.1	3.0	1.5	1.1	1.2	1.1
10.....	9.0	16.0	7.2	5.7	3.3	2.5	3.3	2.6	1.4	1.0	1.1	1.2
11.....	8.5	16.7	6.9	5.9	3.2	2.5	3.7	2.3	1.4	.8	1.0	1.6
12.....	7.9	15.4	6.6	7.2	4.7	2.4	3.3	2.1	1.4	.9	1.0	3.8
13.....	8.3	15.2	7.7	8.5	5.1	2.3	2.7	1.9	1.3	1.2	.9	3.5
14.....	8.0	14.1	7.2	7.8	4.8	2.3	2.4	1.8	1.2	1.9	.8	11.1
15.....	7.8	12.2	7.0	7.2	4.3	2.2	2.1	2.0	1.5	2.4	.8	11.2
16.....	7.8	10.3	6.9	6.9	4.0	2.1	1.9	3.6	1.8	2.1	.8	19.8
17.....	7.1	9.5	6.8	6.3	4.1	2.0	1.8	4.6	1.8	1.7	.8	3.5
18.....	7.2	8.6	7.3	6.5	5.2	2.1	2.8	3.5	2.0	1.4	.7	6.6
19.....	6.3	8.4	7.4	5.0	5.0	2.4	2.4	3.0	2.0	1.1	.8	4.7
20.....	6.0	8.3	7.7	5.1	5.4	2.5	2.4	3.1	1.6	1.0	.9	4.2
21.....	5.3	8.5	7.1	4.9	5.6	2.6	3.3	3.6	1.5	.9	.9	3.6
22.....	5.0	8.7	8.8	4.8	6.2	2.5	3.7	3.7	1.5	.8	.9	3.2
23.....	5.0	8.8	8.7	4.7	6.8	2.3	3.7	4.0	1.8	.8	1.0	2.8
24.....	5.2	8.2	8.1	4.6	6.9	2.2	3.4	3.6	1.8	.8	1.0	2.7
25.....	5.3	7.9	7.7	4.5	7.1	2.2	4.0	3.0	1.6	.8	1.1	2.5
26.....	5.2	7.7	7.3	4.3	6.7	2.5	1.4	2.6	1.3	.8	1.2	2.4
27.....	5.4	7.7	7.0	4.2	6.0	2.6	3.9	2.2	1.1	.7	1.2	4.3
28.....	5.4	7.7	6.5	4.1	5.6	2.7	3.8	2.7	1.0	.7	1.1	6.9
29.....	5.1	.....	5.9	4.0	5.1	2.9	3.6	2.4	.9	.7	1.1	8.4
30.....	5.0	.....	5.7	4.0	4.7	4.3	3.3	2.7	.8	1.0	1.1	7.9
31.....	4.9	.....	5.2	.....	4.2	.....	3.3	4.3	.....	1.1	.....	5.3

*Daily gage height of Tennessee River at Chattanooga, Tenn., for 1895.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	4.7	7.6	6.8	7.8	6.0	5.6	3.2	4.4	3.2	0.9	1.1	1.4
2.....	3.9	7.2	7.3	7.4	5.8	5.2	3.4	4.1	3.4	.8	1.2	1.5
3.....	8.3	7.2	12.1	6.8	5.5	4.8	3.8	3.8	3.2	.8	1.3	1.4
4.....	8.2	7.5	18.2	6.5	5.4	4.5	4.0	3.5	3.1	.8	1.6	1.4
5.....	8.1	7.6	19.9	6.3	5.7	4.2	4.0	3.4	2.9	.8	1.6	1.6
6.....	3.1	7.4	18.2	6.2	6.0	4.4	5.0	3.0	2.8	.8	1.3	1.6
7.....	3.3	6.9	13.4	6.0	6.5	4.6	5.0	3.2	2.8	.8	1.3	1.5
8.....	4.6	6.5	10.5	9.6	7.0	5.2	5.1	3.3	2.8	.8	1.2	1.4
9.....	10.9	6.4	9.2	10.7	8.2	5.1	5.5	3.3	2.5	.8	1.1	1.3
10.....	20.5	5.0	8.6	11.4	8.6	4.6	5.7	3.0	2.4	.9	1.2	1.3
11.....	28.5	4.0	8.1	13.0	9.0	4.2	5.1	3.0	2.2	1.0	1.3	1.4
12.....	32.1	3.3	7.5	12.5	8.8	3.8	4.4	2.9	2.4	1.0	1.7	1.5
13.....	31.2	4.2	7.8	10.4	8.9	3.6	3.8	2.2	2.5	1.0	2.1	1.8
14.....	28.3	*	8.0	8.8	9.5	2.5	3.4	2.9	2.4	1.0	2.4	1.9
15.....	19.5	4.3	8.7	7.9	9.0	3.4	3.2	2.8	2.3	.9	2.2	2.0
16.....	12.3	4.7	9.4	7.4	8.2	3.4	3.2	2.7	2.2	1.0	2.0	1.9
17.....	10.9	4.7	9.2	7.0	7.7	3.8	3.7	2.1	2.1	1.0	1.9	1.8
18.....	10.0	4.2	9.6	9.0	7.1	3.7	3.6	4.3	2.3	1.0	1.8	1.7
19.....	9.7	4.7	9.4	11.8	7.0	3.5	3.3	4.9	2.2	.9	1.7	1.6
20.....	9.1	4.6	8.9	11.8	7.2	3.2	3.0	5.7	2.0	.9	1.5	1.5
21.....	9.6	5.1	14.3	9.9	7.1	3.1	2.7	5.3	2.1	.8	1.3	1.6
22.....	10.2	5.6	20.6	8.6	6.7	3.3	2.7	6.1	1.9	.8	1.1	2.1
23.....	9.9	6.1	22.7	7.7	6.5	3.1	2.0	5.3	1.6	.7	1.3	3.3
24.....	9.1	6.7	22.0	7.1	5.8	3.0	2.4	4.9	1.4	.7	1.4	4.2
25.....	10.8	6.8	18.2	6.7	5.6	2.9	2.4	4.6	1.3	.7	1.3	4.6
26.....	10.8	6.5	13.0	6.3	5.6	2.9	2.9	4.7	1.3	.7	1.3	4.3
27.....	10.0	6.3	11.3	6.0	7.0	2.6	3.8	4.2	1.2	.7	1.3	4.2
28.....	9.3	6.3	10.6	6.0	7.5	2.5	10.2	3.7	1.1	.7	1.3	4.5
29.....	8.8	.....	9.5	5.9	7.4	2.5	10.4	3.5	1.0	.7	1.3	5.2
30.....	8.6	.....	8.9	5.9	6.7	2.6	7.4	3.7	0.9	.7	1.5	4.7
31.....	8.4	.....	8.4	.....	6.0	.....	5.3	3.4	.....	1.0	.....	4.7

\*Frozen.

*Rating table for Tennessee River at Chattanooga, Tennessee.*

[This table is applicable from Jan. 1, 1890, to Dec. 31, 1895.]

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
0.7	16,360	11.0	66,850	22.5	133,665	34.0	293,820
0.8	16,560	11.5	69,755	23.0	136,570	34.5	302,720
0.9	16,780	12.0	72,660	23.5	139,475	35.0	311,620
1.0	17,000	12.5	75,565	24.0	142,380	35.5	320,520
1.5	18,160	13.0	78,470	24.5	145,285	36.0	329,420
2.0	19,500	13.5	81,375	25.0	148,190	36.5	338,320
2.5	21,100	14.0	84,280	25.5	151,095	37.0	347,220
3.0	23,000	14.5	87,185	26.0	154,000	37.5	356,120
3.5	25,090	15.0	90,090	26.5	162,500	38.0	365,020
4.0	27,300	15.5	92,995	27.0	171,000	38.5	373,920
4.5	29,660	16.0	95,900	27.5	179,900	39.0	382,820
5.0	32,200	16.5	98,805	28.0	188,800	39.5	391,720
5.5	34,895	17.0	101,710	28.5	197,700	40.0	400,620
6.0	37,800	17.5	104,615	29.0	206,600	40.5	409,520
6.5	40,705	18.0	107,520	29.5	215,500	41.0	418,420
7.0	43,610	18.5	110,425	30.0	224,400	41.5	427,320
7.5	46,515	19.0	113,330	30.5	233,300	42.0	436,220
8.0	49,420	19.5	116,235	31.0	242,200	42.5	445,120
8.5	52,325	20.0	119,140	31.5	251,100	43.0	454,020
9.0	55,230	20.5	122,045	32.0	260,000	43.5	462,920
9.5	58,135	21.0	124,950	32.5	268,900	44.0	471,820
10.0	61,040	21.5	127,855	33.0	276,800	44.5	480,720
10.5	63,945	22.0	130,760	33.5	284,920		

NOTE—This table applied to the foregoing "daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Daily gage height of Tennessee River at Chattanooga, Tenn., for 1896.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	4.9	4.4	4.1	14.8	3.4	2.6	3.3	5.5	2.4	2.5	1.2	2.4
2.....	5.0	6.2	3.9	27.7	3.4	3.0	3.2	5.2	2.1	2.3	1.2	2.5
3.....	4.9	10.0	3.8	34.4	3.4	4.4	3.1	4.8	1.9	2.6	1.3	2.5
4.....	4.9	11.6	3.7	38.8	3.5	5.7	3.2	4.6	1.8	3.0	1.5	2.6
5.....	4.7	10.5	3.6	40.5	4.0	5.2	3.3	4.5	1.6	2.7	1.5	2.6
6.....	4.3	9.3	3.5	36.9	4.6	4.7	3.2	4.9	1.6	2.6	1.6	2.7
7.....	3.6	11.8	3.4	23.3	4.6	4.1	3.6	5.0	1.5	2.1	2.3	2.9
8.....	3.3	14.0	3.5	11.6	4.3	3.5	5.0	4.2	2.0	1.7	3.5	3.0
9.....	3.2	13.8	3.5	9.0	4.0	3.5	3.9	3.8	2.8	1.5	4.2	2.8
10.....	3.2	13.2	3.4	8.0	3.7	4.5	14.2	3.4	2.7	1.4	4.1	2.8
11.....	3.1	12.8	3.6	7.2	3.4	7.0	21.1	3.3	2.4	1.2	3.3	2.7
12.....	3.1	11.4	3.6	6.7	3.1	6.3	21.6	3.4	2.0	1.2	3.2	2.6
13.....	2.9	10.1	3.8	6.2	2.9	5.1	15.6	3.2	1.8	1.2	5.8	2.4
14.....	2.7	11.1	3.8	5.8	2.8	4.3	11.5	3.2	1.6	1.2	7.3	2.6
15.....	2.6	12.8	3.7	5.5	2.7	3.6	11.2	3.1	1.6	1.5	6.5	4.1
16.....	2.4	13.6	3.8	5.2	2.6	3.2	11.4	3.0	1.5	1.7	5.5	6.5
17.....	2.3	12.5	5.5	5.0	2.5	8.0	11.0	3.0	1.3	1.6	4.9	6.6
18.....	2.3	11.0	10.1	4.8	2.4	2.8	13.9	2.9	1.4	1.6	4.3	6.3
19.....	2.3	9.0	13.1	4.2	2.4	2.9	12.5	2.7	1.4	1.7	3.8	6.4
20.....	2.3	7.6	15.7	4.4	2.2	3.1	9.6	2.6	1.3	1.6	3.4	6.8
21.....	2.3	6.7	13.8	4.2	2.1	3.7	7.6	2.4	1.2	1.6	3.0	7.0
22.....	2.5	6.0	11.2	4.1	2.1	3.5	6.5	2.4	1.2	1.4	2.8	7.2
23.....	3.1	5.4	9.5	4.1	2.5	3.5	8.5	2.2	1.3	1.2	2.5	7.3
24.....	5.0	4.9	8.4	4.0	3.2	3.3	8.8	2.2	1.4	1.2	2.4	7.0
25.....	6.5	4.7	7.9	4.0	3.6	3.1	8.6	2.8	1.6	1.2	2.3	6.6
26.....	8.2	4.6	7.5	3.8	3.8	2.9	7.8	2.6	2.0	1.2	2.2	5.9
27.....	8.0	4.5	7.2	3.8	3.2	2.6	11.1	2.7	1.7	1.2	2.1	5.3
28.....	7.0	4.4	6.7	3.8	3.1	2.6	12.2	3.2	1.5	1.2	2.2	4.8
29.....	6.0	4.2	6.2	3.6	2.8	2.8	9.3	4.0	1.5	1.1	5.3	4.4
30.....	5.3	.....	5.8	3.6	2.7	3.0	7.2	3.6	2.7	1.1	9.4	3.7
31.....	4.8	.....	7.7	.....	2.5	.....	6.2	2.8	.....	1.3	.....	3.0

The following discharge measurements were made by Max Hall and others during 1897:

May 8—Gage height, 7.07 feet; discharge, 44,187 second-feet.  
 May 28—Gage height, 4.52 feet; discharge, 25,892 second-feet.  
 June 29—Gage height, 5.76 feet; discharge, 32,943 second-feet.  
 July 13—Gage height, 4.59 feet; discharge, 26,884 second-feet.  
 Sept. 7—Gage height, 1.67 feet; discharge, 10,313 second-feet.  
 Oct. 6—Gage height, 0.48 feet; discharge, 5,969 second-feet.  
 Nov. 16—Gage height, 0.83 feet; discharge, 5,552 second-feet.  
 Dec. 23—Gage height, 10.30 feet; discharge, 67,000 second-feet.

*Daily gage height of Tennessee River at Chattanooga, Tenn., for 1897.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	2.4	3.0	12.5	8.7	5.9	4.3	5.0	4.4	2.1	0.8	0.8	1.0
2.....	2.5	7.0	9.6	12.2	6.3	4.2	4.0	3.9	2.2	.8	.9	1.2
3.....	2.8	10.1	8.6	15.0	7.4	4.1	3.8	3.8	1.9	.7	1.0	1.3
4.....	2.6	10.5	9.0	16.0	9.6	4.1	3.4	3.6	1.8	.6	1.2	2.0
5.....	2.6	9.4	9.5	26.0	9.6	4.1	3.4	3.3	1.8	.5	1.2	3.3
6.....	2.7	8.3	12.1	30.4	8.5	4.1	4.0	3.5	1.7	.5	1.3	3.8
7.....	2.9	8.8	19.2	29.7	7.7	4.4	3.8	4.4	1.7	.5	1.4	3.9
8.....	3.0	10.7	25.1	25.4	7.2	4.4	3.8	4.2	1.6	.4	1.2	3.5
9.....	2.8	14.1	24.3	20.0	6.6	4.0	4.4	4.2	1.6	.4	1.2	2.9
10.....	2.8	15.5	21.3	16.0	6.2	5.2	4.0	5.6	1.4	.4	1.2	2.6
11.....	2.7	13.2	22.3	14.0	6.0	5.0	4.1	5.2	1.3	.5	1.1	2.4
12.....	2.6	10.8	28.4	26.0	6.2	5.7	4.5	4.6	1.2	.6	1.0	2.1
13.....	2.4	9.9	34.9	11.4	7.8	5.0	4.6	4.1	1.2	.9	1.0	1.8
14.....	2.6	10.0	37.9	10.3	18.4	4.3	4.2	3.5	1.1	1.4	.9	1.8
15.....	4.1	10.5	37.9	9.7	22.4	3.9	3.8	3.1	1.0	1.1	.8	2.5
16.....	6.5	10.7	37.0	9.8	20.3	3.6	3.6	2.8	1.0	1.2	.8	2.7
17.....	6.6	9.8	36.0	10.2	16.5	3.7	4.5	2.0	.9	1.2	.8	2.5
18.....	6.3	8.6	33.8	9.9	11.9	3.6	6.3	3.0	.9	1.2	.8	2.5
19.....	.64	7.6	29.6	9.3	9.1	3.3	6.1	3.4	.8	1.1	.8	2.6
20.....	6.8	7.0	29.6	8.8	7.7	3.3	5.6	3.0	.8	1.4	.7	3.4
21.....	7.0	7.0	32.4	8.1	6.9	4.1	6.7	3.0	.9	2.0	.7	4.5
22.....	7.2	8.3	33.3	7.5	6.4	5.0	6.1	3.4	.9	1.9	.7	7.1
23.....	7.3	18.2	30.9	7.0	5.9	4.8	5.8	3.1	.8	1.6	.7	10.2
24.....	7.0	25.2	25.0	6.7	5.6	5.3	6.0	3.8	.9	1.4	.7	9.3
25.....	6.6	31.6	18.1	6.4	5.3	5.5	3.4	3.4	.8	1.6	.7	7.7
26.....	5.9	34.3	14.2	6.2	5.1	6.2	3.7	2.9	.8	1.4	.7	6.4
27.....	5.3	33.8	12.2	6.0	4.8	5.4	12.3	2.8	.7	1.2	.7	5.6
28.....	4.8	23.6	10.8	6.1	4.6	5.5	8.7	2.8	.7	1.0	.7	5.0
29.....	4.4	.....	9.8	6.2	4.4	6.2	6.7	2.5	.7	.9	.7	4.5
30.....	3.7	.....	9.1	5.8	4.2	5.2	5.6	2.2	.8	.8	.9	4.0
31.....	3.0	.....	3.6	.....	4.2	....	5.0	2.1	.....	.8	.....	3.8

*Rating table for Tennessee River at Chattanooga, Tennessee, for (a) 1896-1897.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet	Second ft.	Feet	Second ft	Feet.	Second ft.
0.2	3,080	1.4	10,208	3.4	22,088	12.0	73,172
0.3	3,674	1.6	11,396	3.6	23,276	13.0	79,112
0.4	4,268	1.8	12,584	3.8	24,464	14.0	85,052
0.5	4,862	2.0	13,772	4.0	25,652	15.0	90,992
0.6	5,456	2.2	14,960	4.4	28,028	16.0	96,932
0.7	6,050	2.4	16,148	4.8	30,404	18.0	108,812
0.8	6,644	2.6	17,336	6.0	37,532	20.0	120,690
0.9	7,238	2.8	18,524	8.0	49,412	22.0	132,570
1.0	7,832	3.0	19,712	10.0	61,292	24.0	144,450
1.2	9,020	3.2	20,900	11.0	67,232	26.0	156,330

a Above 26 feet use table as published in the Eighteenth Ann. Rept., Part IV, p. 120.

NOTE—This table applied to the foregoing "daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.



The following discharge measurements were made on the Tennessee River at Chattanooga, Tenn., by Max Hall and others during 1898:

May 10—Gage height, 4.14 feet; discharge, 22,066 second-feet.  
 July 29—Gage height, 5.30 feet; discharge, 29,693 second-feet.  
 August 19—Gage height, 6.37 feet; discharge, 36,671 second-feet.  
 Oct. 6—Gage height, 17.60 feet; discharge, 120,359 second-feet.  
 Oct. 28—Gage height, 6.00 feet; discharge, 35,953 second-feet.  
 Nov. 29—Gage height, 4.75 feet; discharge, 29,569 second-feet.  
 Nov. 29—Gage height, 4.70 feet; discharge, 31,340 second-feet.

*Daily gage height, in feet, of Tennessee River at Chattanooga, Tenn., for 1898.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1	3.45	7.55	3.30	17.45	6.40	3.35	2.45	8.15	3.55	3.55	4.60	5.10
2	3.25	6.70	3.15	17.80	5.80	3.30	2.35	7.55	3.95	3.30	4.30	5.05
3	3.05	6.15	3.00	15.00	5.40	3.30	2.25	6.45	9.15	3.20	4.25	4.90
4	2.90	5.40	2.95	11.45	5.05	3.30	2.05	5.35	18.50	3.90	4.05	5.00
5	2.75	5.00	3.30	10.35	4.70	2.85	2.35	6.25	25.00	8.90	3.90	5.05
6	2.65	4.55	3.45	12.15	4.45	2.55	2.10	11.85	22.15	16.90	3.90	5.60
7	2.70	4.45	3.50	11.60	4.35	2.35	2.10	14.65	15.70	16.50	4.25	5.35
8	2.80	4.35	3.40	10.30	4.10	2.20	2.15	12.55	11.25	10.75	4.45	5.90
9	3.05	4.30	3.25	9.30	4.20	2.05	2.60	10.15	9.50	8.80	4.50	5.35
10	3.25	4.15	3.05	8.30	4.15	1.95	3.05	8.50	8.60	8.40	4.45	5.55
11	3.25	4.00	2.90	8.60	4.45	1.80	3.50	9.05	7.45	7.55	4.65	5.10
12	5.50	3.90	2.85	9.50	4.65	1.95	3.40	12.30	6.45	6.55	5.05	4.75
13	13.20	3.80	2.80	9.40	4.40	1.80	3.20	14.85	5.70	6.00	5.30	4.55
14	14.40	3.80	2.85	9.00	4.15	1.75	2.80	15.85	6.20	5.70	4.95	4.30
15	12.25	3.80	3.15	3.95	3.95	1.95	2.85	14.95	4.80	5.35	4.55	4.00
16	12.20	3.70	5.10	0.15	3.90	1.75	3.50	11.60	4.45	4.90	4.40	3.85
17	12.35	3.55	5.05	8.60	3.80	2.00	4.55	8.90	4.25	4.70	4.55	3.70
18	10.00	3.50	5.20	3.20	3.70	2.35	5.35	7.10	3.95	5.25	4.75	3.60
19	9.20	3.30	5.50	8.00	3.70	3.50	4.60	6.40	3.75	6.70	4.90	3.75
20	11.70	3.30	6.10	7.95	3.65	4.05	4.15	6.05	3.55	7.75	5.30	5.20
21	13.80	3.20	5.70	7.40	3.50	5.35	4.00	5.95	3.45	9.30	5.85	5.35
22	13.40	3.25	5.45	7.35	3.50	5.55	3.30	5.65	3.55	8.80	6.05	6.00
23	12.55	3.40	5.15	6.60	3.50	5.05	3.30	5.40	5.00	7.65	6.55	5.35
24	12.35	3.50	4.65	6.85	3.40	4.55	3.40	4.75	5.05	7.25	6.85	5.35
25	12.35	3.60	4.30	6.55	3.45	3.70	3.55	4.30	6.40	7.20	6.55	5.20
26	16.05	3.80	4.15	5.30	3.70	3.40	3.55	4.05	7.20	7.60	6.10	5.40
27	18.20	3.50	4.45	6.45	4.95	2.90	4.55	4.00	6.15	6.90	5.65	5.90
28	16.70	3.35	4.45	7.10	5.60	2.80	5.55	4.10	5.00	6.20	5.15	5.70
29	14.15	.....	4.65	7.05	4.90	2.80	5.35	4.00	4.30	5.65	4.80	5.15
30	11.20	.....	5.55	6.75	4.20	2.55	5.90	4.10	3.85	5.15	4.95	4.90
31	8.95	.....	13.25	.....	3.70	.....	7.90	3.85	.....	4.85	.....	4.50

*Rating table for Tennessee River at Chattanooga, Tenn., for 1898.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second feet.	Feet.	Second feet.	Feet.	Second feet.
0.5	5,900	3.7	19,550	6.9	42,370	10.5	68,650
0.6	6,266	3.8	20,120	7.0	43,100	11.0	72,300
0.7	6,634	3.9	20,700	7.1	43,830	11.5	75,950
0.8	7,004	4.0	21,320	7.2	44,560	12.0	79,600
0.9	7,376	4.1	21,950	7.3	45,290	12.5	83,250
1.0	7,750	4.2	22,580	7.4	46,020	13.0	86,900
1.1	8,126	4.3	23,350	7.5	46,750	13.5	90,550
1.2	8,504	4.4	24,120	7.6	47,480	14.0	94,200
1.3	8,884	4.5	24,850	7.7	48,210	14.5	97,850
1.4	9,266	4.6	25,580	7.8	48,940	15.0	101,500
1.5	9,650	4.7	26,310	7.9	49,670	15.5	105,150
1.6	10,046	4.8	27,040	8.0	50,400	16.0	108,800
1.7	10,444	4.9	27,770	8.1	51,130	16.5	112,450
1.8	10,844	5.0	28,500	8.2	51,860	17.0	116,100
1.9	11,246	5.1	29,230	8.3	52,590	17.5	119,750
2.0	11,650	5.2	29,960	8.4	53,320	18.0	123,400
2.1	12,056	5.3	30,690	8.5	54,050	18.5	127,050
2.2	12,464	5.4	31,420	8.6	54,780	19.0	130,700
2.3	12,874	5.5	32,150	8.7	55,510	19.5	134,350
2.4	13,286	5.6	32,880	8.8	56,240	20.0	138,000
2.5	13,700	5.7	33,610	8.9	56,970	20.5	141,650
2.6	14,126	5.8	34,340	9.0	57,700	21.0	145,300
2.7	14,562	5.9	35,070	9.1	58,430	21.5	148,950
2.8	15,008	6.0	35,800	9.2	59,160	22.0	152,600
2.9	15,464	6.1	36,530	9.3	59,890	22.5	156,250
3.0	15,930	6.2	37,260	9.4	60,620	23.0	159,900
3.1	16,410	6.3	37,990	9.5	61,350	23.5	163,550
3.2	16,900	6.4	38,720	9.6	62,080	24.0	167,200
3.3	17,400	6.5	39,450	9.7	62,810	24.6	171,680
3.4	17,920	6.6	40,180	9.8	63,540		
3.5	18,460	6.7	40,910	9.9	64,270		
3.6	19,000	6.8	41,640	10.0	65,000		

NOTE—This table applied to the foregoing "daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following measurements were made by Max Hall and others during 1899:

May 3—Gage height, 6.71 feet; discharge, 37,770 second-feet.  
 May 26—Gage height, 4.76 feet; discharge, 25,526 second-feet.  
 June 21—Gage height, 4.15 feet; discharge, 21,391 second-feet.  
 Sept. 15—Gage height, 1.90 feet; discharge, 10,819 second-feet.  
 Oct. 27—Gage height, 0.80 foot; discharge, 6,566 second-feet.

*Daily gage height in feet of Tennessee River at Chattanooga, Tenn., for 1899.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec.
1.....	4.75	5.70	19.25	22.80	7.60	4.15	3.45	4.20	2.20	1.20	1.10	1.70
2.....	4.95	5.65	17.60	19.50	7.10	4.25	3.30	3.55	2.35	1.10	1.10	1.70
3.....	5.30	5.60	15.15	14.90	6.70	4.40	3.05	3.05	2.80	1.05	1.05	1.70
4.....	5.80	10.70	14.15	12.95	6.34	4.85	2.80	2.75	3.05	.95	1.10	1.70
5.....	5.95	23.10	17.95	13.25	6.15	4.65	2.60	2.45	2.65	.90	1.50	1.80
6.....	7.25	30.45	24.50	14.70	7.10	4.25	2.60	2.45	2.25	.80	1.50	1.70
7.....	18.80	34.30	26.55	15.70	8.54	4.15	2.65	2.40	1.95	.85	1.50	1.60
8.....	18.40	36.95	27.60	18.05	9.35	3.75	3.05	2.40	1.80	1.00	1.45	1.50
9.....	17.35	38.25	27.70	17.75	10.04	3.51	2.90	2.25	1.60	1.15	1.35	1.40
10.....	17.15	36.75	16.15	15.70	10.70	3.40	2.60	2.10	1.80	1.60	1.20	1.40
11.....	13.85	30.30	11.85	14.20	11.15	3.90	2.65	2.10	1.70	1.80	1.15	1.60
12.....	10.50	19.35	10.64	12.90	10.40	4.30	2.45	2.00	2.00	1.85	1.10	5.20
13.....	9.15	12.15	9.55	11.65	9.60	5.25	2.30	2.00	1.80	1.70	1.00	6.45
14.....	8.10	9.50	11.20	10.70	9.30	5.80	2.20	2.25	2.00	1.65	1.00	7.40
15.....	7.55	8.50	24.55	10.00	9.55	6.45	2.15	2.65	1.85	1.40	1.00	7.15
16.....	7.30	7.55	34.25	9.40	9.20	6.10	1.95	2.65	1.65	1.25	1.00	6.20
17.....	7.40	7.95	36.90	8.75	8.70	6.40	1.90	2.40	1.45	1.15	1.00	5.20
18.....	7.45	9.55	36.15	8.40	7.75	6.20	1.80	2.30	1.35	1.15	1.00	4.25
19.....	7.25	11.30	35.85	8.00	6.90	5.25	1.90	2.15	1.20	1.10	1.00	3.85
20.....	7.00	12.65	37.05	7.55	6.40	4.70	2.05	1.90	1.05	1.10	.95	4.25
21.....	6.80	11.50	39.20	7.35	5.90	4.20	2.05	1.70	1.00	1.10	.85	4.40
22.....	6.45	10.65	40.00	7.05	5.60	3.75	2.40	1.60	1.05	1.10	.85	4.40
23.....	5.90	10.10	38.70	7.85	5.35	3.50	2.70	1.45	1.30	1.05	1.00	4.15
24.....	5.65	9.75	32.70	9.65	5.30	3.25	3.50	1.30	1.50	1.00	1.15	5.65
25.....	6.05	9.50	23.15	9.35	5.05	3.15	3.40	1.20	1.50	.95	1.10	6.15
26.....	6.35	9.20	16.30	10.75	4.80	3.00	3.00	1.20	1.45	.85	1.70	6.30
27.....	5.85	13.20	13.65	10.30	4.65	3.25	3.05	1.20	1.30	.80	1.80	5.85
28.....	5.75	18.45	13.95	9.20	4.40	3.65	3.65	1.25	1.20	.80	1.85	5.65
29.....	5.55	.....	17.30	8.35	4.30	3.50	4.25	1.50	1.25	.90	1.80	5.10
30.....	5.30	.....	21.20	7.75	4.20	3.30	4.25	1.85	1.30	1.00	1.75	4.65
31.....	5.80	.....	22.80	.....	4.25	.....	5.15	1.75	.....	1.05	.....	3.85

During 1900 the following measurements were made by Max Hall and others:

March 13—Gage height, 11.25 feet; discharge, 66,012 second-feet.

July 27—Gage height, 3.45 feet; discharge, 18,470 second-feet.

*Daily gage height in feet of Tennessee River at Chattanooga, Tenn., for 1900.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	3.06	3.35	8.06	7.85	6.20	2.85	8.85	6.20	2.10	2.00	2.90	8.70
2.....	*	2.85	8.70	7.20	5.65	2.80	8.15	5.40	2.00	1.90	2.60	6.50
3.....	*	2.60	10.90	6.85	5.35	3.00	6.95	4.70	2.30	1.70	2.50	6.60
4.....	*	2.50	12.50	7.25	5.15	3.20	6.30	4.20	2.50	1.50	2.90	5.10
5.....	*	2.90	12.75	8.05	4.95	3.20	5.80	3.60	2.30	1.40	3.30	5.60
6.....	2.10	3.80	10.65	8.55	4.80	3.50	5.40	3.20	2.00	1.30	3.70	6.90
7.....	2.20	3.95	10.00	7.85	4.65	5.65	5.00	2.90	1.70	1.30	4.20	8.30
8.....	2.30	3.90	11.65	7.05	4.45	6.65	4.50	2.60	1.60	1.60	4.20	9.20
9.....	2.35	5.35	14.55	6.50	4.45	6.15	4.2	2.40	1.40	1.80	3.70	8.50
10.....	2.45	8.40	16.50	6.10	4.35	5.80	4.20	2.30	1.30	2.10	3.20	7.00
11.....	3.35	9.40	18.15	6.50	4.30	5.00	4.30	2.10	1.20	2.10	3.00	6.10
12.....	6.05	8.95	14.25	7.50	4.30	4.90	3.80	2.00	1.10	2.50	2.70	5.40
13.....	8.15	13.90	11.65	7.40	4.15	4.50	3.40	1.90	1.00	3.00	2.60	4.90
14.....	8.70	21.55	9.85	7.00	4.00	5.20	3.30	1.90	1.10	2.50	2.30	4.50
15.....	8.45	24.00	8.65	6.50	3.85	5.30	3.30	2.10	1.80	1.90	2.20	4.30
16.....	7.90	21.40	8.00	6.30	3.75	5.25	3.30	2.20	3.10	1.80	2.10	4.20
17.....	6.35	17.00	7.90	8.75	3.60	5.45	3.20	2.30	4.00	1.60	2.60	4.00
18.....	5.50	12.05	7.55	10.65	3.50	6.15	3.10	2.30	4.10	1.50	2.00	3.60
19.....	5.90	9.25	7.55	9.75	3.40	8.85	3.00	2.30	4.60	1.40	1.90	3.40
20.....	8.50	7.70	8.55	9.40	3.40	9.20	2.90	2.20	4.70	1.30	1.90	3.30
21.....	9.40	7.10	11.60	11.70	3.25	8.90	2.70	1.90	3.90	1.20	2.10	3.20
22.....	8.85	7.70	14.95	12.00	3.15	7.65	2.50	1.80	3.00	1.20	2.20	4.00
23.....	7.95	8.50	17.40	11.35	3.05	6.40	2.50	1.70	2.60	1.40	2.30	4.20
24.....	7.20	8.55	16.45	10.70	3.00	6.25	2.80	1.80	2.40	2.20	2.80	4.70
25.....	6.15	8.50	12.65	9.75	3.15	7.15	3.00	1.90	2.70	4.10	3.20	5.20
26.....	5.50	9.50	11.15	8.50	3.20	7.60	3.10	2.50	2.70	7.00	7.80	5.40
27.....	5.00	9.30	10.90	7.80	3.35	8.05	3.30	3.10	2.60	7.50	13.90	5.20
28.....	4.65	8.45	10.70	7.45	3.60	8.20	4.60	2.70	2.40	6.00	15.60	4.60
29.....	4.20	.....	10.20	7.05	3.60	8.60	8.00	2.50	2.30	4.90	15.60	4.80
30.....	3.90	.....	9.35	6.60	3.35	8.70	8.20	2.30	2.20	3.70	13.20	4.20
31.....	3.55	.....	8.50	.....	3.05	.....	7.30	2.20	.....	3.40	.....	4.50

\*Frozen at gage.

WATER-POWERS OF ALABAMA.

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*Rating table for Tennessee River at Chattanooga, Tennessee, for 1899-1900.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
0.8	6,600	10.7	62,340	20.6	123,720	30.5	185,100
0.9	6,950	10.8	62,960	20.7	124,340	30.6	185,720
1.0	7,300	10.9	63,580	20.8	124,960	30.7	186,340
1.1	7,670	11.0	64,200	20.9	125,580	30.8	186,960
1.2	8,040	11.1	64,820	21.0	126,200	30.9	187,580
1.3	8,430	11.2	65,440	21.1	126,820	31.0	188,200
1.4	8,820	11.3	66,060	21.2	127,440	31.1	188,820
1.5	9,220	11.4	66,680	21.3	128,060	31.2	189,440
1.6	9,620	11.5	67,300	21.4	128,680	31.3	190,060
1.7	10,020	11.6	67,920	21.5	129,300	31.4	190,680
1.8	10,430	11.7	68,540	21.6	129,920	31.5	191,300
1.9	10,840	11.8	69,160	21.7	130,540	31.6	191,920
2.0	11,250	11.9	69,780	21.8	131,160	31.7	192,540
2.1	11,660	12.0	70,400	21.9	131,780	31.8	193,160
2.2	12,080	12.1	71,020	22.0	132,400	31.9	193,780
2.3	12,500	12.2	71,640	22.1	133,020	32.0	194,400
2.4	12,930	12.3	72,260	22.2	133,640	32.1	195,020
2.5	13,360	12.4	72,880	22.3	134,260	32.2	195,640
2.6	13,800	12.5	73,500	22.4	134,880	32.3	196,260
2.7	14,240	12.6	74,120	22.5	135,500	32.4	196,880
2.8	14,680	12.7	74,740	22.6	136,120	32.5	197,500
2.9	15,140	12.8	75,360	22.7	136,740	32.6	198,120
3.0	15,600	12.9	75,980	22.8	137,360	32.7	198,740
3.1	16,080	13.0	76,600	22.9	137,980	32.8	199,360
3.2	16,550	13.1	77,220	23.0	138,600	32.9	199,980
3.3	17,050	13.2	77,840	23.1	139,220	33.0	200,600
3.4	17,550	13.3	78,460	23.2	139,840	33.1	201,220
3.5	18,050	13.4	79,080	23.3	140,460	33.2	201,840
3.6	18,550	13.5	79,700	23.4	141,080	33.3	202,460
3.7	19,050	13.6	80,320	23.5	141,700	33.4	203,080
3.8	19,600	13.7	80,940	23.6	142,320	33.5	203,700
3.9	20,200	13.8	81,560	23.7	142,940	33.6	204,320
4.0	20,800	13.9	82,180	23.8	143,560	33.7	204,940
4.1	21,420	14.0	82,800	23.9	144,180	33.8	205,560
4.2	22,040	14.1	83,420	24.0	144,800	33.9	206,180
4.3	22,660	14.2	84,040	24.1	145,420	34.0	206,800
4.4	23,280	14.3	84,660	24.2	146,040	34.1	207,420
4.5	23,900	14.4	85,280	24.3	146,660	34.2	208,040
4.6	24,520	14.5	85,900	24.4	147,280	34.3	208,660
4.7	25,140	14.6	86,520	24.5	147,900	34.4	209,280
4.8	25,780	14.7	87,140	24.6	148,520	34.5	209,900
4.9	26,380	14.8	87,760	24.7	149,140	34.6	210,520
5.0	27,000	14.9	88,380	24.8	149,760	34.7	211,140
5.1	27,620	15.0	89,000	24.9	150,380	34.8	211,760
5.2	28,240	15.1	89,620	25.0	151,000	34.9	212,380
5.3	28,860	15.2	90,240	25.1	151,620	35.0	213,000
5.4	29,480	15.3	90,860	25.2	152,240	35.1	213,620
5.5	30,100	15.4	91,480	25.3	152,860	35.2	214,240
5.6	30,720	15.5	92,100	25.4	153,480	35.3	214,860

*Rating table for Tennessee River at Chattanooga, Tennessee, for 1899-1900.*

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
5.7	31,340	15.6	92,720	25.5	154,100	35.4	215,480
5.8	31,960	15.7	93,340	25.6	154,720	35.5	216,100
5.9	32,580	15.8	93,960	25.7	155,340	35.6	216,720
6.0	33,200	15.9	94,580	25.8	155,960	35.7	207,340
6.1	33,820	16.0	95,200	25.9	156,580	35.8	217,960
6.2	34,440	16.1	95,820	26.0	157,200	35.9	218,580
6.3	35,060	16.2	96,440	26.1	157,820	36.0	219,200
6.4	35,680	16.3	97,060	26.2	158,440	36.1	219,820
6.5	36,300	16.4	97,680	26.3	159,060	36.2	220,440
6.6	36,920	16.5	98,300	26.4	159,680	36.3	221,060
6.7	37,540	16.6	98,920	26.5	160,300	36.4	221,680
6.8	38,160	16.7	99,540	26.6	160,920	36.5	222,300
6.9	38,780	16.8	100,160	26.7	161,540	36.6	222,920
7.0	39,400	16.9	100,780	26.8	162,160	36.7	223,540
7.1	40,020	17.0	101,400	26.9	162,780	36.8	224,160
7.2	40,640	17.1	102,020	27.0	163,400	36.9	224,780
7.3	41,260	17.2	102,640	27.1	164,020	37.0	225,400
7.4	41,880	17.3	103,260	27.2	164,640	37.1	226,020
7.5	42,500	17.4	103,880	27.3	165,260	37.2	226,640
7.6	43,120	17.5	104,500	27.4	165,880	37.3	227,260
7.7	43,740	17.6	105,120	27.5	166,500	37.4	227,880
7.8	44,360	17.7	105,740	27.6	167,120	37.5	228,500
7.9	44,980	17.8	106,360	27.7	167,740	37.6	229,120
8.0	45,600	17.9	106,980	27.8	168,360	37.7	229,740
8.1	46,220	18.0	107,600	27.9	168,980	37.8	230,360
8.2	46,840	18.1	108,220	28.0	169,600	37.9	230,980
8.3	47,460	18.2	108,840	28.1	170,220	18.0	231,600
8.4	48,080	18.3	109,460	28.2	170,840	38.1	232,220
8.5	48,700	18.4	110,080	28.3	171,460	38.2	232,840
8.6	49,320	18.5	110,700	28.4	172,080	38.3	233,460
8.7	49,940	18.6	111,320	28.5	172,700	38.4	234,080
8.8	50,560	18.7	111,940	28.6	173,320	38.5	234,700
8.9	51,180	18.8	112,560	28.7	173,940	38.6	235,320
9.0	51,800	18.9	113,180	28.8	174,560	38.7	235,940
9.1	52,420	19.0	113,800	28.9	175,180	38.8	236,560
9.2	53,040	19.1	114,420	29.0	175,800	38.9	237,180
9.3	53,660	19.2	115,040	29.1	176,420	39.0	237,800
9.4	54,280	19.3	115,660	29.2	177,040	39.1	238,420
9.5	54,900	19.4	116,280	29.3	177,660	39.2	239,040
9.6	55,520	19.5	116,900	29.4	178,280	39.3	239,660
9.7	56,140	19.6	117,520	29.5	178,900	39.4	240,280
9.8	56,760	19.7	118,140	29.6	179,520	39.5	240,900
9.9	57,380	19.8	118,760	29.7	180,140	39.6	241,520
10.0	58,000	19.9	119,380	29.8	180,760	39.7	242,140
10.1	58,620	20.0	120,000	29.9	181,380	39.8	242,760
10.2	59,240	20.1	120,620	30.0	182,000	39.9	243,380
10.3	59,860	20.2	121,240	30.1	182,620	40.0	244,000

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
10.4	60,480	20.3	121,860	30.2	183,240		
10.5	61,100	20.4	122,480	30.3	183,860		
10.6	61,720	20.5	123,100	30.4	184,480		

NOTE—This table applied to the foregoing "daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

The following discharge measurements were made on the Tennessee River at Chattanooga, Tenn., during 1901:

1901.

Jan. 24—Hydrographer, Max Hall: Gage height, 5.60 feet; discharge, 30,317 second-feet.

April 4—Hydrographer, K. T. Thomas: Gage height, 24.20 feet; discharge, 155,457 second-feet.

July 31—Hydrographer, K. T. Thomas: Gage height, 2.80 feet; discharge, 15,393 second-feet.

Aug. 18—Hydrographer, K. T. Thomas: Gage height, 31.70 feet; discharge, 198,718 second-feet.

*Daily gage height of Tennessee River at Chattanooga, Tenn., for 1901.*

Day	Jan.	Feb.	Mar.	April	May	Jun.	July	Aug.	Sept.	Oct.	Nov.	Dec
1.....	5.2	6.5	3.7	12.4	10.8	12.0	6.0	2.8	9.9	4.2	2.6	2.3
2.....	5.7	6.7	3.7	13.2	9.3	11.1	5.9	2.8	9.8	4.5	2.6	2.2
3.....	5.8	7.2	3.7	19.7	8.5	9.8	6.3	2.9	9.7	4.6	2.5	2.3
4.....	5.6	8.7	3.7	24.1	7.6	8.5	6.4	2.8	10.3	4.5	2.5	2.5
5.....	5.1	10.1	3.8	23.9	7.0	7.7	6.0	2.6	9.4	4.4	2.5	2.5
6.....	4.7	10.0	4.0	22.4	6.7	6.9	5.2	2.6	7.9	4.8	2.5	2.5
7.....	4.4	9.4	4.7	18.9	6.4	6.9	5.1	3.2	6.9	4.6	2.4	3.0
8.....	4.1	8.9	4.1	14.2	6.2	6.9	5.4	9.1	6.4	4.1	2.4	3.2
9.....	3.9	8.5	4.0	11.8	5.9	6.5	5.6	12.2	5.9	3.9	2.4	3.2
10.....	3.8	7.7	7.0	10.3	5.6	6.9	6.3	9.9	5.5	3.7	2.4	3.5
11.....	6.1	7.6	9.8	9.2	5.6	8.2	6.6	7.3	5.3	3.4	2.4	3.5
12.....	15.4	7.0	11.2	8.4	5.4	7.4	5.6	5.8	5.1	3.4	2.4	4.0
13.....	26.6	7.1	9.7	7.9	5.6	6.4	5.0	5.3	5.7	3.5	2.5	4.1
14.....	28.1	7.2	8.2	6.8	5.5	6.1	4.4	6.5	5.9	4.0	2.5	4.7
15.....	25.3	7.0	7.3	9.8	5.5	6.4	4.1	14.0	6.0	4.3	2.5	17.9
16.....	19.5	6.4	6.4	10.3	5.4	7.5	3.6	27.3	6.1	4.1	2.5	26.8
17.....	12.7	5.8	5.8	10.2	5.2	8.9	3.7	32.8	6.3	4.1	2.4	28.8
18.....	9.7	5.3	5.4	9.6	4.9	9.8	3.9	32.6	8.8	4.0	2.4	26.7
19.....	8.1	5.1	5.0	10.8	5.3	9.3	3.7	28.6	9.9	3.7	2.3	19.9
20.....	7.2	5.0	4.7	21.1	8.0	8.9	3.7	23.4	9.3	3.3	2.3	11.4
21.....	6.4	4.9	4.7	26.5	10.2	8.4	4.2	18.6	8.3	3.1	2.2	8.3
22.....	5.9	4.7	4.8	24.7	20.2	7.7	3.9	17.0	7.3	3.1	2.1	6.6
23.....	5.4	4.5	5.2	23.0	26.5	10.1	3.7	16.5	8.4	3.1	2.1	5.7
24.....	5.6	4.4	5.0	22.2	29.7	9.5	3.5	18.5	5.6	3.0	2.5	5.8
25.....	5.8	4.2	5.0	19.0	32.4	7.6	3.1	16.5	5.2	3.0	2.5	6.9
26.....	5.8	4.1	7.7	17.1	32.5	9.6	3.0	13.1	4.9	2.9	2.5	7.9
27.....	5.4	3.8	15.9	14.9	23.5	9.8	2.9	11.0	4.6	2.8	2.6	10.2
28.....	5.2	3.7	22.3	14.9	13.5	8.4	2.9	10.3	4.4	2.7	2.5	16.0
29.....	5.2	.....	21.7	14.5	12.1	7.2	2.8	10.7	4.4	2.6	2.5	21.0
30.....	5.2	.....	18.4	13.8	11.9	6.4	2.8	10.0	4.3	2.5	2.4	32.0
31.....	5.5	.....	14.7	.....	12.3	.....	2.8	9.8	.....	2.5	.....	37.4

Rating table of Tennessee River at Chattanooga, Tennessee, for 1901.

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.	Feet.	Second ft.
2.0	11,250	11.6	69,600	21.2	132,000	30.8	194,400
2.1	11,660	11.7	70,250	21.3	132,650	30.9	195,050
2.2	12,080	11.8	70,900	21.4	133,300	31.0	195,700
2.3	12,500	11.9	71,550	21.5	133,950	31.1	196,350
2.4	12,930	12.0	72,200	21.6	134,600	31.2	197,000
2.5	13,360	12.1	72,850	21.7	135,250	31.3	197,650
2.6	13,800	12.2	73,500	21.8	135,900	31.4	198,300
2.7	14,240	12.3	74,150	21.9	136,550	31.5	198,950
2.8	14,680	12.4	74,800	22.0	137,200	31.6	199,600
2.9	15,140	12.5	75,450	22.1	137,850	31.7	200,250
3.0	15,600	12.6	76,100	22.2	138,500	31.8	200,900
3.1	16,080	12.7	76,750	22.3	139,150	31.9	201,550
3.2	16,550	12.8	77,400	22.4	139,800	32.0	202,200
3.3	17,050	12.9	78,050	22.5	140,450	32.1	202,850
3.4	17,550	13.0	78,700	22.6	141,100	32.2	203,500
3.5	18,050	13.1	79,350	22.7	141,750	32.3	204,150
3.6	18,550	13.2	80,000	22.8	142,400	32.4	204,800
3.7	19,050	13.3	80,650	22.9	143,050	32.5	205,450
3.8	19,600	13.4	81,300	23.0	143,700	32.6	206,100
3.9	20,200	13.5	81,950	23.1	144,350	32.7	206,750
4.0	20,800	13.6	82,600	23.2	145,000	32.8	207,400
4.1	21,420	13.7	83,250	23.3	145,650	32.9	208,050
4.2	22,040	13.8	83,900	23.4	146,300	33.0	208,700
4.3	22,660	13.9	84,550	23.5	146,950	33.1	209,350
4.4	23,280	14.0	85,200	23.6	147,600	33.2	210,000
4.5	23,900	14.1	85,850	23.7	148,250	33.3	210,650
4.6	24,520	14.2	86,500	23.8	148,900	33.4	211,300
4.7	25,140	14.3	87,150	23.9	149,550	33.5	211,950
4.8	25,760	14.4	87,800	24.0	150,200	33.6	212,600
4.9	26,380	14.5	88,450	24.1	150,850	33.7	213,250
5.0	27,000	14.6	89,100	24.2	151,500	33.8	213,900
5.1	27,620	14.7	89,750	24.3	152,150	33.9	214,550
5.2	28,240	14.8	90,400	24.4	152,800	34.0	215,200
5.3	28,860	14.9	91,050	24.5	153,450	34.1	215,850
5.4	29,480	15.0	91,700	24.6	154,100	34.2	216,500
5.5	30,100	15.1	92,350	24.7	154,750	34.3	217,150
5.6	30,720	15.2	93,000	24.8	155,400	34.4	217,800
5.7	31,340	15.3	93,650	24.9	156,050	34.5	218,450
5.8	31,960	15.4	94,300	25.0	156,700	34.6	219,100
5.9	32,580	15.5	94,950	25.1	157,350	34.7	219,750
6.0	33,200	15.6	95,600	25.2	158,000	34.8	220,400
6.1	33,850	15.7	96,250	25.3	158,650	34.9	221,050
6.2	34,500	15.8	96,900	25.4	159,300	35.0	221,700
6.3	35,150	15.9	97,550	25.5	159,950	35.1	222,350
6.4	35,800	16.0	98,200	25.6	160,600	35.2	223,000
6.5	36,450	16.1	98,850	25.7	161,250	35.3	223,650
6.6	37,100	16.2	99,500	25.8	161,900	35.4	224,300
6.7	37,750	16.3	100,150	25.9	162,550	35.5	224,950



Rating table for Tennessee River at Chattanooga, Tenn., for 1901.

Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.	Gage height.	Discharge.
Feet.	Second Ft.	Feet.	Second Ft.	Feet.	Second ft.	Feet.	Second Ft.
6.8	38,400	16.4	100,800	26.0	163,200	35.6	225,600
6.9	39,050	16.5	101,450	26.1	163,850	35.7	226,250
7.0	39,700	16.6	102,100	26.2	164,500	35.8	226,900
7.1	40,350	16.7	102,750	26.3	165,150	35.9	227,550
7.2	41,000	16.8	103,400	26.4	165,800	36.0	228,200
7.3	41,650	16.9	104,050	26.5	166,450	36.1	228,850
7.4	42,300	17.0	104,700	26.6	167,100	36.2	229,500
7.5	42,950	17.1	105,350	26.7	167,750	36.3	230,150
7.6	43,600	17.2	106,000	26.8	168,400	36.4	230,800
7.7	44,250	17.3	106,650	26.9	169,050	36.5	231,450
7.8	44,900	17.4	107,300	27.0	169,700	36.6	232,100
7.9	45,550	17.5	107,950	27.1	170,350	36.7	232,750
8.0	46,200	17.6	108,600	27.2	171,000	36.8	233,400
8.1	46,850	17.7	109,250	27.3	171,650	36.9	234,050
8.2	47,500	17.8	109,900	27.4	172,300	37.0	234,700
8.3	48,150	17.9	110,550	27.5	172,950	37.1	235,350
8.4	48,800	18.0	111,200	27.6	173,600	37.2	236,000
8.5	49,450	18.1	111,850	27.7	174,250	37.3	236,650
8.6	50,100	18.2	112,500	27.8	174,900	37.4	237,300
8.7	50,750	18.3	113,150	27.9	175,550	37.5	237,950
8.8	51,400	18.4	113,800	28.0	176,200	37.6	238,600
8.9	52,050	18.5	114,450	28.1	176,850	37.7	239,250
9.0	52,700	18.6	115,100	28.2	177,500	37.8	239,900
9.1	53,350	18.7	115,750	28.3	178,150	37.9	240,550
9.2	54,000	18.8	116,400	28.4	178,800	38.0	241,200
9.3	54,650	18.9	117,050	28.5	179,450	38.1	241,850
9.4	55,300	19.0	117,700	28.6	180,100	38.2	242,500
9.5	55,950	19.1	118,350	28.7	180,750	38.3	243,150
9.6	56,600	19.2	119,000	28.8	181,400	38.4	243,800
9.7	57,250	19.3	119,650	28.9	182,050	38.5	244,450
9.8	57,900	19.4	120,300	29.0	182,700	38.6	245,100
9.9	58,550	19.5	120,950	29.1	183,350	38.7	245,750
10.0	59,200	19.6	121,600	29.2	184,000	38.8	246,400
10.1	59,850	19.7	122,250	29.3	184,650	38.9	247,050
10.2	60,500	19.8	122,900	29.4	185,300	39.0	247,700
10.3	61,150	19.9	123,550	29.5	185,950	39.1	248,350
10.4	61,800	20.0	124,200	29.6	186,600	39.2	249,000
10.5	62,450	20.1	124,850	29.7	187,250	39.3	249,650
10.6	63,100	20.2	125,500	29.8	187,900	39.4	250,300
10.7	63,750	20.3	126,150	29.9	188,550	39.5	250,950
10.8	64,400	20.4	126,800	30.0	189,200	39.6	251,600
10.9	65,050	20.5	127,450	30.1	189,850	39.7	252,250
11.0	65,700	20.6	128,100	30.2	190,500	39.8	252,900
11.1	66,350	20.7	128,750	30.3	191,150	39.9	253,550
11.2	67,000	20.8	129,400	30.4	191,800	40.0	254,200
11.3	67,650	20.9	130,050	30.5	192,450		
11.4	68,300	21.0	130,700	30.6	193,100		
11.5	68,950	21.1	131,350	30.7	193,750		

Note—This table applied to the foregoing "daily gage heights" gives the cubic feet per second flowing in the river on each date for which the gage height is given.

*Estimated monthly discharge of Tennessee River at Chattanooga, Tennessee.*

[Drainage area, 21,382 square miles.]

Month.	Discharge in second-feet				Run-off.	
	Maxi- mum.	Mini- mum.	Mean.	Total in acre- feet.	Depth in inches.	Second- feet per square mile.
1890.						
January .....	78,470	30,150	42,749	2,628,551	2.31	2.00
February .....	308,060	44,191	76,081	4,225,310	3.72	3.56
March .....	445,120	52,906	129,093	7,937,670	6.96	6.03
April .....	121,464	43,029	64,855	3,859,132	3.38	3.03
May .....	72,079	41,286	51,200	3,148,186	2.76	2.39
June .....	41,286	23,400	29,102	1,731,685	1.52	1.36
July .....	47,677	19,500	27,036	1,662,390	1.45	1.26
August .....	46,515	21,100	30,881	1,898,810	1.66	1.44
September .....	47,096	22,200	27,843	1,656,770	1.45	1.30
October .....	58,135	25,950	37,982	2,335,437	2.04	1.77
November .....	42,448	20,700	26,394	1,570,549	1.37	1.23
December .....	77,889	20,400	36,088	2,218,980	1.95	1.69
Per annum .....	445,120	19,500	48,275	34,873,470	30.57	2.26
1891.						
January .....	92,995	39,543	59,484	3,657,552	3.21	2.78
February .....	356,120	59,878	154,822	8,598,380	7.53	7.23
March .....	381,040	63,364	135,160	8,310,718	7.30	6.32
April .....	97,643	38,962	61,873	3,681,690	3.22	2.89
May .....	37,219	26,380	30,215	1,857,860	1.63	1.41
June .....	47,096	26,840	36,276	2,158,567	1.90	1.70
July .....	36,057	21,800	26,429	1,625,066	1.43	1.24
August .....	98,224	23,000	40,402	2,484,238	2.18	1.89
September .....	36,638	19,200	25,777	1,533,835	1.34	1.20
October .....	19,200	17,910	18,461	1,135,130	.99	.86
November .....	41,867	17,440	23,510	1,398,939	1.23	1.10
December .....	66,269	22,200	39,299	2,416,417	2.12	1.84
Per annum .....	381,040	17,440	54,309	38,858,392	34.08	2.54
1892.						
January .....	363,240	41,286	103,453	6,361,118	5.57	4.83
February .....	69,755	33,733	46,755	2,689,348	2.43	2.25
March .....	64,526	31,680	45,769	2,814,244	2.47	2.14
April .....	299,160	40,705	101,287	6,026,982	5.27	4.73
May .....	53,487	31,160	39,772	2,445,501	2.14	1.86
June .....	56,972	28,680	43,265	2,574,447	2.25	2.02
July .....	71,498	26,380	44,520	2,737,446	2.40	2.08
August .....	31,680	21,800	25,121	1,544,640	1.35	1.17
September .....	29,660	17,660	21,403	1,273,564	1.11	1.00
October .....	19,800	17,220	17,952	1,103,833	.97	.84
November .....	43,610	17,220	27,924	1,661,590	1.45	1.30
December .....	56,972	21,450	32,793	2,016,376	1.76	1.53
Per annum .....	363,240	17,220	45,835	33,249,089	29.17	2.15

# WATER-POWERS OF ALABAMA.

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*Estimated monthly discharge of Tennessee River at Chattanooga, Tennessee—Continued.*

[Drainage area, 21,382 square miles.]

Month.	Discharge in second-feet.			Total in acre-ft.	Run-off.	
	Maxi-mum.	Mini-mum.	Mean.		Depth in inches.	Second-ft. per sq. mile.
1893.						
January .....	44,191	22,600	26,812	1,648,616	1.44	1.25
February .....	283,140	38,381	105,921	5,882,535	5.15	4.95
March .....	72,660	36,057	50,320	3,094,076	2.71	2.35
April .....	73,241	31,160	42,137	2,507,326	2.20	1.97
May .....	224,400	30,150	71,525	4,397,929	3.85	3.34
June .....	123,207	27,760	49,679	2,956,099	2.59	2.32
July .....	34,895	21,450	25,741	1,582,763	1.38	1.20
August .....	33,152	18,410	23,477	1,443,554	1.27	1.10
September .....	76,727	20,750	33,933	2,019,149	1.77	1.59
October .....	58,716	18,660	25,550	1,571,018	1.37	1.19
November .....	31,160	20,400	22,263	1,324,738	1.16	1.04
December .....	30,640	21,100	24,970	1,535,355	1.35	1.17
Per annum .....	283,140	18,410	41,861	29,963,158	26.24	1.96
1894.						
January .....	56,972	22,600	37,389	2,298,975	2.02	1.75
February .....	151,095	31,680	70,893	3,937,185	3.45	3.31
March .....	59,297	33,152	46,796	2,877,392	2.53	2.19
April .....	52,325	27,300	36,287	2,159,222	1.90	1.70
May .....	44,191	23,800	31,137	1,914,552	1.68	1.43
June .....	28,680	19,500	21,983	1,308,076	1.15	1.03
July .....	29,170	18,910	24,486	1,505,595	1.31	1.14
August .....	30,150	18,910	22,971	1,412,441	1.23	1.07
September .....	27,300	16,560	19,160	1,140,097	1.00	.90
October .....	20,750	16,360	17,445	1,072,658	.94	.82
November .....	20,400	16,360	17,330	1,031,204	.90	.81
December .....	68,012	16,780	30,862	1,897,643	1.66	1.44
Per annum .....	151,095	16,360	31,395	22,555,040	19.77	1.53
1895.						
January .....	261,780	23,400	76,446	4,700,512	4.12	3.57
February .....	47,096	24,200	35,787	1,987,503	1.74	1.67
March .....	134,827	42,448	72,341	4,448,103	3.90	3.38
April .....	78,470	37,219	51,047	3,037,501	2.67	2.39
May .....	58,135	34,314	43,929	2,701,106	2.37	2.05
June .....	35,476	21,100	26,417	1,651,917	1.37	1.23
July .....	63,364	20,750	29,638	1,822,381	1.60	1.39
August .....	38,381	21,800	26,927	1,655,687	1.45	1.26
September .....	24,660	16,780	20,316	1,208,883	1.05	.95
October .....	17,000	16,360	16,665	1,024,698	.90	.78
November .....	20,750	17,220	18,162	1,080,714	.94	.85
December .....	33,152	17,660	21,561	1,325,743	1.16	1.01
Per annum .....	261,780	16,360	36,603	26,564,748	23.27	1.71

*Estimated monthly discharge of Tennessee River at Chattanooga,  
Tennessee—Continued.*

[Drainage area, 21,418 square miles.]

Month.	Discharge in second-feet.			Total in acre-feet.	Run-off.	
	Maxi-mum.	Mini-mum.	Mean		Depth in inches.	Second-ft. per sq. mile.
1896.						
January .....	50,600	15,554	26,169	1,609,079	1.41	1.22
February .....	85,052	26,840	55,577	3,196,826	2.79	2.59
March .....	95,150	22,088	39,257	2,413,834	2.11	1.88
April .....	409,520	23,276	87,649	5,215,478	4.56	4.09
May .....	29,216	14,366	20,574	1,265,054	1.10	0.96
June .....	43,472	17,336	24,365	1,449,815	1.27	1.14
July .....	130,196	20,306	55,390	3,405,820	2.99	2.59
August .....	34,562	14,960	22,433	1,379,360	1.21	1.05
September .....	18,524	9,020	12,346	734,636	.64	.58
October .....	19,712	8,426	11,588	712,523	.62	.54
November .....	57,728	9,020	22,603	1,344,969	1.18	1.06
December .....	45,254	16,148	27,951	1,718,651	1.51	1.31
The year .....	409,520	8,426	33,825	24,446,045	21.39	1.58
1897.						
January .....	45,254	16,148	27,932	1,717,483	1.50	1.30
February .....	308,060	19,712	89,962	4,996,236	4.37	4.20
March .....	363,240	52,976	165,448	10,173,067	8.90	7.72
April .....	231,520	36,344	81,056	4,823,156	4.22	3.78
May .....	134,948	26,543	50,124	3,082,025	2.70	2.34
June .....	38,126	21,494	29,107	1,731,983	1.52	1.36
July .....	74,657	21,791	34,428	2,116,909	1.86	1.61
August .....	34,562	14,366	25,847	1,589,280	1.39	1.21
September .....	14,960	6,050	8,951	532,620	.47	.42
October .....	13,772	4,268	7,842	482,189	.43	.37
November .....	9,614	6,050	7,350	436,164	.38	.34
December .....	62,183	8,129	24,627	1,514,265	1.33	1.15
The year .....	363,240	4,268	46,055	33,195,377	29.07	2.15
1898.						
January .....	124,860	14,344	59,509	3,778,785	3.20	2.77
February .....	47,115	16,900	22,994	1,277,022	1.11	1.07
March .....	88,725	15,008	24,774	1,523,304	1.28	1.11
April .....	121,940	39,085	60,048	3,573,096	3.12	2.80
May .....	38,720	17,920	23,701	1,457,327	1.28	1.11
June .....	32,515	10,245	16,395	975,569	.85	.77
July .....	49,670	11,853	20,063	1,233,633	1.08	.94
August .....	107,705	20,410	50,638	3,113,629	2.72	2.36
September .....	174,500	18,190	47,349	2,817,454	2.46	2.21
October .....	115,370	16,900	44,215	2,718,691	2.38	2.06
November .....	42,005	20,700	28,415	1,690,806	1.48	1.33
December .....	35,800	19,000	28,909	1,777,556	1.56	1.35
The year .....	174,500	10,245	35,584	25,936,872	22.52	1.66

*Estimated monthly discharge of Tennessee River at Chattanooga, Tennessee—Continued.*

[Drainage area, 21,418 square miles.]

Month.	Discharge in second-feet.			Total in acre-feet.	Run-off.	
	Maxi-mum.	Mini-mum.	Mean.		Depth in inches.	Second-feet per square mile.
1899.						
January .....	112,560	25,450	47,250	2,905,289	2.55	2.21
February .....	233,150	30,720	95,554	5,306,801	4.64	4.46
March .....	244,000	55,210	142,700	8,774,281	7.68	6.66
April .....	137,360	39,710	69,286	4,122,803	3.59	3.23
May .....	65,130	22,040	40,450	2,487,173	2.18	1.89
June .....	35,990	15,600	23,088	1,373,831	1.20	1.08
July .....	27,930	10,430	15,053	925,573	.81	.70
August .....	22,040	8,040	11,900	731,702	.64	.56
September .....	15,840	7,300	10,118	603,063	.53	.47
October .....	10,635	6,600	7,851	482,739	.43	.37
November .....	10,635	6,775	8,216	488,886	.43	.38
December .....	41,880	8,820	22,061	1,356,478	1.19	1.03
The year .....	244,000	6,600	41,127	29,557,619	25.87	1.09
1900.						
January .....	54,280	11,660	30,807	1,894,248	1.66	1.44
February .....	144,800	13,360	52,077	2,892,210	2.53	2.43
March .....	103,880	42,810	66,020	4,059,412	3.55	3.08
April .....	70,400	32,820	46,819	2,785,924	2.44	2.19
May .....	34,440	15,600	21,086	1,296,528	1.13	.98
June .....	53,040	14,680	33,295	1,981,190	1.73	1.55
July .....	50,870	13,360	24,874	1,517,145	1.33	1.15
August .....	34,440	10,020	14,602	897,841	.78	.68
September .....	25,140	7,300	13,393	796,939	.70	.63
October .....	42,500	8,040	14,230	874,968	.76	.66
November .....	92,720	10,840	25,138	1,495,815	1.31	1.17
December .....	53,040	17,050	29,001	1,783,201	1.56	1.35
The year .....	144,800	7,300	30,928	22,275,421	19.48	1.44
1901.						
January .....		189,200	19,600	50,641	2.72	2.36
February .....		59,850	19,050	36,516	1.77	1.70
March .....		139,150	19,050	44,952	2.42	2.10
April .....		166,450	38,400	95,080	4.95	4.44
May .....		205,450	26,380	68,736	3.70	3.21
June .....		72,200	33,850	47,673	2.49	2.23
July .....		37,100	14,680	23,932	1.29	1.12
August .....		207,400	13,800	75,761	4.08	3.54
September .....		61,150	22,660	38,859	2.02	1.81
October .....		25,760	13,360	18,979	1.03	.89
November .....		13,800	11,660	13,076	.68	.61
December .....		237,300	12,080	65,509	3.53	3.06
The year .....		237,200	11,660	48,310	30.68	2.26

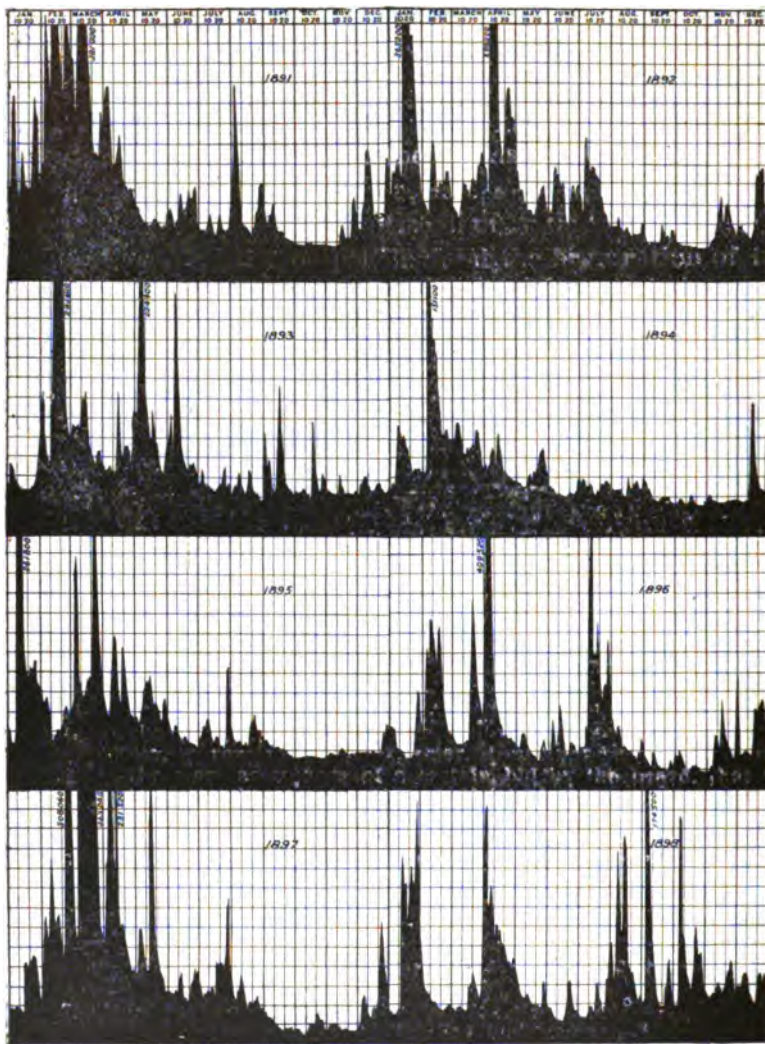


Fig. 16—Discharge of Tennessee River at Chattanooga, Tenn, 1891-1898.

*Minimum monthly discharge of Tennessee River at Chattanooga, Tenn., with corresponding net horsepower per foot of fall on a water wheel realizing 80 per cent. of the theoretical power.*

	1899.			1900.			1901		
	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.	Minimum cubic feet per second.	Minimum net H. P. per foot of fall.	No. of days duration of minimum.
January .....	25,450	2,314	1	11,660	1,060	1	19,600	1,782	1
February ...	30,720	2,793	1	13,360	1,215	1	19,050	1,732	1
March .....	55,210	5,019	1	42,810	3,892	2	19,050	1,732	4
April .....	39,710	3,610	1	32,820	3,075	1	38,400	3,491	1
May .....	22,040	2,004	1	15,600	1,418	1	23,390	2,398	1
June .....	15,600	1,418	1	14,680	1,335	1	33,850	3,077	1
July .....	10,430	948	1	13,360	1,215	2	14,680	1,335	3
August .....	8,040	731	3	10,020	911	1	13,800	1,255	2
September ..	7,300	664	1	7,300	664	1	22,660	2,060	1
October ....	6,600	600	3	8,040	731	2	13,360	1,215	2
November ..	6,775	616	2	10,840	985	2	11,660	1,060	1
December ..	8,820	802	2	17,050	1,550	2	12,080	1,098	1

NOTE.—To find the minimum net horse power available at a shoal on this stream, near this station, for any month, multiply the total fall of the shoal by the "net H. P. per foot of fall" in this table for that month.

## 2. SHOALS IN TENNESSEE RIVER NEAR FLORENCE, ALABAMA.

In Tennessee River, in the vicinity of Florence, Ala., (see Fig. 96), are several shoals capable of the development of power in large quantities. The compiler has brought together the data regarding these, his intention being not to discuss the manner in which the immense water power of these shoals can be developed, but to give some idea of its magnitude and the possibility of its utilization.

The shoals are a succession of cascades amid many islands, in a river bed varying in width from a half mile to three miles. The numerous channels thus formed are very irregular in fall and direction. The difference between high and low water is only 5 or 6 feet, corresponding to a rise of 50 feet at Chattanooga. Beginning at Brown's Ferry, 12 miles below Decatur, Ala., the river has the following falls:

From Browns Ferry to the mouth of Elk River the fall is 26 feet in 11 miles. This is known as Elk River Shoals. Its most precipitous part is at the lower end, where there is a fall of 16.5 feet in about 4 miles.

From the mouth of Elk River to the head of Muscle Shoals, a distance of 5 miles, there is a fall of only 2 feet.

From the head of Muscle Shoals to Bainbridge the fall is 85 feet in 17 miles, and is known as Big Muscle Shoals.

From Bainbridge to Florence the fall is 23 feet in 7 miles, and is known as Little Muscle Shoals.

From Florence to the head of the Colbert Shoals the fall is 3 feet in 11 miles.

From the head of the Colbert Shoals to Waterloo the fall is 21 feet in 6 miles.

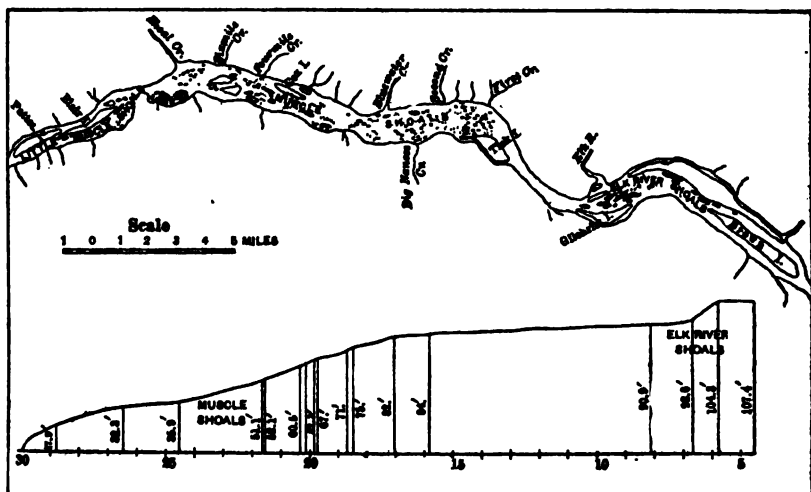


Fig. 17.—Map showing shoals in Tennessee River, near Florence, Ala.

The total fall from Browns Ferry to Waterloo is, therefore, 160 feet in a distance of 57 miles. Sixteen miles of the distance, however, has a fall of only 5 feet, leaving a fall of 155 feet in the 41 miles that cover the four shoals mentioned. The shoals are really more precipitous than the foregoing figures would indicate. For instance, 84.6 feet of the fall at Big Muscle Shoals is in a distance of 14 miles.

The bed rock at Elk River Shoals is Carboniferous limestone; that of Muscle Shoals is a hard siliceous rock of dark color and flinty structure.

The following is a statement of the minimum discharge of Tennessee River at Chattanooga:

\*The numbers 5 to 30 at bottom of cut represent miles.



	<i>Sec.-ft.</i>
From 1890 to 1895, inclusive .....	16.
From 1896 to 1900, inclusive.....	6,600
From January 1 to November 16, 1901, inclusive .....	12,930

From this it is estimated that 6,600 second-feet is the minimum discharge for driest years, and that 12,930 second-feet is the minimum for average years. Assuming that tributaries entering the river below Chattanooga will safely supply all of the water needed for lockage, we can use these discharges in estimating the water power of these shoals, which are about 200 miles below Chattanooga, by river, and drain an area more than 7,000 square miles greater than the watershed above Chattanooga.

*Estimated minimum net horsepower of Tennessee River in Alabama on turbines realizing 80 per cent. of the theoretical power.*

Locall.y.	Fall	Minimum net power in driest years.	Minimum net power in average years.
	<i>Fall.</i>	<i>Horsepower.</i>	<i>Horsepower.</i>
Elk River Shoals .....	26	15,600	30,550
Big Muscle Shoals .....	85	51,000	99,875
Little Muscle Shoals .....	23	13,800	27,025
Colbert Shoals .....	21	12,600	24,675
Total .....	155	93,000	182,125

The foregoing table assumes that the total fall can in each case be utilized. While this assumption is not correct, it stands as an offset to the assumption that the water supply available will be as low as the minimum discharge at Chattanooga, 200 miles above. The drainage area above Chattanooga is 21,418 square miles, while the drainage area above the shoals under consideration is about 29,000 square miles. It may therefore safely be assumed that the actual power available for development at the shoals is greater than that shown by the table.

The foregoing statements of fall and distance are from a report by Mr. William B. Gaw, chief assistant engineer, United States Army, 1868, and the map and profile are from drawings prepared under the direction of Lieut. Col. J. W. Barlow, United States Engineers, 1890.

## 3. TRIBUTARIES OF TENNESSEE RIVER.

Paint Rock Creek, Elk River, Shoal Creek, Flint Creek, Big Nance Creek, Town Creek, and Big Bear Creek are all large streams, and most of them have fine undeveloped water powers. But no surveys have been made of them, and no measurements of discharge so far. There are also many large bold springs in this basin, that are said to have a pure and unfailing water supply, but the Hydrographic Survey has not reached them, and no report can be made on them at this time.



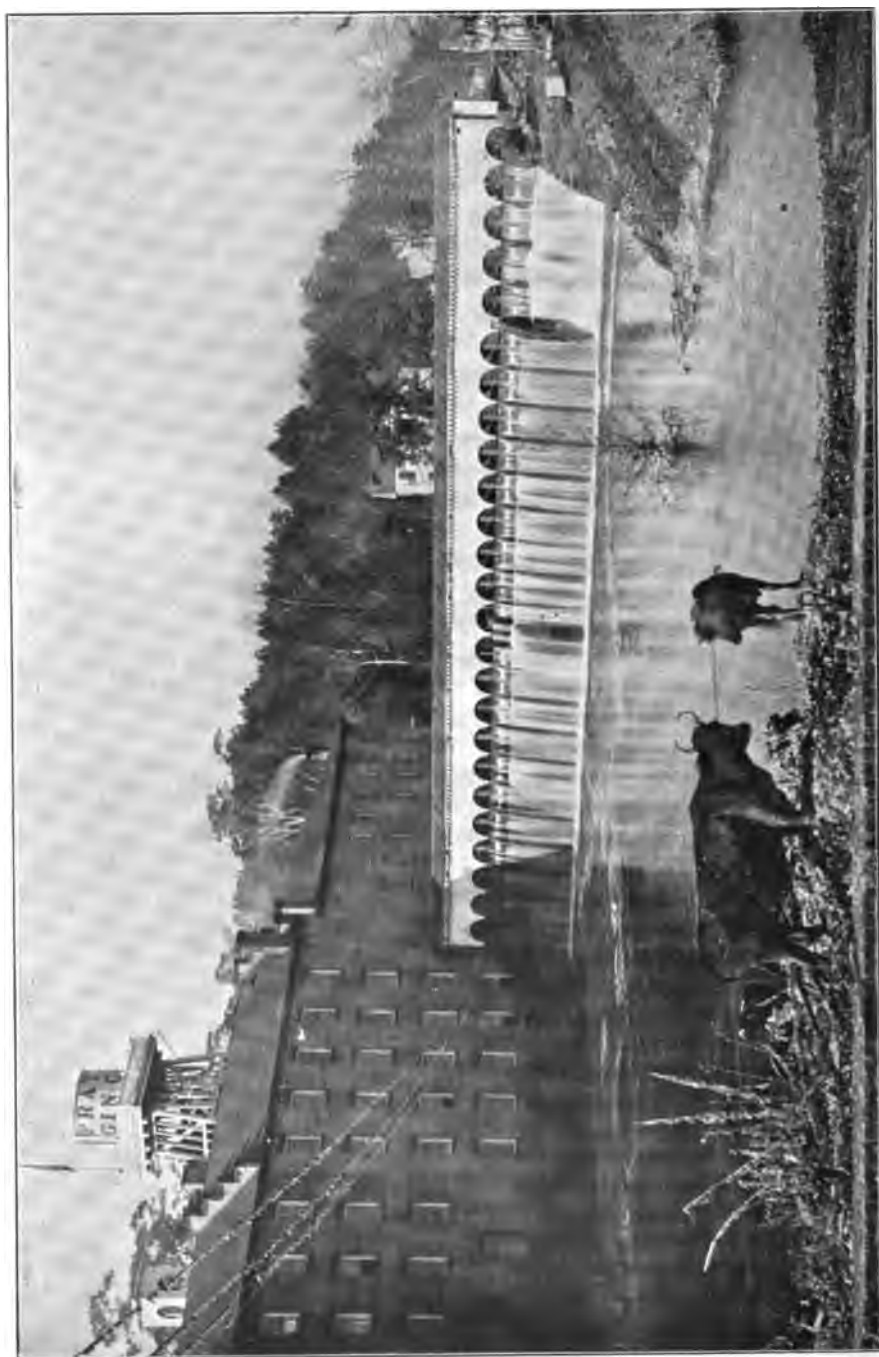


Plate D. Dam of the Pratt Gin Company, on Autauga Creek, Prattville, Ala.

## CHAPTER VIII.

### UTILIZED WATER POWERS OF ALABAMA.

The following is a list by counties of the water powers that are utilized. The most of these powers are small, but they make a large aggregate, and they represent only an insignificant part of the power that is capable of development.

#### \*AUTAUGA COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Charity P. Carter, Billingsley, flour and grist mill.....			15
Montgomery's Mill, Prattville, flour and grist mill.....			30
Public Grist Mill, Billingsley, flour and grist mill .....			9
Parker's Mill, Milton, flour and grist mill.....			20
Dawson's Mill, Netezen, lumber and timber mill.....			20
Ellis Mill, Jones Switch, lumber and timber mill.....			4
* Long Leaf Yellow Pine Saw Mill, Autaugaville, lumber and timber mill .....			15
Ray's Saw Mill, Jones Switch, lumber and timber mill.....			10
Swift Creek Mill Co. (Swift Creek), Autaugaville, lumber and timber mill .....			70
John H. Herod, Netezen, lumber and timber mill.....			6
Prattville Cotton M. & Banking Co. (Autauga Creek), Pratt- ville, cotton goods. The dam at Prattville is shown in Plate D opposite .....			200
† Continental Gin Co., (Autauga Creek), Prattville, cotton gin Prattville Ice Factory (Autauga Creek), Prattville, ice factory Doster Ginnery (Autauga Creek), Prattville, cotton gin.....			
G. H. Roy, Vine Hill, cotton gin .....			

#### WATER POWER AT PRATTVILLE.

The water power at Prattville was first developed about 1830, when it was used by a man named May to operate a small saw mill. About 1833 this water power and the adjacent lands were purchased by Mr. Daniel Pratt, who then erected a cotton gin factory, which was driven by the water power. The dam at that time was about eight feet high. A number of years after the purchase of this property by Mr. Pratt he increased the dam so that it now has a height of 16 feet, and is built of brick. At present it is used jointly by the Prattville Cotton Mills & Banking Company and the Continental Gin Company, the former using about 255 horse-power and the latter about 100 horse-power. About half a mile below the dam above referred to is another dam affording about 8 feet head, and owned by the M. E. Pratt estate. This power operates a grist

\*From U. S. Census, 1900.

†From report of Probate Judge.

mill, cotton ginnery and ice factory, and the water wheel at that point has a rated capacity of 54 horse-power. About one mile above the dam of the Cotton Mill and Gin Company, there was formerly another dam 12 feet high, which afforded power for a cotton mill. This mill, however, was burned a number of years ago, and the dam has been allowed to go to ruin. It would probably afford 200 horse-power, or possibly a little more, should it be rebuilt.

There is also a dam about two miles below Prattville known as the Montgomery mill property. This dam is about 12 feet high and affords power for a grist mill and ginnery. Only a small portion of the available power is used. It could afford, easily, 250 horse-power if the proper wheels were installed.

#### \*BARBOUR COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Hagler's Mill, Louisville, flour and grist mill.....			17
Carpenter's Mill, Louisville, flour and grist mill.....			15
Hoffman's Mill, Clayton, flour and grist mill .....			50
Hartman's Mill, Clayton, flour and grist mill .....			10
Zorn Mills, Lodi, flour and grist mill .....			8
William M. Wood, Bush, flour and grist mill.....			12
Will Stewart, White Oak Springs, flour and grist mill.....			12
Winn's Mill, Clayton, flour and grist mill.....			12
John White, Spivey, flour and grist mill .....			10
Weston's Mill, Louisville, flour and grist mill.....			8
H. J. Turner, White Oak Springs, flour and grist mill.....			10
Spencer's Mill, Clayton, flour and grist mill.....			10
Perkin's Mill, Elamville, flour and grist mill .....			12
Angus McSwain, White Oak Springs, flour and grist mill.....			12
William Johnson, Clayton, flour and grist mill.....			10
John M. Jenkins, Starhill, flour and grist mill.....			10
Solomon's Mills, Solomon's Mills, flour and grist mill.....			25
Danner Mill, Elamville, flour and grist mill.....			12
William H. Chambers, Oateston, flour and grist mill.....			12
Wilson Deshazo, Cottonhill, flour and grist mill.....			16

#### BIBB COUNTY.

*	Scottsville Flour & Grist Mill, Scottsville, flour and grist mill	30
	Palmetto Flouring & Grist Mill, Brierfield, flour and grist mill	30
	Williams Grist Mill, Blocton, flour and grist mill.....	10
	William S. Mathews, Data, flour and grist mill.....	8
	Six Mile Custom Mill, Six Mile, flour and grist mill.....	15
†	Mayfield Bros., Mertz, lumber and timber mill.....	29
	Scottsville, Wool Carder, Scottsville, woolen goods.....	20
	J. M. Battle, (Six Mile Creek), Six Mile, flour and grist mill.	50
	W. C. Trott, (Six Mile Creek), Six Mile, cotton gin and grist mill.....	50
	W. H. Thomas, (Six Mile Creek), Ashley, lumber and grist mill	35
	Dock Mahan, (Mahn's Creek), Brierfield, wool carder and grist mill .....	40

\*From U. S. Census, 1900.

†From report of Probate Judge.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
†	Bessemer Land & Improvement Co. (Schultz Creek), Lopez,		
	wool caruer, grist mill and cotton gin .....		100
	R. R. McCally, (Hills Creek), Blocton, gin, lumber and grist		30
	E. M. Timbro, (Schultz Creek), Centerville, grist mill.....		30
	F. H. James, (Haysoppy Creek), Centerville, grist mill.....		20
	A. L. Elam, (Affonee Creek,) Affonee, grist mill.....		15

## \*BLOUNT COUNTY.

Logan Snead, Snead, flour and grist mill.....	10
E. B. Head, Gum Spring, flour and grist mill.....	16
E. R. Wood, Wynnville, flour and grist mill.....	8
Hendrick's Mill, Swansea, flour and grist mill.....	30
Jones M. Burns, Clarence, flour and grist mill.....	15
Wilson Adcock, Tidmore, flour and grist mill.....	10
G. M. D. Tidwell & Sons, Tidwell, flour and grist mill.....	20
Alldridge & Brother, Liberty, flour and grist mill.....	10
Brittain Mill, Summit, flour and grist mill.....	20
Morris' Mill, Ensley, flour and grist mill.....	10
Rufus F. Wyatt, Bangor, flour and grist mill.....	10
Sam Mardis, Blountville, flour and grist mill.....	60
Jno. H. Donahoo & Geo. W. Darden, Rosa, lumber and timber	20

## BULLOCK COUNTY.

Brooks' Mill, Mascotte, flour and grist mill.....	6
Union Springs Waste Mill, Union Springs, flour and grist mill	15
Chappell's Grist Mill, Union Springs, flour and grist mill.....	10
D. H. Mason, (McBride's Creek), Indian, lumber, gin and grist	20
From report of Probate Judge.	
†Chas. Radford, (Conecuh Creek), Union Springs, grist mill..	10

## \*BUTLER COUNTY.

John W. Halso, Pigeon Creek, flour and grist mill.....	10
Glen Graham, Pontus, flour and grist mill.....	6
The Four Mile Mill, Greenville, flour and grist mill.....	10
The N. M. Rhodes Mill & Mercantile Co., Shell, flour, grist and lumber mill .....	50
Mrs. M. E. Crane, Monterey, flour and grist mill.....	15
Rouse & Whiddon, Greenville, flour and grist mill.....	50

## CALHOUN COUNTY.

*	Joseph Francis, Cane Creek, flour and grist mill.....	50
	Richey Mill, Jacksonville, flour and grist mill.....	20
	Canada Grist Mill, Womack, flour and grist mill.....	16
	Cold Water Mills, Cold Water, flour and grist mill.....	20
	Read's Mill, Reads, flour and grist mill.....	60
	Luther Barton, Piedmont, flour and grist mill.....	20
	W. F. McCulley, Oxford, flour and grist mill.....	20
	A. McCurdy, White Plains, flour and grist mill.....	34
	Morris Grist Mill, Morrisville, flour and grist mill.....	18
	Nisbet's Mill, Jacksonville, flour and grist mill.....	30
	James A. Weatherly, DeArmanville, flour and grist mill.....	8

\*From U. S. Census, 1900.

†From report of Probate Judge.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Wood Milling Co., Ohatchee, flour and grist mill.....			26
* { Davis & Henderson, Piedmont, flour and grist mill.....			24
Hendon's Grist Mill, Iron City, flour and grist mill.....			10
Hughes' Saw Mill, Oxford, lumber and timber mill.....			28
F. M. Whiteside, (Choccolocco Creek), White Plains.....		25 or	30
Downing & Morris, (Choccolocco Creek), Choccolocco .....			50
J. T. DeArman, (Choccolocco Creek), Anniston .....			15
W. E. Mellon, (Choccolocco Creek), Oxford .....			40
Lee's Mill, (Choccolocco Creek), Oxford.....			30
T. G. Slaughter, (Choccolocco Creek), Oxford .....			15
J. H. Savage, (Terrapin Creek), Anniston .....			20
J. H. Savage, (Terrapin Creek), Anniston .....			20
Frank Aderhold, (Nances Creek), Ladiga.....			20
John Ramagnand, (Champion Creek), Jacksonville.....			15
James Crook, (Tallasseehatchee Creek), Jacksonville.....			10
W. J. Edmondson, (Tallasseehatchee Creek), Anniston .....			30
W. A. Prickett, (Tallasseehatchee Creek), Alexandria.....			10
Beaty Estate, (Tallasseehatchee Creek), Alexandria.....			30
† { Peter Helfner, (Tallasseehatchee Creek), Alexandria.....			15
James Aderhold, (Ohatchee Creek), Reads.....			20
Pleas. Martin, (Ohatchee Creek), Peekshill.....			25
C. J. Wood, (Ohatchee Creek), Jacksonville.....			30
Wm. Thompson, (Ohatchee Creek), Peekshill.....			8
R. L. Treadway, (Tallasseehatchee Creek), Anniston, R. F. D .....			10
J. H. Francis, (Tallasseehatchee Creek) .....			25
R. H. Cobb, (Tallasseehatchee Creek), Anniston.....			20
G. W. S. Loyd, (Cane Creek), Peaceburg.....			10
Mrs. Loyd, (Cane Creek), Peaceburg, gin .....			6
Morris Mfg. Co., (Cane Creek), Morrisville, shops.....			30
E. G. Morris, (Cane Creek), Morrisville .....			30
P. H. Brothers, (Cane Creek), Zula .....			30
J. H. Francis, (Cane Creek) .....			50

## \*CHAMBERS COUNTY.

D. E. M. Smith, Barber, flour and grist mill.....	24
Cumbees Grist Mill, Stroud, flour and grist mill.....	20
Thomas H. Fuller, Lafayette, flour and grist mill.....	10
R. T. Humphrey, West Point, Ga., flour and grist mill.....	42
J. T. Hudson, Hickory Flat, flour and grist mill.....	4
Wyche Robinson, Lafayette, flour and grist mill.....	16
Stephens' Mill, Driver, flour and grist mill.....	8
Ripville Mills, Wise, flour and grist mill.....	20
Charles F. Higgs, Finley, flour and grist mill.....	20
J. E. Dixon, Lafayette, flour and grist mill.....	10
Ratchford & Tucker, Lafayette, flour and grist mill.....	10
Benjamin F. Knight, Lafayette, flour and grist mill.....	10
Woody & Beall, Moorefield, flour and grist mill.....	6
Leverett's & Abernathy's Mill, Milltown, flour and grist mill..	4
John B. Calhoun, Camphill, flour and grist mill.....	8
G. L. Leverett, Lafayette, flour and grist mill.....	16
West Point Mfg. Co., West Point, cotton goods.....	1,100

\*From U. S. Census, 1900.

†From report of Probate Judge.

‡From report of L. J. Morris.



## CHEROKEE COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Shamblin & Toles Mill, Broomtown, flour and grist mill.....			8
Chandler & Stinson, Center, flour and grist mill.....			20
Shamblin & Toles Mill, Broomtown, flour and grist mill.....			20
J. A. Lumpkin, Forney, flour and grist mill.....			13
Hurleys Mill, Hurley, flour and grist mill.....			12
Tyre G. Craig, Grover, flour and grist mill.....			12
* Rush Mill, Lawrence, flour and grist mill.....			10
E. W. Ragdale, Spring Garden, flour and grist mill.....			30
W. F. Timmerman, Round Mountain, flour and grist mill....			8
M. E. Cohia, Cedar Bluff, flour and grist mill.....			24
M. J. Abernathy, Pleasant Gap, lumber and timber mill.....			15
Hurricane Creek Mfg. & Min. Co., Spring Garden, cotton goods			65
W. A. Stinson, (Terrapin Creek), Center, gin, flour and grist			60
J. J. Scroggin, (Terrapin Creek), Coloma, gin, flour and grist			60
T. F. Stewart, (Terrapin Creek), Spring Garden, flour and grist			60
J. M. Adderhold, (Mill Creek), Piedmont, flour, grist and gin			40
M. L. Braswell, (Hurricane Creek), Pleasant Gap, flour & grist			40
B. F. Newberry, (Yellow Creek), Round Mountain, flour, grist,			
and gin mill .....			40
E. Cobia, (Chattooga River), Cedar Bluff, flour, grist, and gin			60
† R. A. Russell & Co. (Chattooga River), Gaylesville, flour, grist			
and gin mill .....			60
W. F. Henderson, (Mill Creek), Fullerton, flour, grist and gin			40
Rush & Rinehart, (Chattooga River), Fullerton, flour, grist, gin			60
J. G. Toles, (Mill Creek), Broomtown, grist and gin mill....			40
Elliott Bros., (North Spring Creek), Grassland, grist and gin			40
J. T. Webb & Bros., (Spring Creek), Hurley, grist and gin mill			40
J. D. Jordan, (South Spring Creek), Noah, grist and gin mill..			20

## \*CHILTON COUNTY.

James Dorming, Jemison, flour and grist mill.....	10
Mahan's Mill, Clanton, flour and grist mill.....	20
W. W. Sansome, Adams, flour and grist mill.....	12
Honeycutt Mill, Jemison, flour, grist, lumber and timber mill	20

## \*CHOCTAW COUNTY.

Pink Blackwell, Hinton, flour and grist mill.....	12
Aquilla Mills, Aquilla, lumber and timber mill.....	16

## \*CLARKE COUNTY.

Gate's Mill, Vashti, flour and grist mill.....	30
Fleming's Grist Mill, Nealton, flour and grist mill.....	10
Dacy's Mill, Whatley, flour and grist mill.....	5

\*From U. S. Census, 1900.

†From report of Probate Judge.

## \*CLAY COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Henry F. Smedley, Mellow Valley, flour and grist mill.....			15
Hezakiah Ingram, Hatchett Creek, flour and grist mill.....			10
Allen P. Jenkins, Delta, flour and grist mill.....			14
Knight's Mill, Wesobulga, flour and grist mill.....			14
F. M. Munroe, Millerville, flour and grist mill.....			40
John R. Gilbert, Pinckneyville, flour and grist mill.....			8
Hodnett & Co., Hat, flour and grist mill.....			19
Moses R. Watts, Dean, flour and grist mill.....			6
Thomas J. Watts, Shinbone, flour and grist mill.....			8
Bishop, Carpenter & Co., Cherry, flour and grist mill.....			10
Cockrell & Mitchell, Goldburg, flour and grist mill.....			14
McRairie, Gladney & Co., Cherry, flour and grist mill.....			20
Virginia Whellen, Coleta, flour and grist mill.....			6
Stephens & East, Delta, flour and grist mill.....			4
Deberry & Griffin, Flatrock, flour and grist mill.....			15
Child's Mill, Swann, flour and grist mill.....			5
James B. Brown, Pinckneyville, flour and grist mill.....			6
James J. Bachus, Fishhead, flour and grist mill.....			24
Brooks & Handley, Hatchett Creek, flour and grist mill.....			8
Columbus Bell, Lineville, lumber and timber mill.....			10
J. C. Kennedy, Fishhead, lumber and timber mill.....			14
William M. Patterson, Meadow, lumber and timber mill.....			30
Ward & Ford, Lineville, lumber and timber mill.....			15

## \*CLEBURNE COUNTY.

J. T. & E. W. Beason, Beasons Mill, flour and grist mill.....	10
W. M. Evans, Edwardsville, flour and grist mill.....	20
Robert Mill, Oaklevel, flour and grist mill.....	16
Teague & Co., Eudora, flour and grist mill.....	13
H. F. Alsabrook, Borden Springs, flour and grist mill.....	30
Buttram's Mill, Bucham, flour and grist mill.....	20
John A. Brown, Bell Mills, flour and grist mill.....	16
John I. Burgess, Edwardsville, flour and grist mill.....	20
Wade H. Barnes, Muscadine, flour and grist mill.....	4
J. W. Conner, Chulafinnee, flour and grist mill.....	6
Lyon & Killebrue, flour and grist mill.....	34
W. G. Milligan, Oakfuskee, flour and grist mill.....	8
James McMahan, Edwardsville, flour and grist mill.....	12
E. W. Pitchford, Oaklevel, flour and grist mill.....	15
William J. Thrash, Oakfuskee, flour and grist mill.....	6
Wade H. Barnes, Muscadine, flour and grist mill.....	30
W. H. Tumlin & D. S. Baber, Ai, flour and grist mill.....	16

## \*COFFEE COUNTY.

Levy Wise, Ino, flour and grist mill.....	5
Bell Mill, Dot, flour and grist mill.....	8
Lenora F. Hildreth, Enterprise, flour and grist mill.....	17
Harper Flour Mills, Brockton, flour and grist mill.....	4
F. M. Prestwood, Fresco, flour and grist mill.....	20
McIntosh Mill, Eta, flour and grist mill.....	8
Wise's Lower Mill, Elba, flour and grist mill.....	12
Wise's Upper Mill, Elba, flour and grist mill.....	10
Buck & Co., Penn, lumber and timber mill.....	50

\*From U. S. Census, 1900.

## \*COLBERT COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	M. P.
George Martin, Allsboro,	flour and grist mill.....		8
James Burns, Mand,	flour and grist mill.....		4
Tuscumbia Mill, Tuscumbia,	flour and grist mill.....		40
C. C. Hester, Tuscumbia,	flour and grist mill.....		40
Chambee's Grist Mill, Tuscumbia,	flour and grist mill.....		8
Dillard's Mills, Russellville,	lumber and timber.....		12
Steenenson's Mill, Sheffield,	lumber and timber.....		30

## \*CONECUH COUNTY.

George Stenson, Bonnette,	flour and grist mill.....	12
James B. Pate, Brooklyn,	flour and grist mill.....	5
William M. Robinson, Brooklyn,	flour and grist mill.....	5
Jimson C. Cox, Gem,	flour and grist mill.....	5
John N. Varner & Chas. M. Varner,	Herbert, flour and grist..	10
James E. Wilson, Mount Union,	flour and grist mill.....	20
Ransom H. Finley, Zern,	flour and grist mill.....	8
G. G. Broker, Bowles,	lumber and timber mill.....	10
Cary & Johnston, Brooklyn,	lumber and timber mill.....	15
T. N. Piggott, Gravelle,	lumber and timber mill.....	40
Robinson Bros., Brooklyn,	lumber and timber mill.....	30
H. J. Robinson, Burnt Corn,	lumber and timber mill.....	40
Henry Wills, Finklet,	lumber and timber mill.....	30

## \*COOSA COUNTY.

Miller's Mill, Bentleyville,	flour and grist mill.....	20
Nolen's Mill, Darden,	flour and grist mill.....	15
J. T. M. Hodnett & O. P. Hodnett,	Equality, flour and grist mill	12
W. N. Neighbors, Goodwater,	flour and grist mill.....	23
Smith's Mill, Nixburg,	flour and grist mill.....	10
George P. Waits, Rockford,	flour and grist mill.....	8
Crawford Mill, Rockford,	flour and grist mill.....	4
Lawson Grist and Saw Mill,	Rockford, lumber and timber mill	36

## \*COVINGTON COUNTY.

A. J. Fletcher, Andalusia,	flour and grist mill.....	10
Uatu Grist Mill, Andalusia,	flour and grist mill.....	10
William Sharp, Ealums,	flour and grist mill.....	10
Davis B. Gantt, Gantt,	flour and grist mill.....	12
C. E. Rawls, Gantt,	flour and grist mill.....	10
Dorsey's Mill, Glaslasko,	flour and grist mill.....	10
James Aplin, Green Bay,	flour and grist mill.....	20
William Watkins, Liberty Hill,	flour and grist mill.....	8
Kearsey's Mill, Redlevel,	flour and grist mill.....	5
Ephram F. Lassiter, Rosehill,	flour and grist mill.....	10
Thomas Saw Mill, Redlevel,	lumber and timber mill.....	25
Simmons Mill, Beck,	lumber and timber mill.....	40
J. A. Prestwood, Jr., Andalusia,	lumber and timber mill.....	40
George W. Lee, Rat,	lumber and timber mill.....	20
Buck Creek Mill, River Falls,	lumber and timber.....	80
J. F. Guthrie, Vera Cruz,	lumber and timber mill.....	25
Gunter's Mill, Andalusia,	lumber and timber mill.....	40
Gunter's Saw Mill, Gantt,	lumber and timber mill.....	15
Gantt's Mill, River Falls,	lumber and timber mill.....	70
Pollard Gantt, Searight	lumber and timber mill.....	35
Davis B. Gantt, Gantt,	lumber and timber mill.....	40
N. B. Dixon, Mason,	lumber and timber mill.....	60
Bartlett & Barker,	lumber and timber mill.....	60

\*From U. S. Census, 1900.

## \*CRENSHAW COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
E. P. Lasseter, Bullock, flour and grist mill.....			8
G. B. Morgan, Bullock, flour and grist mill.....			15
Folmar's Mill, Goshen, flour and grist mill.....			8
N. Skipper, Honoraville, flour and grist mill.....			10
Daniel & Co., Lapine, flour and grist mill.....			30
John S. Marsh, Rutledge, flour and grist mill.....			20
G. B. Sasser, Luverne, flour and grist mill.....			15

## \*CULLMAN COUNTY.

Joseph W. Hyatt, Baileyton, flour and grist mill.....	10
Miles Humphries, Baileyton, flour and grist mill.....	4
D. H. Laney, Battleground, flour and grist mill.....	6
Robert J. Waldrop, Cranehill, flour and grist mill.....	20
Andrew J. Miller, Summit, flour and grist mill.....	6

## \*DALE COUNTY.

Archer McCall, Candy, flour and grist mill.....	10
Floyd Mill, Dothan, flour and grist mill.....	10
Lewis Mill, Clopton, flour and grist mill.....	15
Murphy Mill, Dothan, flour and grist mill.....	5
Maunds Corn Mill, Ewells, flour and grist mill.....	10
Pope's Mill, Grimes, flour and grist mill.....	60
Charles Thrower, Kleg, flour and grist mill.....	16
Daniel McSwean, Ozark, flour and grist mill.....	20
Preston's Mill, Peach, flour and grist mill.....	20
The Kelley Grist Mill, Pinckard, flour and grist mill.....	150
Atkinson's Saw Mill, Newton, lumber and timber mill.....	16
J. F. Bell, Daleville, lumber and timber mill.....	22

## \*DALLAS COUNTY.

Calhoun's Mill, Carlownville, flour and grist mill.....	10
Ivey & Williams, Morrowville, flour and grist mill.....	8

## DEKALB COUNTY.

L. D. Wooten, Blake, flour and grist mill.....	8
J. D. Hall, Chavies, flour and grist mill.....	10
J. S. Ward, Chumley, flour and grist mill.....	12
Kean & Warren, Cordell, flour and grist mill.....	20
Swindell's Mill, Cotnam, flour and grist mill.....	12
Griffin's Mill, Cotnam, flour and grist mill.....	12
Emeline Clayton, Crossville, flour and grist mill.....	6
Swader's Mill, Dekalb, flour and grist mill.....	15
James Clark, Eula, flour and grist mill.....	15
* David J. Harper, Floy, flour and grist mill.....	3
Elrod's Grist Mill, Flay, flour and grist mill.....	4
Davis Mill, Fort Payne, flour and grist mill.....	16
Thomas F. Everett, Luna, flour and grist mill.....	8
Elrod's Mill, Geraldine, flour and grist mill.....	30
Pruitt's Mill, Skirum, flour and grist mill.....	12
Lebanon Flour & Grist Mill, Lebanon, flour and grist mill....	36
Robert F. Ellison, Mentone, flour and grist mill.....	25
Ellic Ellsworth, Opair, flour and grist mill.....	6
Warren's Grist Mill, Portersville, flour and grist mill.....	12

\*From U. S. Census, 1900.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
John F. Williams, Rains, flour and grist mill.....			8
Edward W. Williams, Rains, flour and grist mill.....			6
McGee's Mill, Sand Rock, flour and grist mill.....			5
Charles G. Matheny, Sauty Mills, flour and grist mill.....			20
Dixie Mills, Sulphur Springs, flour and grist mill.....			10
* Phillips' Mill, Valleyhead, flour and grist mill.....			4
The Roberts Mill Co., Collinsville, flour and grist mill.....			25
W. E. Brown & Son, Sulphur Springs, lumber and timber mill..			15
James M. Durham, Chavies, lumber and timber mill.....			16
William C. Hill & Co., Blanche, lumber and timber mill.....			40
D. D. Hughes, Hughes, lumber and timber mill.....			15
Ward, Pickens & Co., Dawson, lumber and timber mill.....			15
John A. Davis, (Wills Creek), Fort Payne, grist mill and gin			
M. S. Brown and W. C. Thomas, (Lookout Creek), Sulphur			
Springs, flouring mill.....			
† D. D. Hughes, (Wills Creek), Hughes P. O.; flour & grist mill			
P. M. Frazier, (Wills Creek), Lebanon, flour and grist mill..			
S. D. Warren, (Wills Creek), Lebanon, flour and grist mill....			
Grif. Elrod, (Town Creek), South Hill, flour and grist mill....			
Durham & Co., (Town Creek), Chavies, flour, grist & saw mill			

## \*ELMORE COUNTY.

E. & H. T. Andrews, Channahatchee, flour and grist mill.....	25
Benjamin Spigener, Elmore, flour and grist mill.....	5
Sykes Mill, Sykes Mill, flour and grist mill.....	16
John C. Birt (Lancaster Old Mill,) Tallassee, flour and grist	24
Freeman's Grist Mill, Tallassee, flour and grist mill.....	5
J. J. Benson, Kowaliga, lumber and timber mill.....	20
J. T. Rogers, Spigners, lumber and timber mill.....	36
(From Chapter III.)	
†Tallassee Falls Mfg. Co., (Tallapoosa River,) Tallassee, cotton	
and woolen goods .....	8,900
Montgomery Power Co. (Tallapoosa River), Tallassee, electric	
transmission to Montgomery, Ala.....	5,600

## \*ESCAMBIA COUNTY.

Bradley Mill, flour and grist mill .....	10
S. S. Overstreet, Roberts, flour and grist mill.....	20
James F. Douglas, Mason, lumber and timber .....	25

\*From U. S. Census, 1900.

†This is the same company that is now organized under the name of the Mt. Vernon Woodbury Cotton Duck Company, with office at Montgomery, Ala.

## ETOWAH COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Wesson Mills, Attalla, flour and grist mill.....			25
Cox & Brother, Avery, flour and grist mill.....			6
B. H. Rogers, Etowahaton, flour and grist mill.....			40
T. G. Ewing, Ewings, flour and grist mill.....			60
John C. Rollins, Fenton, flour and grist mill.....			8
Reese Mill, Hill, flour and grist mill.....			10
John H. Helms, Ballplay, flour and grist mill.....			6
Ford & Sibert's Mill, Hokes Bluff, flour and grist mill.....			30
* Morgan & Cochran, Keener, flour and grist mill.....			8
W. J. Harris, Nix, flour and grist mill.....			12
John B. Burns, Seaborn, flour and grist mill.....			8
A. B. Stephens, Seaborn, flour and grist mill.....			8
W. H. Cobb, Steels Depot, flour and grist mill.....			20
P. C. Turner, Walnut Grove, flour and grist mill.....			30
P. C. Turner, Walnut Grove, woolen goods.....			13
W. M. Brothers & Son, Gallant, woolen goods.....			8
Gadsden Times-News, Gadsden, printing and publishing.....			4
J. M. Morague, (Big Wills Creek), Gadsden, grist mill.....			100
Wm. McClendon, (Big Wills Creek), Attalla, grist mill.....			40
+ — Griffith, (Big Wills Creek), Keener, grist mill.....			35
Bob Riggers, (Big Canoe Creek), Gadsden, grist mill.....			75
Tom Ewing, (Cane Creek), Gadsden, grist mill.....			40

## FAYETTE COUNTY.

Rodolphus Cotton, Bankston, flour and grist mill.....			20
D. G. Hester, Covin, flour and grist mill.....			12
* John W. Anthony, Glenallen, flour and grist mill.....			30
Landon Miles, Hester, flour and grist mill.....			13
Bishop Emick, Rena, lumber and timber mill.....			40
Phillip N. Fortenberry, Bankston, lumber and timber mill.....			8
W. L. Caine, (Sipsey River), Fayette, saw and grist mill.....			40
T. E. Newton & Bro., (Sispey River), Fayette, saw and grist			40
Lcurgas Ray, (Luxapellila Creek), Montcalm, saw and grist..			30
John Barnes, (Luxapellila Creek), Covin, gin and grist mill..			30
E. Bishop, (Luxapellila Creek), Rainy, saw, gin and grist mill			30
John Williams, (Luxapellila Creek), Covin, gin and grist mill..			30
Washington Hubbert, (Shirley Creek), gin and grist mill....			10
Gilpin & Jones, (Shirley Creek), saw, gin and grist mill.....			16
Jones & Jones, (Shirley Creek), Hugent, saw, gin & grist mill			20
P. N. Fortenberry, (Davis Creek, Bankston, saw, gin and grist			8
G. H. White, (Davis Creek), Davis Creek, saw, gin and grist			16
+ J. W. Blackburn, (Davis Creek), Davis Creek, saw, gin & grist			18
M. I. Barnette, (Davis Creek), Ridge, saw, gin and grist mill..			20
Dolphus Cotton, (Clear Creek), Bankston, saw, gin and grist			16
M. Miller, (Clear Creek), Bankston, saw, gin and grist mill..			
John G. Kizer, (North River), Berry Station, saw, gin & grist			40
Marshall Jones, (Bear Creek), Bear, saw, gin and grist mill			20
R. G. Walker, (Bear Creek), Bear, saw, gin and grist mill....			24
Landon Miles, (Stewart Creek), Hester, grist mill.....			12
J. T. McCaleb, (Mountain Creek), New River, grist mill.....			16
W. A. Ayers, (Beaver Creek), Fayette, gin and grist mill.....			12
G. W. Gray, (Boxes Creek), Stough, grist mill.....			16
Miles Whitson, (Clear Creek), Handy, grist mill.....			12
Bud Wade, (Hollingsworth Creek), New River, grist mill....			12

\*From U. S. Census, 1900.

†From report of Probate Judge.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
<b>*FRANKLIN COUNTY.</b>			
Helm's Mill, Belgreen, flour and grist mill.....			6
M. J. Height, Baggett, flour and grist mill.....			10
James McNair, Kirby, flour and grist mill.....			20
Andrew Posey, Igoburg, flour and grist mill.....			24
Thomas Watson, Phil Campbell, flour and grist mill.....			20
S. T. Bonds, Pleasant Site, flour and grist mill.....			80
Jes. S. Scott, Russellville, flour and grist mill.....			10
Sparks Mill, Underwood, flour and grist mill.....			10
John T. McAlister, Phil Campbell, lumber and timber mill....			10
<b>*GENEVA COUNTY.</b>			
Avant's Mill, Geneva, flour and grist mill.....			15
Lowry's Mill, Geneva, flour and grist mill.....			10
Bell's Mill, Fadette, flour and grist mill.....			15
W. J. Keith and R. Y. Daniels, Geneva, flour and grist mill..			15
Clark's Grist Mill, Highnote, flour and grist mill.....			4
Underwood's Grist Mill, Sanders, flour and grist mill.....			20
Condry's Grist Mill, Whitaker, flour and grist mill.....			15
John T. Coleman, Geneva, lumber and timber.....			30
Clark Bros. & Co., Wicksburg, lumber and timber mill.....			10
Wilson Deshoga, Dundee, lumber and timber.....			15
Nathan Hall, Dotham, lumber and timber.....			20
<b>HALE COUNTY.</b>			
William Steward, Fivemile, flour and grist mill.....			8
William A. Avery, (Five-Mile Creek), Five-Mile, flour and grist			10
J. H. Payne & Co., Ingram, flour and grist mill.....			10
M. M. Avery, Havanna, flour and grist mill.....			15
Pickens Mill, Greensboro, lumber and timber mill.....			15
Greensboro Carriage & Wagon Shops, Greensboro, carriages and wagons .....			6
Richardsons Mills, (Five-Mile Creek), Five-Mile, grist mill and gin .....			20
J. H. Payne's Mill, (Five-Mile Creek), Havana, grist and gin..			20
Avery's Mill, (Five-Mile Creek), Havana, grist mill and gin..			25
J. A. Stephenson, (Prairie Creek), Newbern, grist mill & gin..			20
Irwin & Martin, (Big Creek), Greensboro, grist mill and gin..			25
<b>*HENRY COUNTY.</b>			
Kennedy's Mill, Shorterville, flour and grist mill.....			8
Joshua A. Hart, Granger, flour and grist mill.....			15
Jeffcoat Mill, Gordon, flour and grist mill.....			8
Blacksheer & Saunders, Haleburg, flour and grist mill.....			25
Cumming's Mill, Bush, flour and grist mill.....			20
Joe Baker, Hadland, flour and grist mill.....			27
Badford Grist Mill, Little Rock, flour and grist mill.....			15
Blackshe & Sanders, Haleburg, flour and grist mill.....			15
John L. Smith, Ashford, flour and grist mill.....			13
Mark Shelley, Balkum, flour and grist mill.....			6
Singleterry's Water Mill, Kinsey, lumber and timber.....			27
J. P. Williams & Co., Columbia, lumber and timber.....			25

\*From U. S. Census, 1900.

†From report of Probate Judge.

## \*JACKSON COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Moody's Flouring Mill, Kyles, flour and grist mill.....			40
George W. Brown, Kosh, flour and grist mill.....			8
J. F. Bell, Maxwell, flour and grist mill.....			4
Coffey's Mill, Scottsboro, flour and grist mill.....			8
Gross Mill, Parks Store, flour and grist mill.....			10
Hackworth's Mills, Bolivar, flour and grist mill.....			8
John S. Henegar, Rosalie, flour and grist mill.....			20
Bort Harrison, Section, flour and grist mill.....			6
W. A. Howell, Hollytree, flour and grist mill.....			5
Mathew's Grist Mill, Carns, flour and grist mill.....			10
Page's Mill, Woodville, flour and grist mill.....			6
Paint Rock Milling Co., Paint Rock, flour and grist mill.....			8
Reid & Prince, Estillfork, flour and grist mill.....			20
David H. Starkey, Kosh, flour and grist mill.....			8
Shork Mills, Hollywood, flour and grist mill.....			60
Cagle Mill, Oakley, flour and grist mill.....			12
John Thomas, Pisgah, flour and grist mill.....			20
Martin Walker, Trenton, flour and grist mill.....			40
James P. Williams, Trenton, flour and grist mill.....			20
John V. Wheeler, Pisgah, flour and grist mill.....			20
Charles W. Brown, Glenzaida, lumber and timber mill.....			25
J. N. Gonce, Anderson, lumber and timber mill.....			20
Melton Morris, Daugherty, lumber and timber mill.....			12
David M. Starkey, Kosh, lumber and timber mill.....			20
Tomon Shingle Mill, Culver, lumber and timber mill.....			10

## \*JEFFERSON C6OUNTY.

J. M. Landrum, Pinson, flour and grist mill.....	20
John Lowery Mill, Gary, flour and grist mill.....	12
Hendon's Corn Mill, Trussville, flour and grist mill.....	10
Posey's Mill, Morris, flour and grist mill.....	20
James W. Raney, Ezra, flour and grist mill.....	35
William B. Rogers, Toadvine, flour and grist mill.....	32
G. W. Underwood, Argo, flour and grist mill.....	15
William J. Wedgworth, Cardiff, flour and grist mill.....	10
W. W. Woodruff, Adamsville, flour and grist mill.....	8
W. M. Self, Oneonto, flour and grist mill.....	15
William M. Phillip, Greene, flour and grist mill.....	40
Hurst & Johnson, Pinson, lumber and timber mill.....	18
James W. Raney, Ezra, woolen goods.....	35

## LAMAR COUNTY.

John H. Cantrell, Pharos, flour and grist mill.....	15
Claborn E. Carter, Detroit, flour and grist mill.....	12
Kirk's Mill, (Yellow Creek), Sizemore, flour and grist mill....	8
Mote's Mill, (Beaver Creek), Guin, flour and grist mill.....	6
John T. Moore, (Yellow Creek), Vernon, flour and grist mill..	35
* H. W. Miller, (Luxapella Creek), Millport, flour and grist mill	20
Stanford Mills, Detroit, flour and grist mills.....	12
S. B. Thomas, Arcola, flour and grist mills.....	10
Lafayette J. Hayes, Molloy, lumber and timber mill.....	15
Hiram Hollis, Vernon, lumber and timber mill.....	35
Dr. Wm. H. Kennedy, Kennedy, lumber and timber mill.....	50
S. B. Thomas, Arcola, lumber and timber mill.....	15

\*From U. S. Census, 1900.



	NAME.	POSTOFFICE.	INDUSTRY.	H. P.
	J. O. Kennedy,	Kennedy,	mill and gin .....	
	J. W. Thomas, Jr.,	(Hills Creek),	Alfred, gin, saw and grist	
	W. M. Thomas,	(Hills Creek),	Alfred, gin, saw and grist mill	
	Osborn & Hill,	(Yellow Creek),	Blowhorn, gin, saw and grist	
†	D. M. Hollis,	(Beaver Creek),	Beaverton, gin, saw and grist	
	B. G. Boman,	(Yellow Creek),	Vernon, gin, saw and grist mill	
	A. A. Mathews,	(Yellow Creek),	Arcola, gin, saw and grist mill	
	W. L. Morton,	(Yellow Creek),	Vernon, gin, saw and grist mill	
	Penning Bros.,	Baxter,	gin, saw and grist mill.....	

## \*LAUDERDALE COUNTY.

	William M. Thornton,	Rogersville,	flour an grist mill.....	20
	James A. Bevis,	Threet,	flour and grist mill.....	8
	Jessie J. Bevis,	Kendell,	flour and grist mill.....	6
	George M. Bretherick,	Hines,	flour and grist mill.....	24
	Isa B. Eastep,	Eastep,	flour and grist mill.....	8
	Ingram Brothers,	Anderson,	flour and grist mill.....	8
	Thomas D. Prulitt,	Pruitton,	flour and grist mill.....	24
	Sharpe's Mill,	Florence,	flour and grist mill.....	40
	Nancy Williams,	Lexington,	flour and grist mill.....	20
	H. N. Call,	Reserve,	flour and grist mill.....	18
	Chandler & Chittam,	Oliver,	flour and grist mill.....	20

## LAWRENCE COUNTY.

	Burrell & Casteel,	Progress,	flour and grist mill.....	10
	George's Mill,	Leighton,	flour and grist mill.....	18
	Jones' Estate,	Kinlock,	flour and grist mill.....	10
	Kerby's Mill,	Avoca,	flour and grist mill.....	16
*	Thomas Oliver,	Hatton,	flour and grist mill.....	16
	John S. Stephenson & Co.,	Kinlock,	flour and grist mill.....	27
	Wesley L. Stover,	Crow,	flour and grist mill.....	15
	Terry & Terry,	Courtland,	flour and grist mill.....	20
	Wallace Mill,	Avoca,	flour and grist mill.....	10
	W. M. Willingham,	Camp Spring,	lumber and timber mill....	1
	H. C. McClannaher,	(Town Creek),	Mount Hope, grist mill....	
	John S. Stephenson,	(Sipsey River),	Moulton, flour and grist	
	Ben F. Masterson,	(Big Nance Creek),	Moulton, grist mill....	
†	W. G. Hamilton,	(Big Nance Creek),	Pitt, grist mill.....	
	J. M. Key,	(Brushy Creek),	Pool, grist mill.....	
	W. L. Stover,	(Flint Creek),	Oakville, flour and grist mill....	
	B. A. Casteel,	(Flint Creek),	Sewickley, flour and grist mill..	

## LEE COUNTY.

	Shelton's Mill,	Opelika,	flour and grist mill.....	40
	Floyd Mill,	Opelika,	flour and grist mill.....	10
	George W. McKinnon,	Yale,	flour and grist mill.....	24
	Vaugh Mill,	Loachapoka,	flour and grist mill.....	20
*	N. G. Macon,	(Reed Creek),	Loachapoka, flour and grist mill..	30
	W. O. Moore,	Auburn,	flour and grist mill.....	40
	W. K. Meadows,	(Halawochee Cr.),	Hattie, flour and grist mill	36
	James Crosby,	Osanippa,	flour and grist mill .....	15
	Benjamin F. Stripling,	Yale,	lumber and timber.....	20
	W. W. Wright,	(Chewacla Creek),	Auburn, not in use now.....	
	W. W. Wright & Geo. P. Harrison,	Opelika,	(Saugahatchee Cr.)	
†	H. J. Spratling,	(Frazer Creek),	Opelika, grist mill.....	25
	B. F. Meadows,	(Halawochee Creek),	Opelika, grist mill.....	40

\*From U. S. Census, 1900.

†From report of Probate Judge.

## LIMESTONE COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Weatherford Bros., Elkmont, flour and grist mill.....			6
Carter's Mill, Athens, flour and grist mill.....			16
Dupree & Stepp, Mount Rozell, flour and grist mill.....			25
Haye's Grist Mill, Mooresville, flour and grist mill.....			15
T. M. Holmes, Elkmont, flour and grist mill.....			12
John M. Head, Pettusville, flour and grist mill.....			8
Nancy Haney, Legg, flour and grist mill.....			20
Edward G. Hambleton, Goodsprings, flour and grist mill.....			15
Thomas D. Hastings, Elkmont, flour and grist mill.....			5
James L. Lamar, Goodsprings, flour and grist mill.....			8
Eugene Parham, (Piney Creek), Athens, flour and grist mill..			8
* M. A. Phillips, Shoalford, flour and grist mill.....			12
Ripley's Mill, Ripley, flour and grist mill.....			15
George Vassar, Lax, flour and grist mill.....			8
Witty's Mill, (Birds Branch), Athens, flour and grist mill....			15
William J. Woodfin, Pettusville, flour and grist mill.....			15
Pioneer Mill, Mount Rozell, flour and grist mill.....			20
A. P. Andrews, Elkmont, flour and grist mill.....			8
William N. Webb, Elkriver Mills, flour and grist mill.....			12
Baker's Mills, Elkriver Mills, flour and grist mill.....			8
Allison Miller, Rowland, flour and grist mill.....			10
Grisham Bros., Elkriver Mills, lumber and timber.....			40
Grisham Bros., Elkriver Mills, carriages and wagons.....			40
L. C. Hightower, (Big Creek), Elkriver Mills, saw, flour and grist mill .....			
Wm. Bailey, (Big Creek), Quidnunc, flour and grist mill.....			
J. W. Carter, (Big Creek), O'Neal, gin, flour and grist mill....			
M. J. Witty, (Birds Branch), Athens, flour and grist mill.....			
J. C. Vaughn, (Sulphur Creek), Elkmont, gin, flour & grist mill			
R. B. Malone, (Sulphur Creek), Athens, gin, flour and grist mill			
Wm. Woodfin, (Ragsdale Creek), Elkmont, gin, flour and grist			
J. W. Carter, (Panther Creek), Carter, gin, flour and grist mill			
John Carroll, (Leslie Creek), Centerhill, gin, flour and grist mill			
Wm. Davidson, (Limestone Creek), Lax, gin, flour and grist mill			
R. M. Clem, (Piney Creek), Fairmount, gin, flour and grist mill			
Eugene Parkam, (Piney Creek), Athens, gin, flour and grist mill			
W. M. Hayes, (Limestone Cr.), Mooresville, gin, flour and grist			
W. H. Roberts, (Sugar Creek), Athens, gin, flour and grist mill			
W. H. Marbut, Goodsprings, gin, flour and grist mill.....			

## LOWNDES COUNTY.

*G. B. Holley, Lowndesboro, flour and grist mill.....	10
†W. N. Bozeman, Benton, gin and mill.....	

## \*MADISON COUNTY.

Fannie J. Ridley, Haden, flour and grist mill.....	8
D. L. Middleton Water Mill, Gurley, flour and grist mill.....	20
Delop's Mill, Dan, flour and grist mill.....	8
Hardy Keel Water Mill, Gurley, flour and grist mill.....	15
Annie M. Taylor, Hazelgreen, flour and grist mill.....	8
Bellfactory Mill, Huntsville, flour and grist mill.....	25
Key's Mill, Keysmill, flour and grist mill.....	28
William S. Russell, Madison Station, flour and grist mill.....	12
Chas. F. Rountree, Maysville, flour and grist mill.....	15
William S. Garvin, Monrovia, flour and grist mill.....	15
A. D. and W. E. Rogers, Newmarket, flour and grist mill....	60
Butler Mill Co., Poplaridge, flour and grist mill.....	30
Payne & Miller, Huntsville, flour and grist mill.....	30
Martin's Grist Mill, Huntsville, flour and grist mill.....	15
H. C. Turner, Dan, lumber and timber.....	16
Daily Mercury, Huntsville, printing and publishing.....	6

\*From U. S. Census, 1900.

†From report of Probate Judge.

## \*MACON COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
H. H. Robinson, Loachapoka, flour and grist mill.....			4
M. W. Glass, Societyhill, flour and grist mill.....			8
J. O. H. Perry, Tuskegee, flour and grist mill.....			20

## MARION COUNTY.

	The Carter Mill, Ur, flour and grist mill.....	5
	Bexar Mercantile Co., Bexar, flour and grist mill.....	8
	Eads & Fowler, Glenallen, flour and grist mill.....	12
	The Boatwright Mill, Inez, flour and grist mill.....	12
	Samuel A. & Wm. V. Read, Eldridge, flour and grist mill.....	20
	Jasper N. Green & Sons, Brilliant, flour and grist mill.....	20
	Elislu Vickery, Winfield, flour and grist mill.....	16
*	The Shirley Mill, Ur, flour and grist mill.....	10
	Jesse G. Poe, Bearcreek, flour and grist mill.....	6
	Bull, Atkins & Donaldson, Halesville, flour and grist mill....	52
	Buttahatchee Mill Co., Halesville, lumber and timber.....	52
	John Cumens, Halesville, lumber and timber .....	12
	Kelly Saw Mill, Halesville, lumber and timber.....	15
	John R. Phillips, Bearcreek, lumber and timber.....	50
	Simon W. Moss, Winfield, lumber and timber.....	36
	The Powell Mill & Wool Carder, Duffey, woolen goods.....	50
	Albert J. Hamilton, (Williams Creek), Hamilton, flour and grist	
	W. C. Gann, (Sipsey Creek), Bexar, flour and grist mill.....	
	Q. Northington, (Sipsey Creek), Hamilton, flour and grist mill	
	Crane & Riggs, (Sipsey Creek), Delhi, flour and grist mill.....	
	T. L. Shotts, (Bull Mountain Creek), Shottsville, flour and grist	
	I. J. Loyd, (Bull Mountain Creek), Bull Mountain, flour and grist	
	D. F. Ballard, (Williams Creek), Hamilton, flour and grist mill	
†	James P. Pearce, (Buttahatchee River), Pearce's Mill, flour and	
	grist mill .....	
	James P. Pearce, (New River), Texas, flour and grist mill.....	
	J. C. Carter, (Woods Creek), Elmira, flour and grist mill.....	
	James Young, (Cantrell Mill Creek), Hamilton, flour and grist	
	W. J. Wright, (Barnesville Mill Creek), Barnesville, flour and	
	grist mill .....	
	Henry Guin, Guin, flour and grist mill.....	
	Tucker Moss, (Luxapella Creek), Winfield, flour and grist mill	
	D. G. Morrow, (Woods Creek), Elmira, flour and grist mill.....	

## \*MARSHALL COUNTY.

J. M. Ellison, Preston, flour and grist mill.....	4
Mathis Mill, Albertville, flour and grist mill.....	10
James B. Powell, Columbus City, flour and grist mill.....	4
James F. Prentice, Arab, flour and grist mill.....	7
P. C. Ragsdale, Uniongrove, flour and grist mill.....	10
James P. Smith, Warrenton, flour and grist mill.....	10
Scott's Mill, Friendship, flour and grist mill.....	8
John D. Sumers, Loaz, flour and grist mill.....	15
Lahey Mill, Bartlett, flour and grist mill.....	10
George E. Whisnant & Son, Oleander, flour and grist mill.....	10
I. G. Gross, Columbus City, flour and grist mill.....	12
Walker & Fowler Mills, Friendship, flour and grist mill.....	20
William J. Copelan, Diamond, flour and grist mill.....	5
James Wm. Barclay, Woodville, flour and grist mill.....	10
The Winston Mill, Meltonsville, flour and grist mill.....	12
W. G. Smith Estate, Sidney, flour and grist mill.....	10
Jas. M. Selvage, Grant, flour and grist mill.....	4

\*From U. S. Census, 1900.

†From report of Probate Judge.

## MARENGO COUNTY

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
*Rhodes Mill,	Sweetwater,	flour and grist mill.....	12

## \*MOBILE COUNTY.

N. Q. Thompson,	Citronelle,	flour and grist mill.....	10
H. Brannan & Son,	Pierce,	lumber and timber.....	30
T. A. Hatter & Son,	Creola,	lumber and timber.....	75
Littleton Lee,	Pierce,	lumber and timber.....	60

## \*MONROE COUNTY.

J. B. Solomon,	Manistree,	flour and grist mill.....	15
James H. Simpson,	Mexia,	flour and grist mill.....	10
Benjamin Johnson,	Hollinger,	flour and grist mill.....	15
Andrew Bohanon,	Franklin,	flour and grist mill.....	15
David J. Hatter & Son,	Walt,	lumber and timber.....	60
David J. Hatter & Son,	Walt,	lumber and timber.....	20
C. C. Yarbrough,	Monroeville,	lumber and timber .....	20

## \*MONTGOMERY COUNTY.

Daniel's Mill,	Sellers,	flour and grist mill.....	25
Montgomery Cotton Mill,	Montgomery,	cotton goods .....	35

## MORGAN COUNTY.

*Sarah M. McCutcheon,	Briscoe,	flour and grist mill.....	10
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## PERRY COUNTY.

* {	Henry C. Nichols,	(Dobyns Creek), Theo,	flour and grist mill.	20
	Mary G. Wallace,	Marion,	flour and grist mill.....	4
	Hodger's Mill,	Newbern,	flour and grist mill.....	15
	W. F. Moore,	Marion,	flour and grist mill.....	4
	Downey's Saw Mill,	Greensboro,	lumber and timber .....	15
	Stevenson's Saw & Water Mills,	Newbern,	lumber and timber..	20
	Lucindy Washburn,	(Taylor Creek),	Jericho, lumber and timber	18
	W. T. Downey,	(Limestone Creek),	Folsom, grist mill.....	6
	James Wallace,	(Legroane Creek),	Jericho, grist mill.....	8
	Dr. J. B. Tucker,	(Taylors Creek),	Jericho, grist mill.....	6
	Lucindy Washburn,	(Taylors Creek),	Jericho, grist mill.....	8
	S. M. Bolling,	(Branch of Oakmulgee Cr.),	Pinetucky, grist mill	8
	C. C. Cosby,	(Oakmulgee Creek),	Perryville, grist mill.....	8
	Thomas J. Fountain,	(Little Creek),	Oakmulgee, gin, saw and grist mill .....	8
	Pann Patterson,	(Little Creek),	Oakmulgee, gin, saw and grist	8
† {	Sarah Fountain,	(Little Creek),	Oakmulgee, gin, saw and grist	8
	Thaddeus Smith,	(Little Creek),	Active, grist mill.....	8
	W. M. Elland,	(Fords Mill Creek),	Marion, grist mill.....	20
	J. F. Morton,	(Potato Patch Creek),	Levert, grist mill.....	6
	Elijah Smith,	(Beaver Creek),	Bliss, grist mill.....	6
	Noah Coker,	(Beaver Dam Creek),	Bethlehem, grist mill.....	6
	W. A. Fountain,	(Oakmulgee Creek),	Oakmulgee, rice mill....	10

## \*PICKENS COUNTY.

Richardson & Prichards,	Coalfire,	flour and grist mill.....	25
James Mullenix,	Gordo,	flour and grist mill.....	6
H. B. & A. W. Latham,	Carrollton,	flour and grist mill.....	12
Slaughter's Mill,	Raleigh,	flour and grist mill.....	16
W. A. Kerr,	Reform,	lumber and timber .....	10

\*From U. S. Census, 1900.

†From report of Probate Judge.

## \*PIKE COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
M. J. Youngblood,	Youngblood,	flour and grist mill.....	110
William F. Ingram,	Josie,	flour and grist mill.....	20
Nancy Cotton,	(Cotton's Mill),	Milo, flour and grist mill.....	12
Ely Dees & J. D. Murphee,	Pronto,	flour and grist mill.....	20
George W. King,	Gosnen,	flour and grist mill.....	30
The Lewis Mill,	Rodney,	flour and grist mill.....	24
McQuaggis Mill,	Ansley,	flour and grist mill.....	15
George F. Williams,	Tatum,	flour and grist mill.....	4
Slatting's Grist Mill,	Henderson,	flour and grist mill.....	25
P. A. Motla,	Wingard,	flour and grist mill.....	8
Bowden & Daughtry,	Tennille,	flour and grist mill.....	16
William E. Brown,	Josie,	flour and grist mill.....	10
G. B. Howard,	Goshen,	flour and grist mill.....	20

## \*RANDOLPH COUNTY.

W. W. Dobson,	Wedowee,	flour and grist mill.....	20
J. H. White & Z. N. Lipham,	Clack,	flour and grist mill.....	11
Mrs. Georgia Gibbs,	Wedowee,	flour and grist mill.....	10
Gills Mill,	Ofelia,	flour and grist mill.....	10
Eppie M. White,	Bernice,	flour and grist mill.....	5
Larkin & M. B. Taylor,	Lamar,	flour and grist mill.....	8
Joseph B. Taylor,	Roanoke,	flour and grist mill.....	24
Owins Mill,	Potash,	flour and grist mill.....	15
Rogers Mill,	Ofelia,	flour and grist mill.....	8
C. A. Prescott,	Wedowee,	flour and grist mill.....	20
H. A. Merrill,	Lamar,	flour and grist mill.....	6
Elizabeth H. Merrill,	Micaville,	flour and grist mill.....	12
J. E. McCosh & Co.,	Lime,	flour and grist mill.....	40
William S. McCarley,	Graham,	flour and grist mill.....	20
John H. Landers,	Lofly,	flour and grist mill.....	8
Edward Lavoorn,	—, —, —,	flour and grist mill.....	8
Thomas J. Lavoorn,	Hawk,	flour and grist mill.....	16
Thomas J. Lavoorn, Sr.,	Newell,	flour and grist mill.....	8
James L. & John T. Kaylor,	Kaylor,	flour and grist mill.....	60
Henry C. Jordon,	Clack,	flour and grist mill.....	6
J. B. Hammond,	Sewell,	flour and grist mill.....	8
T. M. Halaway,	Tolbut,	flour and grist mill.....	15
Robert H. Harris,	Louina,	flour and grist mill.....	15
Dock Huckaby,	Almond,	flour and grist mill.....	10
Holley's Mill,	Rock Mills,	flour and grist mill.....	30
E. C. Heaton,	Hawk,	flour and grist mill.....	10
William N. Gladney,	Roanoke,	flour and grist mill.....	12
A. B. East,	Christiana,	flour and grist mill.....	2
Adamson & Edward's Mills,	Ofelia,	flour and grist mill.....	25
Bailey Mill,	Haywood,	flour and grist mill.....	12
F. P. Parker,	Foresters Chapel,	flour and grist mill.....	10
John C. Murphy,	Gay,	flour and grist mill.....	2
E. L. Pool,	Happyland,	flour and grist mill.....	20
James M. Kitchens,	Rockdale,	flour and grist mill.....	8
James H. Wright,	Jeptha,	flour and grist mill.....	12
Adamson & Edwards,	Ofelia,	lumber and timber.....	40
William W. Brooks,	Lofly,	lumber and timber.....	15
William A. Camp,	Almond,	lumber and timber.....	10
James L. & John T. Kaylor,	Kaylor,	lumber and timber.....	20
H. H. Stephens,	Pencil,	lumber and timber.....	20
Samuel H. Striplin,	Roanoke,	leather, tanned, curried & finished	6
Wehadkee Cotton Mills,	Rock Mills,	cotton goods.....	108

\*From U. S. Census, 1900.

## RUSSELL COUNTY.

	NAME.	POSTOFFICE.	INDUSTRY.	H. P.
*	{ Davis' Mill, Crawford, flour and grist mill.....			20
	{ H. R. Dudley, Seale, lumber and timber.....			40
	†E. M. Anderson, (Watermelon Cr.), Seale, grist mill and gin			20

## \*SHELBY COUNTY.

W. C. Denson, Pelham, flour and grist mill.....	12
William H. Shrader, Shelby, flour and grist mill.....	20
William H. Pledger, Pelham, flour and grist mill.....	40
Hendrick & Alverson, Vincent, flour and grist mill.....	40
David A. Whitfield, Vandiver, flour and grist mill.....	10
Brownings Mill, Columbiana, lumber and timber.....	30

## \*ST. CLAIR COUNTY.

The Yarbrough Mill, Ashville, flour and grist mill.....	8
Hare's Mill, Ashville, flour and grist mill.....	8
John R. Dyke, Wolfcreek, flour and grist mill.....	30
Perry E. Wyatt, Coal City, flour and grist mill.....	10
Henry A. Palmer, Partlow, flour and grist mill.....	10
J. M. McLaughlin, Springville, flour and grist mill.....	25
The Machen Mill, Partlow, flour and grist mill.....	10
The Lindsey Mill, Ashville, flour and grist mill.....	10
Hill & Foreman, Springville, flour and grist mill.....	28
Henderson's Mill, Ragland, flour and grist mill.....	5
Helm & Truss, Helms, flour and grist mill.....	20
Grout's Mills, Wolfcreek, flour and grist mill.....	10
The Gilchrist Mill, Ashville, flour and grist mill.....	5
The Cox Mill, Ashville, flour and grist mill.....	10
Rufus W. Beason, Whitney, flour and grist mill.....	11
Rock Bridge Mill, Gallant, lumber and timber mill.....	20

## \*SUMTER COUNTY.

E. B. Hearn, (Kinterbish Creek), Gaston, .....	40
R. H. Stephens, (Kinterbish Creek), Alamuchee.....	20
R. D. Simmons, (Toomsooba Creek), Bell's Station.....	30
R. W. Shaw, Cuba .....	10
W. H. Walker, (Silver Creek), Alamuchee.....	20
J. U. Gillespie, (Coatopa Creek), Coatopa .....	10

## TALLADEGA COUNTY.

{ Jefferson Roberson, Fayetteville, flour and grist mill.....	10
{ J. C. Brock, Eastaboga, flour and grist mill.....	12
{ Riser & Bro., Talladega, flour and grist mill.....	40
{ Shock E. Jemison, Sunnyside, flour and grist mill.....	15
{ Vincent Mill, Talladega, flour and grist mill.....	25
{ O. F. Luttrell, Talladega, flour and grist mill.....	40
* { Riddle Mills, Waldo, flour and grist mill.....	16
{ J. F. Smith, Eastaboga, flour and grist mill.....	40
{ John W. Thweatt, McFall, flour and grist mill.....	12
{ J. B. Turner, McFall, flour and grist mill.....	15
{ Allison's Mill, Talladega, flour and grist mill.....	60
{ J. F. Smith, Eastaboga, lumber and timber.....	40
{ Cragdale Mill, Talladega, lumber and timber.....	40
{ J. B. Turner, McFall, lumber and timber.....	20

\*From U. S. Census, 1900.

†From report of Probate Judge.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Priebe's Mill, (Choccolocco Creek), Jenifer, grist mill.....			200
J. F. Smith's Mill, (Choccolocco Creek), Oxford, grist mill.....			225
B. Schmidt's Mill, (Choccolocco Creek), Lincoln, grist mill.....			200
Craig's Mill, (Choccolocco Creek), Oxford, grist mill.....			150
Wilson's Mill, (Choccolocco Creek), Jenifer, grist mill.....			150
Eureka Mills, (Choccolocco Creek), Eureka, grist mill.....			150
Turner's Mill, (Chehawhaw Creek), McFall, grist mill.....			150
Kants Mill, (Talladega Creek), Chandler Springs, grist mill..			50
Riddle's Mill, (Talladega Creek), Waldo, grist mill.....			75
Taylor's Mill, (Talladega Creek), Talladega, grist mill.....			150
† Reynold's Mill, (Talladega Creek), Nottingham, grist mill....			150
Allison's Mill, (Talladega Creek), Talladega, grist mill.....			75
Duncan's Mill, (Talladega Creek), Alpine, grist mill.....			75
Baker's Mill, (Talladega Creek), Kymulga, grist mill.....			100
Vincent's Mill, (Crooked Creek), Sylacauga, grist mill.....			50
Oden's Mill, (Short Creek), Sylacauga, grist mill.....			75
Jemison's Mill, (Kelley's Creek), Sunnyside, grist mill.....			50
Camp & Sons' Mill, (Salt Creek), Hopeful, grist mill.....			50
Robinson's Mill, (Cedar Creek), Fayetteville, grist mill.....			50
Lackey's Mill, (Horse Creek), Ironaton, grist mill .....			25
Talladega Company, (Choccolocco Creek), Talladega, organized for electric transmission .....			

## \*TALLAPOOSA COUNTY.

George Stewart, Thaddeus, flour and grist mill.....	12
John W. Britt, Jacksons Gap, flour and grist mill.....	20
Benjamin F. Jarvis, Yates, flour and grist mill.....	12
T. J. Hamlet, Hamlet, flour and grist mill.....	15
T. W. Whitman, Dadeville, flour and grist mill.....	20
Sanford Milling & Mfr. Co., Dadeville, flour and grist mill...	25
John W. Hay, Camphill, flour and grist mill.....	15
Hammond's Mill, Dadeville, flour and grist mill.....	20
Hodnett Grist & Flour Mill, Acme, flour and grist mill.....	16
Thomas L. Bulger, Dadeville, flour and grist mill.....	15
Vines Mills, Easton, flour and grist mill.....	40
A. T. & H. C. Vickers, Newsite, flour and grist mill.....	20
J. C. Street, Anniston, flour and grist mill.....	25
Shepherd Bros. & Co., Tohopeka, flour and grist mill.....	10
G. W. Stewart, Thaddeus, flour and grist mill.....	25
Albert J. Hollaway, Alexander City, flour and grist mill.....	20
Mrs. Millner, Mary, flour and grist mill.....	25
Jno. L. Patterson, Hackneyville, flour and grist mill.....	12
Thomas B. Griffin, Matilda, flour and grist mill.....	10
Daviston Mill, Daviston, flour and grist mill.....	8
Lamberth & Dewberry, Logpit, flour and grist mill.....	20
Silver Shoals Mill, Buttston, flour and grist mill.....	80
M. R. Hays & Bro., Notasulga, flour and grist mill.....	40
Farrows Flour & Grist Mill, Susanna, flour and grist mill.....	60
J. H. Yarbrough, Hackneyville, flour and grist mill.....	12
T. F. Garnett, Tallassee, lumber and timber.....	20
G. W. Stewart, Thaddeus, lumber and timber.....	20

\*From U. S. Census, 1900.

†From report of Probate Judge.

## \*TUSCALOOSA COUNTY.

NAME.	POSTOFFICE.	INDUSTRY.	H. P.
Price's Mill, Binion, flour and grist mill.....			8
Keene's Mill, Cottondale, flour and grist mill.....			20
B. E. Thompson, Cottondale, flour and grist mill.....			15
Wm. D. Shadix, (Sandy Creek), Double Springs, flour and grist			4
J. W. Spencer, Elrod, flour and grist mill.....			10
Webb's Mill, Elrod, flour and grist mill.....			20
Hagler's Mill, Falls, flour and grist mill.....			40
Patton's Mill, Fosters, flour and grist mill.....			12
David M. Montgomery, Moores Bridge, flour and grist mill....			10
Looney John Mills, New Lexington, flour and grist mill.....			6
Elitson's Mill, New Lexington, flour and grist mill.....			12
Alfred Gilliland, Newtonville, flour and grist mill.....			12
Andrew J. Hewett, Skelton, flour and grist mill.....			5
Tierce's Mill, Tierce, flour and grist mill.....			10
James M. Yerby, Tuscaloosa, flour and grist mill.....			20
O. W. Glenn, Tyner, flour and grist mill.....			8
The Rope & Yarn Mills, (Binion's Creek), Samantha, cordage and twine .....			60

## \*WALKER COUNTY.

Boldo Grist Mill, Boido, flour and grist mill.....	40
James B. Wakefield, Prospect, flour and grist mill.....	10
Lewis W. Odum, Oakman, flour and grist mill.....	10
Mahala E. & Dalton Odum, Parrish, flour and grist mill.....	10
Joseph Z. Norris, Galloway, flour and grist mill.....	5
Thomas J. King, Oakman, flour and grist mill.....	10
Lewis Guthrie, Pocahontas, flour and grist mill.....	10
Wm. Cobb, Oakman, flour and grist mill.....	10
Peter McGough, Carbonhill, lumber and timber.....	10

## \*WASHINGTON COUNTY.

Mrs. Samuel Wilkins, Healing Springs, flour and grist mill.....	6
Consey's Mill, Healing Springs, lumber and timber mill.....	10

## WILCOX COUNTY.

* { Ward & Grimes, (Pine Barren Creek), Pineapple, flour and grist	33
* { George A. Barge, Snowhill, flour and grist mill.....	15
* { J. W. Cooper, Candler, flour and grist mill.....	16
* { George Barge, (Pine Barren Creek), Furman, grist mill and gin	
* { Glover & Carter, (Pine Barren Cr.), Pineapple, grist mill and gin	
* { S. McCormick, (Pine Barren Cr.), Pineapple, grist mill and gin	
* { D. McIntosh, (Pursley Creek), Camden, grist mill and gin....	

\*From U. S. Census, 1900.

†From report of Probate Judge.



## WINSTON COUNTY.

NAME	POSTOFFICE.	INDUSTRY.	H. P.
( Richard H. Blake,	Houston,	flour and grist mill.....	8
Thomas O. Partridge,	Elk,	flour and grist mill.....	10
Wm. D. Shadix, (Sandy Creek),	Double Springs,	flour and grist	4
George D. Wilson,	Haleysville,	flour and grist mill.....	8
Manna A. Posey,	Motes,	flour and grist mill.....	10
Martin A. & Martha Peak,	Peaks Mill,	flour and grist mill.....	10
Milligan Mill,	Double Springs,	flour and grist mill.....	10
James Cantrell,	Addison,	flour and grist mill.....	4
Burks Mill,	Cranal,	flour and grist mill.....	10
( Nauvoo Mill, (Black Water Creek),	Nauvoo,	grist mill and gin	
Anderson Ward Mill, (Clear Creek),	Haleysville,	flour and grist	
J. Calvin Cagle, (Clear Creek),	Double Springs,	saw, flour and grist mill and gin .....	
Jonathan Barton Mill, (Clear Creek),	Deer,	grist mill .....	
Hadder Mill, (Clear Creek),	Double Springs,	grist mill.....	
Posey Mill, (Clear Creek),	Motes,	grist mill, saw and gin.....	
S. D. Spain, (Clear Creek),	Malta,	grist mill, saw and gin.....	
Gus Posey Mill, (Clear Creek),	Elk,	grist mill, saw and gin....	
† Wm. Dodd, (Splunge Creek),	Natural Bridge,	grist mill, saw and gin .....	
Kelley Mill, (Black Water Creek),	Lynn,	grist mill, saw and gin	
Peaks Mill, (Grindstone Creek),	Peaks Mill,	grist mill, saw and gin .....	
Jack Curtis, (Sandy Creek),	Double Springs,	grist mill, saw and gin .....	
Manley Payne, (Beech Creek),	Gumpond,	grist mill, saw and gin	
( Christian Mill, (Christian Creek),	Peaks Mill,	grist mill, saw	

\*From U. S. Census, 1900.

†From report of Probate Judge.

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**GEOLOGICAL SURVEY OF ALABAMA**

EUGENE ALLEN SMITH, State Geologist

**BULLETIN No. 8**

**THE MATERIALS AND MANUFACTURE  
OF PORTLAND CEMENT**

BY

EDWIN C. ECKEL

**THE CEMENT RESOURCES of ALABAMA**

BY

EUGENE A. SMITH



# GEOLOGICAL SURVEY OF ALABAMA

EUGENE ALLEN SMITH, STATE GEOLOGIST.

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## BULLETIN No. 8.

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### **The Materials and Manufacture of Portland Cement.**

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EDWIN C. ECKEL.

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---

MONTGOMERY, ALABAMA  
THE BROWN PRINTING COMPANY, PRINTERS AND BINDERS  
1904





*To His Excellency,*     Gov. R. M. CUNNINGHAM :

Sir: I have the honor to submit herewith Bulletin No. 8, on the Cement Resources of Alabama; with Preliminary Chapters on the Materials and Manufacture of Portland Cement, by Mr. Edwin C. Eckel, of the United States Geological Survey.

That part of the Report relating specially to the Alabama occurrences was prepared by the writer in coöperation with the United States Geological Survey, and in slightly different form, has been published in Bulletin No. 225, of that Survey. The chapters by Mr. Eckel, which add so much to the value and completeness of the Bulletin, have been generously contributed by him.

Our indebtedness to Senator John T. Morgan is particularly great, since the investigations on which this report is based, were undertaken mainly at his instance, and the coöperation above mentioned, secured through his influence.

Very respectfully,

EUGENE A. SMITH,  
*State Geologist.*

University of Alabama,  
July 1, 1904.

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GEOLOGICAL SURVEY OF ALABAMA  
EUGENE ALLEN SMITH, STATE GEOLOGIST

# A GEOLOGICAL MAP OF ALABAMA

BY  
EUGENE ALLEN SMITH

1904

LEGEND

GEOLOGICAL FORMATIONS

POST EOCENE

Sands, Clays, and Pebbles Earth.

EOCENE

Marble, Building Stone and Cement Rock.

St. Stephens Limestone

Shell Marls.

Calabome and Buhrstone

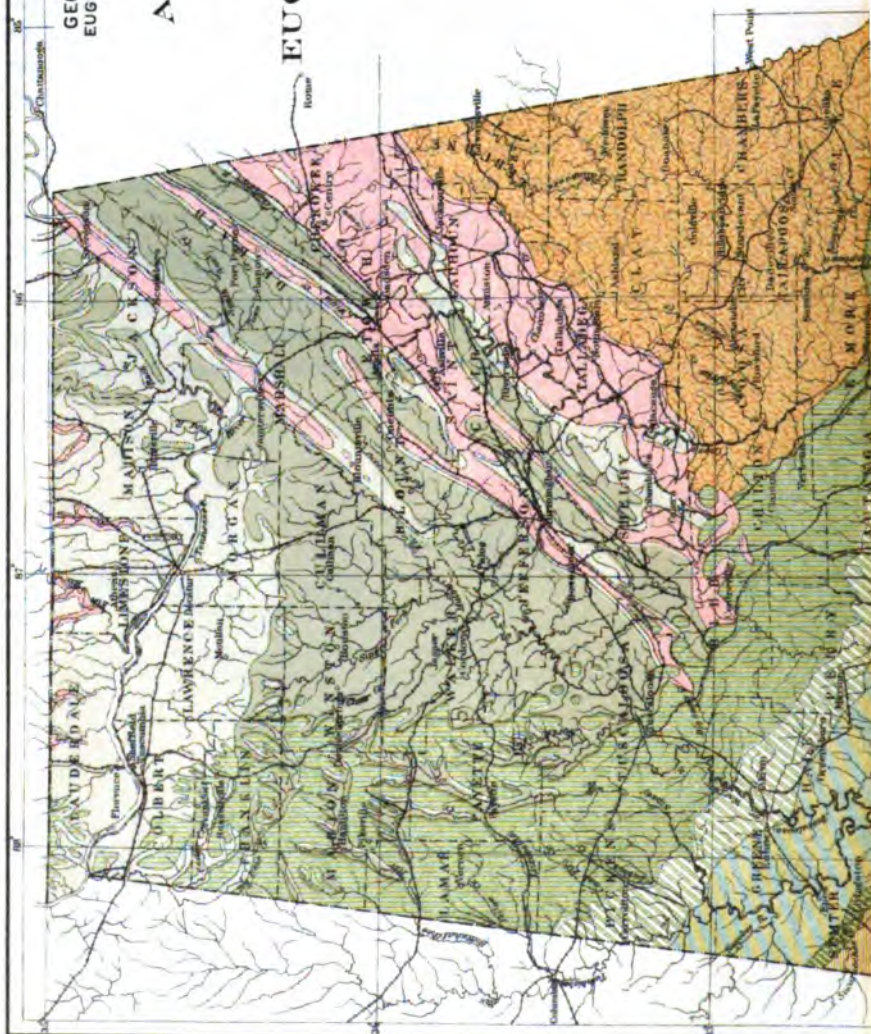
Lignitic and Midway

Lignitic and Midway

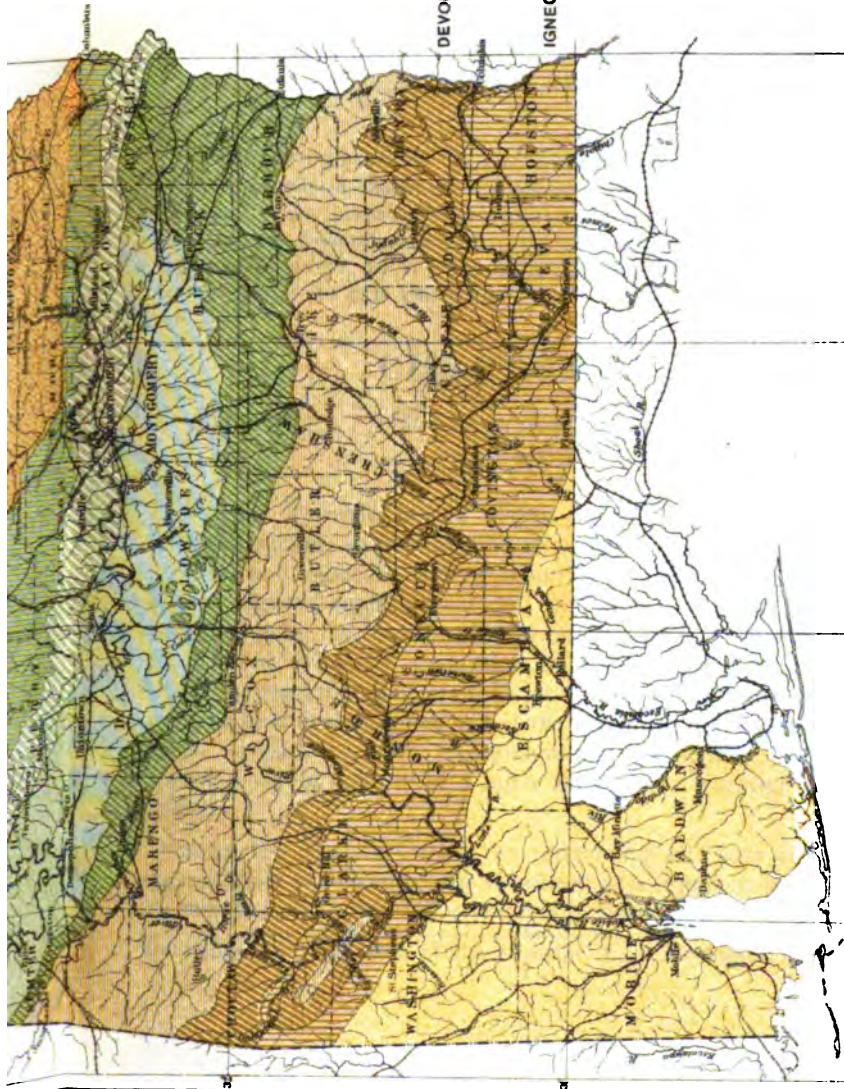
CRETACEOUS

Ripley and Blue Marl

Ripley and Blue Marl







# CRETACEOUS

Ripley and Blue Marl

☐ *Artisan Water Sands.*  
 Selma Chalk

☐ *Artisan Water Sands.*  
 Eufaw

☐ *Artisan Water Sands.*  
 Tualoosa

*Putney Clays, Ochers,*  
*Artisan Water Sands.*

## CARBONIFEROUS

☐ *Coal and Shales.*  
 Coal Measures

☐ *Limestones for lime-burning, Furnace Flues, and Cement.*  
 Lower Carboniferous

## DEVONIAN, SILURIAN AND CAMBRIAN

☐ *Iron Ores, Bauxite, and Limestones for lime-burning, Furnace Flues, and Cement.*

## IGNEOUS AND METAMORPHIC ROCKS

☐ *Gold, Copper Ore, Pyrite, Mica, Graphite.*

GULF OF MEXICO



# PART I.

---

## THE MATERIALS AND MANUFACTURE OF PORTLAND CEMENT.\*

---

BY EDWIN C. ECKEL.

---

[The following paper on the raw materials and methods of manufacture of Portland Cement has been prepared as the result of field work and other investigations carried out by the writer for the United States Geological Survey. Certain sections of the contribution have appeared, in slightly different form, in *Municipal Engineering* during the past two years.]

## CHAPTER 1.

### THE RELATION OF PORTLAND TO OTHER CEMENTS.

It seems desirable, before taking up the specific subject of Portland cement, to indicate the relationships existing between Portland and other cementing materials. These relationships, both as regards resemblances and differences, seem to be best brought out by the classification presented below. This grouping is based primarily upon the amount of chemical change caused by the process of manufacture and use; and secondarily upon the chemical composition of the cement after setting. As regard is paid to both technologic and commercial considerations, it would seem to be a fairly satisfactory working classification.

GROUP I.—SIMPLE CEMENTS: Including all those cementing materials produced by the expulsion of a liquid or gas from the raw material; and whose setting properties are due to the

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simple reabsorption of the same liquid or gas and the reassumption of original composition; the set cement being therefore similar in composition to the raw material.

*Sub-group Ia. Hydrate Cements:* Setting properties due to reabsorption of water.

*Sub-group Ib. Carbonate Cements:* Setting properties due to reabsorption of carbon dioxide.

**GROUP II.—COMPLEX CEMENTS:** Including all those cementing materials whose setting properties are due to the action of entirely new chemical compounds which were formed during manufacture or use; the set cement being therefore different in composition from the raw material.

*Sub-group Ia. Silicate Cements:* Setting properties due largely to the formation of silicates.

*Sub-group Ib. Oxychloride Cements:* Setting properties due to the formation of oxychlorides.

#### GROUP I—SIMPLE CEMENTS.

The cementing materials included in the present group are those known commercially as "plasters," "hard-finishing cements," and "limes."

The material from which the "plasters" and "hard-finishing cements" are derived is gypsum, a hydrous calcium sulphate; while the limes are derived from limestone, which is essentially calcium carbonate, though usually accompanied by greater or less amounts of magnesium carbonate.

On heating gypsum to a certain temperature, the raw material parts readily with much of its water, leaving an almost anhydrous calcium sulphate, known commercially as plaster-of-Paris. On exposing this plaster to water, it re-hydrates, and again takes the composition of the gypsum from which it was derived.

In like manner limestone, on being sufficiently heated, gives off its carbon dioxide, leaving calcium oxide or "quicklime." This, on exposure to moisture and air carrying carbon dioxide, reabsorbs carbon dioxide and reassumes its original composition, calcium carbonate.

The cementing materials included in this group, therefore,

while differing in composition and properties, agree in certain important points. They are all manufactured by heating a natural raw material sufficiently to remove much or all of its water or carbon dioxide; and, in all, the setting properties of the cementing material are due to the fact that, on exposure to the water or carbon dioxide which has thus been driven off, the cement reabsorbs the previously expelled liquid or gas, and re-assumes the chemical composition of the raw material from which it was derived.

Plaster-of-Paris, after setting, is not chemically different from the gypsum from which it was derived; while if the sand, added to avoid shrinkage, be disregarded, hardened lime mortar is nothing more or less than an artificial limestone.

#### *Sub-group Ia. Hydrate Cements.*

The materials here included are known in commerce as "plaster-of-Paris," "cement plaster," "Keene's cement," "Parian cement," etc. All of these hydrate cements are based upon one raw material,—gypsum. The partial dehydration of pure gypsum produces plaster-of-Paris. By the addition of gypsum, either by nature or during manufacture, of relatively small amounts of other materials; or by slight variations in the processes of manufacture, the time of setting, hardness, and other important technical properties of the resulting plaster can be changed to a sufficient degree to warrant separate naming and descriptions of the products.

Both the technology and the chemistry of the processes involved in the manufacture of the hydrate cements are simple. The mineral gypsum, when pure, is a hydrous sulphate of lime, of the formula  $\text{CaSO}_4 \cdot 2\text{H}_2\text{O}$ , corresponding to the composition calcium sulphate 79.1%, water 20.9%. Gypsum, as mined, rarely even approximates to this ideal composition, its impurities often amounting to 25% or even more. These impurities, chiefly clayey materials and fragments of quartz and limestone, often exercise an appreciable effect upon the properties of the plaster resulting from burning such impure gypsum.

On burning pure gypsum at a relatively low temperature (350°-400° F.) much of its water of combination is driven off, leaving a partially dehydrated lime sulphate. This, when ground, is plaster of Paris, or if it either naturally or artificial-

ly contains certain impurities, it is called "cement plaster." When either plaster of Paris or cement plaster are mixed with water, the percentage of water which was driven off during calcination is reabsorbed, and the mixture hardens, having again becomes a hydrous sulphate of lime. The processes involved in the manufacture and setting of the dead-burned plasters and hard-finish plasters are slightly more complicated, but the reactions involved are of the same general type.

#### *Sub-group Ib. Carbonate Cements.*

The cementing materials falling in the present sub-group are oxides, derived from natural carbonates by the application of heat. On exposure, under proper conditions, to any source of carbon dioxide, the cementing material recarbonates and "sets." In practice the carbon dioxide required for setting is obtained simply by exposure of the mortar to the air. In consequence the set of these carbonate cements, as commonly used, is very slow (owing to the small amount of carbon dioxide which can be taken up from ordinary air); and, what is more important from an engineering point of view, none of the mortar in the interior of a wall ever acquires hardness, as only the exposed portions have an opportunity to absorb carbon dioxide. From the examination of old mortars it has been thought probable that a certain amount of chemical action takes place between the sand and the lime, resulting in the formation of lime silicates; but this effect is slight and of little engineering importance compared with the hardening which occurs in consequence of the reabsorption of carbon dioxide from the air.

Limestone is the natural raw material whose calcination furnishes the cementing materials of this group. If the limestone be an almost pure calcium carbonate it will, on calcination, yield calcium oxide or "quicklime." If, however, the limestone should contain any appreciable percentage of magnesium carbonate, the product will be a mixture of the oxides of calcium and magnesium commercially known as magnesian lime. A brief sketch of the mineralogic relationships of the various kinds of limestone, in connection with the chemistry of lime-burning, will be of service at this point of the discussion.

Pure limestone has the composition of the mineral calcite, whose formula is  $\text{CaCO}_3$ , corresponding to the composition

calcium oxide 56%, carbon dioxide, 44%. In the magnesian limestones part of this calcium carbonate is replaced by magnesium carbonate, the resulting rock therefore having a formula of the type  $X \text{ CaCO}_3, Y \text{ MgCO}_3$ . This replacement may reach the point at which the rock has the composition of the mineral dolomite—an equal mixture of the two carbonates, with the formula  $\text{CaCO}_3, \text{MgCO}_3$ , corresponding to the composition calcium oxide 30.43%, magnesium oxide, 21.74%, carbon dioxide, 47.83%. Limestones may therefore occur with any intermediate amount of magnesium carbonate, and the lime which they produce on calcination will carry corresponding percentages of magnesium oxide, from 0% to 21.74%. Commercially those limes which carry less than 10% of magnesium oxide are, for building purposes, marketable as "pure limes"; while those carrying more than that percentage will show sufficiently different properties to necessitate being marketed as "magnesian limes."

Aside from the question of magnesia, a limestone may contain a greater or lesser amount of impurities. Of these the most important are silica ( $\text{SiO}_2$ ), alumina ( $\text{Al}_2\text{O}_3$ ), and iron oxide ( $\text{Fe}_2\text{O}_3$ ). These impurities, if present in sufficient quantity, will materially affect the properties of the lime produced, as will be noted under the heads of Hydraulic Limes and Natural Cements.

The Carbonate Cements may be divided into two classes:—

- (1) High calcium limes;
- (2) Magnesian limes.

*High Calcium Limes.*—On heating a relatively pure carbonate of lime to a sufficiently high degree, its carbon dioxide is driven off, leaving calcium oxide ( $\text{CaO}$ ) or "quicklime." Under ordinary conditions, the expulsion of the carbon dioxide is not perfectly effected until a temperature of  $925^\circ \text{C}$ . is reached. The process is greatly facilitated by blowing air through the kiln, or by the injection of steam. On treating quicklime with water, "slacking" occurs, heat being given off, and the hydrated calcium oxide ( $\text{CaH}_2\text{O}_2$ ) being formed. The hydrated oxide will, upon exposure to the atmosphere, slowly reabsorb sufficient carbon dioxide to reassume its original composition as lime carbonate. As this reabsorption can take place

only at points where the mortar is exposed to the air, the material in the middle of thick walls never becomes recarbonated. In order to counteract the shrinkage which would otherwise take place during the drying of the mortar, sand is invariably added in the preparation of lime mortars, and as noted above, it is probable that certain reactions take place between the lime and the sand. Such reactions, however, though possibly contributing somewhat to the hardness of old mortars, are only incidental and subsidiary to the principal cause of setting,—recarbonation. The presence of impurities in the original limestone affects the character and value of the lime produced. Of these impurities, the presence of silica and alumina in sufficient quantities will give hydraulic properties to the resulting limes; such materials will be discussed in the next group as Hydraulic Limes and Natural Cements.

*Magnesian Limes.*—The presence of any considerable amount of magnesium carbonate in the limestone from which a lime is obtained has a noticeable effect upon the character of the product. If burned at the temperature usual for a pure limestone, magnesian limestones give a lime which slakes slowly without evolving much heat, expands less in slaking, and sets more rapidly than pure lime. To this class belongs the well known and much used limes of Canaan (Conn.); Tuckahoe, Pleasantville and Ossining, (N. Y.); various localities in New Jersey and Ohio; Cedar Hollow (Penn.), and Chewacla (Ala.) Under certain conditions of burning, pure magnesian limestone yields hydraulic products, but in this case, as in the case of the product obtained by burning pure magnesite, the set seems to be due to the formation of a hydroxide rather than of a carbonate. Magnesian limestones carrying sufficient silica and alumina will give, on burning, a hydraulic cement falling in the next group under the head of Natural Cements.

#### GROUP II—COMPLEX CEMENTS.

The cementing materials grouped here as Silicate or Hydraulic Cements, include all those materials whose setting properties are due to the formation of new compounds, during manufacture or use, and not to the mere reassumption of the original composition of the material from which the cement was made. These new compounds may be formed either by chemical change



during manufacture or by chemical interaction, in use, of materials which have merely been mechanically mixed during manufacture.

In the class of silicate cements are included all the materials commonly known as cements by the engineer (natural cements, Portland cement, pozzuolanic cements), together with the hydraulic limes.

Though differing widely in raw material, methods of manufacture and properties, the silicate cements agree in two prominent features: they are all hydraulic (though in very different degrees); and this property of hydraulicity is, in all, due largely or entirely to the formation of tri-calcic silicate ( $3 \text{ CaO SiO}_2$ ). Other silicates of lime, as well as silico-aluminates, may also be formed; but they are relatively unimportant, except in certain of the natural cements and hydraulic limes where the lime-aluminates may be of greater importance than is here indicated. This will be recurring to in discussing the groups named.

The silicate cements are divisible, on technologic grounds, into four distinct classes. The basis for this division is given below. It will be seen that the first named of these classes (the pozzuolanic cements) differs from the other three very markedly inasmuch as its raw materials are not calcined after mixture; while in the last three classes the raw materials are invariably calcined after mixture. The four classes differ somewhat in composition, but more markedly in methods of manufacture and in the properties of the finished cements.

### *Classes of Silicate Cements.*

1. *Pozzuolanic\* Cements*: Produced by the mechanical mixture, without calcination, of slaked lime and a silico-aluminous material (the latter being usually a volcanic ash or blast-furnace slag.)

2. *Hydraulic Limes*: Produced by the calcination, at a temperature not much higher than that of decarbonation, of a siliceous limestone so high in lime carbonate that a considerable amount of free lime appears in the finished product.

---

\*Also written Puzzolan.

3. *Natural Cements*: Produced by the calcination, at a temperature between those of decarbonation and clinkering, of a siliceous limestone (which may also carry notable amounts of alumina and of magnesium carbonate) in which the lime carbonate is so low, relatively to the silica and alumina, that little or no free lime appears in the cement.

4. *Portland Cements*: Produced by the calcination, at the temperature of semi-vitrefaction ("clinkering") of an artificial mixture of calcareous with silico-aluminous materials, in the proportion of about three parts of lime carbonate to one part of clayey material.

### NATURAL CEMENTS.

Natural cements are produced by burning a naturally impure limestone, containing from 15 to 40 per cent. of silica, alumina, and iron oxide. This burning takes place at a comparatively low temperature, about that of ordinary lime burning. The operation can therefore be carried on in a kiln closely resembling an ordinary lime kiln. During the burning the carbon dioxide of the limestone is almost entirely driven off, and the lime combines with the silica, alumina, and iron oxide, forming a mass containing silicates, aluminates, and ferrites of lime. In case the original limestone contained much magnesium carbonate, the burned rock will also contain a corresponding amount of magnesia and magnesian compounds.

After burning, the burned mass will not slack if water be added. It is necessary, therefore, to grind it quite finely. After grinding, if the resulting powder (-natural cement) be mixed with water it will harden rapidly. This hardening or setting will also take place under water. The natural cements differ from ordinary limes in two noticeable ways:

- (1) The burned mass does not slack on the addition of water.
- (2) After grinding, the powder has hydraulic properties, i. e., if properly prepared, it will set under water.

Natural cements are quite closely related to both hydraulic limes on the one hand, and Portland cement on the other, agreeing with both in the possession of hydraulic properties. They differ from hydraulic limes, however, in that the burned natural cement rock will not slake when water is poured on it.

The natural cements differ from Portland cements in the following important particulars:

(1) Natural cements are not made by burning carefully prepared and finely ground artificial mixtures, but by burning masses of natural rock.

(2) Natural cements, after burning and grinding, are usually yellow to brown in color and light in weight, their specific gravity being about 2.7 to 2.9; while Portland cement is commonly blue to gray in color and heavier, its specific gravity ranging from 3.0 to 3.2.

(3) Natural cements are always burned at a lower temperature than Portland, and commonly at a *much* lower temperature, the mass of rock in the kiln never being heated high enough to even approach the fusing or clinkering point.

(4) In use, natural cements set more rapidly than Portland cement, but do not attain such a high ultimate strength.

(5) In composition, while Portland cement is a definite product whose percentages of lime, silica, alumina and iron oxide vary only between narrow limits, various brands of natural cements will show very great differences in composition.

The material utilized for natural cement manufacture is invariably a clayey limestone, carrying from 13 to 35 per cent. of clayey material, of which 10 to 22 per cent. or so is silica, while alumina and iron oxide together may vary from 4 to 16 per cent. It is the presence of these clayey materials which give the resulting cement its hydraulic properties. Stress is often carelessly or ignorantly laid on the fact that many of our best known natural cements carry large percentages of magnesia, but it should, at this date, be realized that magnesia (*in natural cements at least*) may be regarded as being almost exactly interchangeable with lime, so far as the hydraulic properties of the product are concerned. The presence of magnesium carbonate in a natural cement rock is then merely incidental, while the silica, alumina and iron oxide are essential. The 30 per cent. or so of magnesium carbonate which occurs in the cement rock of the Rosendale District, N. Y., could be replaced by an equal amount of lime carbonate, and the burnt stone would still give a hydraulic product. If, however, the clayey portion (silica, alumina, and iron oxide) of the Rosendale rock could be removed, leaving only the magnesium and lime carbonates, the

burnt rock would lose all of its hydraulic properties and would yield simply a magnesian lime.

This point has been emphasized because many writers on the subject have either explicitly stated or implied that it is the magnesian carbonate of the Rosendale, Akron, Louisville, Utica, and Milwaukee rocks that causes them to yield a natural cement on burning.

### *PORTLAND CEMENT.*

Portland cement is produced by burning a finely ground artificial mixture containing essentially lime, silica, alumina, and iron oxide, in certain definite proportions. Usually this combination is made by mixing limestone or marl with clay or shale, in which case about three times as much of the lime carbonate should be present in the mixture as of the clayey materials. The burning takes place at a high temperature, approaching 3,000° F., and must, therefore, be carried on in kilns of special design and lining. During the burning, combination of the lime with silica, alumina, and iron oxide takes place. The product of the burning is a semi-fused mass called clinker, and consisting of silicates, aluminates, and ferrites of lime in certain definite proportions. This clinker must be finely ground. After such grinding the powder (—Portland cement) will set under water.

As noted above, under the head of Natural Cements, Portland cement is blue to gray in color, with a specific gravity of 3.0 to 3.2, and sets more slowly than natural cements, but soon attains a higher tensile strength.

### *PUZZOLAN CEMENTS.*

The cementing materials included under this name are made by mixing powdered slaked lime with either a volcanic ash or a blast-furnace slag. The product is therefore simply a mechanical mixture of two ingredients, as the mixture is not burned at any stage of the process. After mixing, the mixture is finely ground. The resulting powder (Puzzolan cement) will set under water.

Puzzolan cements are usually light bluish to light yellow in color, and of lower specific gravity and less tensile strength than Portland cement. They are better adapted to use under water than to use in air.

## CHAPTER 2.

### PORTLAND CEMENT: DEFINITION, COMPOSITION AND CONSTITUTION.

In the following section various possible raw materials for Portland cement manufacture will be taken up, and their relative suitability for such use will be discussed. In order that the statements there made may be clearly understood, it will be necessary to preface this discussion by a brief explanation regarding the composition and constitution of Portland cement.

*Use of term Portland.*—While there is a general agreement of opinion as to what is understood by the term Portland cement, a few points of importance are still open questions. The definitions of the term given in specifications are in consequence often vague and unsatisfactory.

It is agreed that the cement mixture must consist essentially of lime, silica, and alumina in proportions which can vary but slightly; and that this mixture must be burned at a temperature which will give a semi-fused product—a “clinker.” These points must therefore be included in any satisfactory definition. The point regarding which there is a difference of opinion is whether or not cements made by burning a natural rock can be considered true Portlands. The question as to whether the definition of Portland cement should be drawn so as to include or exclude such products is evidently largely a matter of convention; but, unlike most conventional issues, the decision has very important practical consequences. The question at issue may be stated as follows:

If we make artificial mixture of the raw materials and a very high degree of burning the criteria on which to base our definition, we must in consequence of that decision exclude from the class of Portland cements certain well known products, manufactured at several points in France and Belgium by burning a natural rock, without artificial mixture, and at a considerably lower temperature than is attained in ordinary Portland cement practice. These “natural Portlands” of France and Belgium

have always been considered Portland cements by the most critical authorities, though all agree that they are not particularly *high grade* Portlands. So that a definition, based upon the criteria above named, will of necessity exclude from our class of Portland cements some very meritorious products.

There is no doubt that in theory a rock could occur, containing lime, silica, and alumina in such correct proportions as to give a good Portland cement on burning. Actually, however, such a perfect cement rock is of extremely rare occurrence. As above stated, certain brands of French and Belgian "Portland" cements are made from such natural rocks, without the addition of any other material; but these brands are not particularly high grade, and in the better Belgian cements the composition is corrected by the addition of other materials to the cement rock, before burning.

The following definition of Portland cement is of importance because of the large amount of cement which will be accepted annually under the specifications\* in which it occurs. It is also of interest as being the nearest approach to an official government definition of the material that we have in this country:

"By a Portland cement is meant the product obtained from the heating or calcining up to incipient fusion of intimate mixtures, either natural or artificial, of argillaceous with calcareous substances, the calcined product to contain at least 1.7 times as much of lime, by weight, as of the materials which give the lime its hydraulic properties, and to be finely pulverized after said calcination, and thereafter additions or substitutions for the purpose only of regulating certain properties of technical importance to be allowable to not exceeding 2 per cent. of the calcined product."

It will be noted that this definition does not require pulverizing or artificial mixing of the materials prior to burning. It seems probable that the Belgian "natural Portlands" were kept in mind when these requirements were omitted. In dealing with American made cements, however,—and the specifications are headed "Specifications for American Portland Cement,"—it is a serious error to omit these requirements. No true Portland cements are at present manufactured in America from natural

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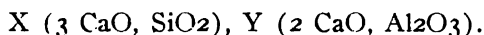
\*Professional Paper, No. 28, Corps of Engineers, U.S.A., p. 30.

mixtures, without pulverizing and artificially mixing the materials prior to burning. Several plants, however, have placed on the market so-called Portland cements made by grinding up together the underburned and overburned materials formed during the burning of natural cements. Several of these brands contain from 5 to 15 per cent. of magnesia; and under no circumstances can they be considered true Portland cements.

In view of the conditions above noted, the writer believes that the following definition will be found more satisfactory than the above quoted:

*Definition of Portland cement.*—Portland cement is an artificial product obtained by finely pulverizing the clinker produced by burning to semi-fusion an intimate mixture of finely ground calcareous and argillaceous material, this mixture consisting approximately of one part of silica and alumina to three parts of carbonate of lime (or an equivalent amount of lime oxide.)

*Composition and Constitution.*—Portland cement may be said to tend toward a composition approximating to pure tricalcic silicate ( $3 \text{ CaO}, \text{SiO}_2$ ) which would correspond to the proportion  $\text{CaO } 73.6\%$ ,  $\text{SiO}_2 \text{ } 26.4\%$ . As can be seen, however, from the published analyses, actual Portland cements as at present made differ in composition very markedly from this. Alumina is always present in considerable quantity, forming with part of the lime, the dicalcic aluminate ( $2 \text{ CaO}, \text{SiO}_2$ ). This would give, as stated by Newberry, for the general formula of a pure Portland.



But the composition is still further complicated by the presence of accidental impurities, or intentionally added ingredients. These last may be simply adulterants, or they may be added to serve some useful purpose. Calcium sulphate is a type of the latter class. It serves to retard the set of the cement, and, in small quantities, appears to have no injurious effect which would prohibit its use for this purpose. In dome kilns, sufficient sulphur trioxide is generally taken up by the cement from the fuel gases to obviate the necessity for the later addition of calcium sulphate, but in the rotary kiln its addition to the ground cement, in the form of either powdered gypsum or plaster-of-Paris, is a necessity.

Iron oxide, within reasonable limits, seems to act as a substitute for alumina, and the two may be calculated together. Magnesium carbonate is rarely entirely absent from limestones or clays, and magnesia is therefore almost invariably present in the finished cement. Though magnesia, when magnesium carbonate is burned at low temperature, is an active hydraulic material, it does not combine with silica or alumina at the clinkering heat employed in Portland cement manufacture. At the best it is an inert and valueless constituent in the cement; many regard it as positively detrimental in even small amounts, and because of this feeling manufacturers prefer to carry it as low as possible. Newberry has stated that in amounts of less than  $3\frac{1}{2}\%$  it is harmless,—and American Portlands from the Lehigh district usually reach well up toward that limit. In European practice it is carried somewhat lower.



## CHAPTER 3.

### RAW MATERIALS. GENERAL CONSIDERATIONS.

For the purposes of the present chapter, it will be sufficiently accurate to consider that a Portland cement mixture, when ready for burning, will consist of about 75 per cent. of lime carbonate ( $\text{Ca CO}_3$ ) and 20 per cent. of silica ( $\text{SiO}_2$ ), alumina ( $\text{Al}_2\text{O}_3$ ) and iron oxide ( $\text{Fe}_2\text{O}_3$ ) together, the remaining 5 per cent. including any magnesium carbonate, sulphur and alkalis that may be present.

The essential elements which enter into this mixture,—lime, silica, alumina and iron,—are all abundantly and widely distributed in nature, occurring in different forms in many kinds of rocks. It can, therefore, be readily seen that, theoretically, a satisfactory Portland cement mixture could be prepared by combining, in an almost infinite number of ways and proportions, many possible raw materials. Obviously, we, too, might expect to find perfect graduations in the *artificialness* of the mixture, varying from the one extreme where a natural rock of absolutely correct composition was used to the other extreme where two or more materials, in nearly equal amounts, are required to make a mixture of correct composition.

The almost infinite number of raw materials which are theoretically available are, however, reduced to a very few in practice under existing commercial conditions. The necessity for making the mixture as cheaply as possible rules out of consideration a large number of materials which would be considered available if chemical composition was the only thing to be taken into account. Some materials otherwise suitable are too scarce; some are too difficult to pulverize. In consequence, a comparatively few combinations of raw materials are actually used in practice.

In certain localities deposits of argillaceous (clayey) limestone or "cement rock" occur, in which the lime, silica, alumina and iron oxide exist in so nearly the proper proportions that only a relatively small amount (say 10 per cent. or so) of other material is required in order to make a mixture of correct composition.

In the majority of plants, however, most or all of the necessary lime is furnished by one raw material, while the silica, alumina and iron oxide are largely or entirely derived from another raw material. The raw material which furnished the lime is usually natural,—a limestone, chalk or marl; but occasionally an artificial product is used, such as the chemically precipitated lime carbonate which results as waste from alkali manufacture. The silica, alumina and iron oxide of the mixture are usually derived from clays, shales or slates; but in a few plants blast-furnace slag is used as the silico-aluminous ingredient in the manufacture of true Portland cement.

The various combinations of raw material which are at present used in the United States in the manufacture of Portland cement may be grouped under six heads. This grouping is as follows:

1. Argillaceous limestone (cement rock) and pure limestone.
2. Pure hard limestone and clay or shale.
3. Soft chalky limestone and clay.
4. Marl and clay.
5. Alkali waste and clay.
6. Slag and limestone.

#### ORIGIN AND GENERAL CHARACTERS OF LIMESTONE.

The cement materials which are described in the four following sections as argillaceous limestone or cement rock, pure hard limestone, chalk, and marl, though differing sufficiently in their physical and economic characters to be discussed separately and under different names, agree in that they are all forms of limestone. The origin, chemical composition, physical characters, and properties of limestone will, therefore, be briefly taken up in the present chapter to serve as an introduction to the more detailed statements concerning the various types of limestone to be found in the succeeding chapters.

*Origin of limestones.\**—Limestones have been formed largely by the accumulation at the sea bottom of the calcareous re-

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\*For a more detailed discussion of this subject the reader will do well to consult Chapter VIII of Prof. J. F. Kemp's "Handbook of Rocks."



mains of such organisms as the foraminifera, corals, and mollusks. Most of the thick and extensive limestone deposits of the United States were probably deep-sea deposits formed in this way. Many of these limestones still show the fossils of which they were formed, but in others all trace of organic origin has been destroyed by the fine grinding to which the shells and corals were subjected before their deposition at the sea-bottom. It is probable also that part of the calcium carbonate of these limestones was a purely chemical deposit from solution, cementing the shell fragments together.

A far less extensive class of limestones—though important in the present connection—owe their origin to the indirect action of organisms. The "marls," so important today as Portland cement materials, fall in this class. As the class is of limited extent, however, its method of origin may be dismissed here, but will be described later in the section on Marls.

Deposition from solution by purely chemical means has undoubtedly given rise to numerous limestone deposits. When this deposition took place in caverns or in the open air, it gave rise to onyx deposits and to the "travertine marls" of certain Ohio and other localities; when it took place in isolated portions of the sea through the evaporation of the sea water it gave rise to the limestone beds which so frequently accompany deposits of salt and gypsum.

*Varieties of limestone.*—A number of terms are in general use for the different varieties of limestone, based upon differences of origin, texture, composition, etc. The more important of these terms will be briefly defined.

The *marbles* are limestones which, through the action of heat and pressure, have become more or less distinctively crystalline. The term *marl*, as at present used in cement manufacture, is applied to a loosely cemented mass of lime carbonate formed in lake basins as described on a later page. *Calcareous tufa* and *travertine* are more or less compact limestones deposited by spring or stream waters along their courses. *Oolitic* limestones, so called because of their resemblance to a mass of fish-roe, are made up of small rounded grains of lime carbonate. *Chalk* is a fine-grained limestone composed of finely comminuted shells, particularly those of the foraminifera. The presence of much silica gives rise to a *siliceous* or *cherty* limestone. If the

silica present is in combination with alumina, the resulting limestone will be *clayey* or *argillaceous*.

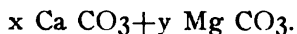
*Chemical composition of limestone*—A theoretically pure limestone is merely a massive form of the mineral calcite. Such an ideal limestone would therefore consist entirely of calcium carbonate or carbonate of lime, with the formula  $\text{CaCO}_3$  ( $\text{CaO} + \text{CO}_2$ ), corresponding to the composition calcium oxide ( $\text{CaO}$ ) 56 per cent.; carbon dioxide or carbonic acid ( $\text{CO}_2$ ) 44 per cent.

As might be expected, the limestones we have to deal with in practice depart more or less widely from this theoretical composition. These departures from ideal purity may take place along either of two lines,—

- a. The presence of magnesia in place of part of the lime;
- b. The presence of silica, iron, alumina, alkalies, or other impurities.

It seems advisable to discriminate between these two cases, even though a given sample of limestone may fall under both heads, and they will therefore be discussed separately.

a. *The presence of magnesia in place of part of the lime.*—The theoretically pure limestones are, as above noted, composed entirely of calcium carbonate and correspond to the chemical formula  $\text{CaCO}_3$ . Setting aside for the moment the question of the presence or absence of such impurities as iron, alumina, silica, etc., it may be said that lime is rarely the only base in a limestone. During or after the formation of the limestone a certain percentage of magnesia is usually introduced in place of part of the lime, thus giving a more or less magnesian limestone. In the magnesian limestones part of this calcium carbonate is replaced by magnesium carbonate ( $\text{Mg CO}_3$ ), the general formula for a magnesian limestone being therefore



In this formula  $x$  may vary from 100% to zero, while  $y$  will vary inversely from zero to 100%. In the particular case of this replacement where the two carbonates are united in equal *molecular* proportions, the resultant rock is called *dolomite*. It has the formula,— $\text{CaCO}_3, \text{MgCO}_3$ —corresponding to the composition calcium carbonate 54.35 per cent.; magnesium carbonate 45.65 per cent. In the case where the calcium carbonate has been entirely replaced by magnesium carbonate, the resulting

pure carbonate of magnesia is called magnesite, having the formula  $\text{MgCO}_3$  and the composition magnesia ( $\text{MgO}$ ) 47.6 per cent.; carbon dioxide ( $\text{CO}_2$ ), 52.4 per cent.

Rocks of this series may therefore vary in composition from pure calcite-limestones at one end of the series to pure magnesite at the other. The term limestone has, however, been restricted in general use to that part of the series lying in composition between calcite and dolomite, while all those more uncommon phases carrying more magnesium carbonate than the 45.65 per cent. of dolomite are usually described simply as impure magnesites.

The presence of much magnesia in the finished cement is considered undesirable,  $3\frac{1}{2}$  per cent. being the maximum permissible under most specifications, and therefore the limestone to be used in Portland cement manufacture should carry not over 5 to 6 per cent. of magnesium carbonate.

Though magnesia is often described as an "impurity" in limestone, this word, as can be seen from the preceding statements, hardly expresses the facts in the case. The magnesium carbonate present, whatever its amount, simply serves to replace an equivalent amount of calcium carbonate, and the resulting rock, whether little or much magnesia is present, is still a pure carbonate rock. With the impurities to be discussed in later paragraphs, however, this is not the case. Silica, alumina, iron, sulphur, alkalies, etc., when present, are actual impurities, not merely chemical replacements of part of the calcium carbonate.

b. *The presence of silica, iron, alumina, alkalies, and other impurities.*—Whether a limestone consists of pure calcium carbonate or more or less of magnesium carbonate, it may also contain a greater or lesser amount of distinct impurities. From the point of view of the cement manufacturer, the more important of these impurities are silica, alumina, iron, alkalies, and sulphur, all of which have a marked effect on the value of the limestone as a cement material. These impurities will therefore be taken up in the order in which they are named above.

The silica in a limestone may occur either in combination with alumina, as a clayey impurity, or not combined with alumina. As the effect on the value of the limestone would be very different in the two cases, they will be taken up separately.

*Silica alone.*—Silica, when present in a limestone containing no alumina, may occur in one of three forms, and the form in

which it occurs is of great importance in connection with cement manufacture.

(1) In perhaps its commonest form, silica is present in nodules, masses or beds of flint or chert. Silica occurring in this form will not readily enter into combination with the lime of a cement mixture, and a cherty or flinty limestone is therefore almost useless in cement manufacture.

(2) In a few cases, as in the hydraulic limestone of Teil, France, a large amount of silica is present and very little alumina; notwithstanding which the silica readily combines with the lime on burning. It is probable that in such cases the silica is present in the limestone in a very finely divided condition, or possibly as hydrated silica, possibly as the result of chemical precipitation or of organic action. In the majority of cases, however, a highly siliceous limestone will not make a cement on burning unless it contains alumina in addition to the silica.

(3) In the crystalline limestone (marbles) and less commonly in uncrystalline limestones, whatever silica is present may occur as a complex silicate in the form of shreds of mica, hornblende, or other silicate mineral. In this form silicate is somewhat intractable in the kiln, and mica and other silicate minerals are therefore to be regarded as inert and useless impurities in a cement rock. These silicates will flux at a lower temperature than pure silica and are thus not so troublesome as flint or chert. They are, however, much less serviceable than if the same amount of silica were present in combination with alumina as a clay.

*Silica with alumina.*—Silica and alumina, combined in the form of clay, are common impurities in limestone, and are of special interest to the cement manufacturer. The best known example of such an argillaceous limestone is the cement rock of the Lehigh district of Pennsylvania. Silica and alumina, when present in this combined form, combine readily with the lime under the action of heat, and an argillaceous limestone therefore forms an excellent basis for a Portland cement mixture.

*Iron.*—Iron when present in a limestone occurs commonly as the oxide ( $\text{Fe}_2\text{O}_3$ ), or sulphide ( $\text{FeS}_2$ ); more rarely as iron carbonate or in a complex silicate. Iron in the oxide, carbonate or silicate form, is a useful flux, aiding in the combination of the

lime and silica in the kiln. When present as a sulphide, in the form of the mineral pyrite it is to be avoided in quantities over 2 or 3 per cent.

*Physical characters of limestones.*—In texture, hardness, and compactness, the limestones vary from the loosely consolidated marls through the chalks to the hard compact limestones and marbles. Parallel with these variations are variations in absorptive properties and density. The chalky limestones may run as low in specific gravity as 1.85, corresponding to a weight of say 110 pounds per cubic foot, while the compact limestones commonly used for building purposes range in specific gravity between 2.3 and 2.9, corresponding approximately to a range in weight of from 140 to 185 pounds per cubic foot.

From the point of view of the Portland cement manufacturer, these variations in physical properties are of economic interest chiefly in their bearing upon two points: the percentage of water carried by the limestone as quarried, and the ease with which the rock may be crushed and pulverized. To some extent the two properties counterbalance each other; the softer the limestone the more absorbent is it likely to be. These purely economic features will be discussed in more detail in later chapters.

*Effect of heating on limestone.*—On heating a non-magnesian limestone to or above  $300^{\circ}\text{C.}$ , its carbon dioxide will be driven off, leaving quicklime (calcium oxide,  $\text{CaO}$ ). If a magnesian limestone be similarly treated, the product would be a mixture of calcium oxide and magnesium oxide ( $\text{MgO}$ ). The rapidity and perfection of this decomposition can be increased by passing steam or air through the burning mass. In practice this is accomplished either by the direct injection of air or steam, or more simply by thoroughly wetting the limestone before putting it into the kiln.

If, however, the limestone contains an appreciable amount of silica, alumina and iron, the effects of heat will not be of so simple a character. At temperature of  $800^{\circ}\text{C.}$  and upwards these clayey impurities will combine with the lime oxide, giving silicates, aluminates and related salts of lime. In this manner a natural cement will be produced. An artificial mixture of certain and uniform composition, burned at a higher temperature, will give a Portland cement, the details of whose manufacture are discussed on later pages.

## CHAPTER 4.

### RAW MATERIALS IN DETAIL.

#### *Argillaceous Limestone: Cement Rock.*

An argillaceous limestone containing approximately 75 per cent. of lime carbonate and 20 per cent. of clayey materials (silica, alumina, and iron oxide), would, of course, be the ideal material for use in the manufacture of Portland cement, as such rock would contain within itself in the proper proportions all the ingredients necessary for the manufacture of a good Portland. It would require the addition of no other material, but when burnt alone would give a good cement. This ideal cement material is, of course, never realized in practice, but certain deposits of argillaceous limestone approach the ideal composition very closely.

The most important of these argillaceous limestone or "cement rock" deposits is, at present, that which is so extensively utilized in Portland cement manufacture in the "Lehigh district" of Pennsylvania and New Jersey. As this area still furnishes about two-thirds of all the Portland cement manufactured in the United States, its raw materials will be described in some detail.

*Cement rock of the Lehigh district.*—The Lehigh district of the cement trade comprises parts of Berks, Lehigh, and Northampton counties, Pennsylvania, and of Warren county, New Jersey. Within this relatively small area about twenty Portland cement mills are located, producing slightly over two-thirds of the entire American output. As deposits of the cement rock used by these plants extend far beyond the present "Lehigh district," a marked extension of the district will probably take place as the needs for larger supplies of raw material becomes more apparent.

The "cement rock" of the Lehigh district is a highly argillaceous limestone of Trenton (Lower Silurian) age. The formation is about 300 feet in thickness in this area. The rock is a very dark gray in color and usually has a slaty fracture. In composition it ranges from about 60 per cent. lime carbonate



with 30 per cent. of clayey material, up to say 80 per cent. lime carbonate with 15 per cent. of silica, alumina and iron. The lower beds of the formation are always higher in lime carbonate than are the beds nearer the top of the formation. The content of magnesium carbonate in these cement rocks is always high, (as Portland cement materials go), ranging from 3 to 6 per cent.

Near, and in some cases immediately underlying these cement beds, are beds of purer limestone ranging from 85 to 96 per cent. lime carbonate. The usual practice in the Pennsylvania and New Jersey plants has been therefore to mix a relatively small amount of this purer limestone with the low lime "cement rock" in such proportions as to give a cement mixture of proper composition.

The economic and technologic advantages of using such a combination of materials are very evident. Both the pure limestone and the cement rock, particularly the latter, can be quarried very easily and cheaply. As quarried they carry but little water so that the expense of drying them is slight. The fact that about four-fifths of the cement mixture will be made up of a natural cement rock permits coarser grinding of the raw mixture than would be permissible in plants using pure limestone or marl with clay. This point is more fully explained on a later page. It seems probable, also, that when using a natural cement rock as part of the mixture the amount of fuel necessary to clinker the mixture is less than when pure limestone is mixed with clay.

Such mixtures of argillaceous limestone or "cement rock" with a small amount of pure limestone evidently possess important advantages over mixtures of pure hard limestone or marl with clay. They are, on the other hand, less advantageous as cement materials than the chalky limestones discussed on later pages.

The analyses in Table 2 are fairly representative of the materials employed in the Lehigh district. The first four analyses are of "cement rock"; the last two are of the purer limestone used for mixing with it.

*Analyses of Lehigh district cement materials.*

	Cement rock				Limestone	
Silica ( $\text{SiO}_2$ ) .....	10.02	9.52	14.52	16.10	8.02	1.98
Alumina ( $\text{Al}_2\text{O}_3$ ) .. ...	6.26	4.72	6.52	2.20	1.90	0.70
Iron Oxide ( $\text{Fe}_2\text{O}_3$ ) ...						
Lime carbonate ( $\text{CaCO}_3$ )..	78.65	80.71	73.52	76.23	92.05	95.15
Magnesium carbon- ate ( $\text{MgCO}_3$ ) .....	4.71	4.92	4.69	3.54	3.04	2.03

"Cement rock" in other parts of the United States.—Certain Portland cement plants, particularly in the western United States, are using combinations of materials closely similar to those in the Lehigh district. Analyses of the materials used at several of these plants are given in Table III.

*Analyses of "cement rock" materials from the western United States.*

	Utah.		California		Colorado	
	Cement rock	Limestone	Cement rock	Limestone	Cement rock	Limestone
Silica ( $\text{SiO}_2$ ) .....	21.2	6.8	20.06	7.12	14.20	
Alumina ( $\text{Al}_2\text{O}_3$ ) .....	8.0	3.0	10.07	2.36	5.21	
Iron Oxide ( $\text{Fe}_2\text{O}_3$ ) .....			3.39	1.16	1.73	
Lime carbonate ( $\text{CaCO}_3$ )..	62.08	89.8	63.40	87.70	75.10	88.0
Magnesium carbonate ( $\text{MgCO}_3$ ) .....	3.8	0.76	1.54	0.84	1.10	

In addition to the "cement rocks" noted in this chapter, it is necessary to call attention to the fact that many of the chalky limestones discussed on page 26 are sufficiently argillaceous

ous to be classed as 'cement rocks.' Because of their softness, however, all the chalky limestones will be described together.

*Pure hard limestones.*

Soon after the American Portland cement industry had become fairly well established in the Lehigh district, attempts were made in New York State to manufacture Portland cement from a mixture of pure limestone and clay. These attempts were not commercially successful, and although their lack of success was not due to any defects in the limestone used, a certain prejudice arose against the use of the hard limestones. In recent years, however, this has disappeared, and a very large proportion of the American output is now made from mixtures of limestone with clay or shale. (See page 21 for comparative figures.) This reestablishment in favor of the hard limestones is doubtless due, in great part, to recent improvements in grinding machinery, for the purer limestones are usually much harder than argillaceous limestones like the Lehigh district "cement rock," and it was very difficult to pulverize them finely and cheaply with the crushing appliances in use when the Portland cement industry was first started in America.

A series of analyses of representative pure hard limestones, together with analyses of the clays or shales with which they are mixed, is given in the table.

*Analyses of pure hard limestones and clayey materials.*

Limestones.				
Silica (SiO <sub>2</sub> ) .....	1.72	0.86	0.56	0.40
Alumina (Al <sub>2</sub> O <sub>2</sub> ) .....	1.63	0.63	1.23	0.44
Iron oxide (Fe <sub>2</sub> O <sub>3</sub> ) .....	6.59	1.03	0.29	
Lime carbonate (CaCO <sub>3</sub> ) ....	90.58	97.06	97.23	97.99
Magnesium carbonate (MgCO <sub>3</sub> ) .....	.....	.....	0.75	0.42

Clays and Shales.				
Silica (SiO <sub>2</sub> ) .....	63.56	55.80	56.30	60.00
Alumina (Al <sub>2</sub> O <sub>2</sub> ) .....	27.32	30.20	29.86	23.36
Iron oxide (Fe <sub>2</sub> O <sub>3</sub> ) .....				4.32
Lime carbonate (CaCO <sub>3</sub> ) ....	3.60	2.54	.....	1.70
Magnesium carbonate (MgCO <sub>3</sub> ) .....	2.60	.....	.....	1.50

The first limestone analysis given in the above table represents a curious type, used in several plants in the Middle West. As will be noted, it is a relatively impure limestone, but its principal impurity is iron oxide. It contains 8.22 per cent. of iron oxide and alumina, as compared with 1.72 per cent. of silica; and therefore demands great care in the selection of a suitable high-silica clay to mix with it.

### *Soft Limestones: Chalk.*

*Origin and general character.*—Chalk, properly speaking, is a pure carbonate of lime composed of the remains of the shells of minute organisms, among which those of foraminifera are especially prominent. The chalks and soft limestones discussed in this chapter agree, not only in having usually originated in this way, but also in being rather soft and therefore readily and cheaply crushed and pulverized. As Portland cement materials they are, therefore, almost ideal. One defect, however, which to a small extent counterbalances their obvious advantages is the fact that most of these soft, chalky limestones absorb water quite readily. A chalky limestone which in a dry season will not carry over 2 per cent. of moisture as quarried, may, in consequence of prolonged wet weather show as high as 15 or 20 per cent. of water. This difficulty can, of course, be avoided if care be taken in quarrying to avoid unnecessary exposure to water and, if necessary, to provide facilities for storing a supply of the raw materials during wet seasons.

*Geographic and geologic distribution in the United States.*—The chalks and chalky limestones are confined almost entirely to certain southern and western States. They are all of approximately the same geologic ages,—Cretaceous or Tertiary,—and are mostly confined to one division of the Cretaceous. The principal chalk or soft limestone deposits available for use in Portland cement manufacture occur in three widely separated areas, occupying respectively (a) parts of Alabama and Mississippi; (b) parts of Texas and Arkansas; and, (c) parts of Iowa, Nebraska, North and South Dakota.

*Composition.*—In composition these chalks, or “rotten limestones,” vary from a rather pure calcium carbonate, low in both magnesia and clayey materials, to an impure clayey limestone, requiring little additional clay to make it fit for use in Portland cement manufacture. Analyses quoted from various authors of a number of these chalky limestones are given in Table IV, and will serve to show their range of composition.

*Analyses of Chalky Limestones.*

	Demopolis, Ala.	San Antonio, Texas.	Dallas, Texas.	White Cliffs, Ark.	Yankton, S. Dak.	Milton, N. Dak.
Silica .....	12.13	5.77	23.55	7.97	8.20	9.15
Alumina ....	4.17	2.12	1.50	1.09	7.07	4.80
Iron oxide .....	3.28					
Lime carbonate ..	75.07	90.15	70.21	88.64	83.59	63.75
Magnesium carb'te	.92	.15	.58	.73	n. d.	1.25

*Fresh-Water Marls.*

Marls, in the sense in which the term is used in the Portland cement industry, are incoherent limestones which have been deposited in the basins of existing or extinct lakes. So far as chemical composition is concerned, marls are practically pure limestones, being composed entirely of calcium carbonate. Physically, however, they differ greatly from the compact rocks which are commonly described as limestones, for the marls are granular, incoherent deposits. This curious physical character of marls is due to the conditions under which they have been deposited, and varies somewhat according to the particular conditions which governed their deposition in different localities.

A warning to the reader concerning other uses of the term “marl” may profitably be introduced here. The meaning above given is that in which the term marl is commonly used in the cement industry at the present day. But in geological and agricultural reports, particularly in those issued before the Port-

land cement industry became prominent in this country, the term marl has been used to cover several very different substances. The following three uses of the term will be found particularly common, and must be guarded against when such reports are being examined in search for descriptions of deposits of cement materials.

(1.) In early days the term "marls" and "marlytes" were used to describe deposits of calcareous shales—and often covered shales which were not particularly calcareous. This use of the term will be found in many of the earlier geological reports issued by New York, Ohio, and other interior States.

(2.) In New Jersey and the States southward bordering on the Atlantic and Gulf of Mexico, the term marl is commonly applied to deposits of soft chalky or unconsolidated limestone, often containing considerable clayey and phosphatic matter. These limestones are of marine origin, and not related to the fresh-water marl deposits which are the subject of the present chapter.

(3.) In the same States as are included in the last paragraph, but particularly in New Jersey and Virginia, large deposits of the so-called 'green sand marls' occur. This material is, in no way, related to the true marls (which are essentially lime carbonates), but consists almost entirely of an iron silicate, with very small percentages of clayey, calcareous, and phosphatic matter.

*Origin of marls.*—The exact cause of the deposition of marls has been the subject of much investigation and discussion, particularly in the past few years, since they have become of economic importance. The reader who wishes to obtain further details concerning this question will do well to refer to the following series of papers.

(1.) Blatchley, W. S., and Ashley, G. H. The Lakes of Northern Indiana, and their associated marl deposits, in 25th Ann. Rept. Indiana Dept. Geology and Natural Resources, pp. 31-321.

(2.) Davis, C. A. A contribution to the natural history of marl. Journal of Geology, Vol. 8, pp. 485-497.

(3.) Davis, C. A. Second contribution to the natural history of marl. Journal of Geology, Vol. 9, pp. 491-506.

(4.) Davis, C. A. A contribution to the natural history of marl. Vol. 8, pt. 3, Reports Michigan Geological Survey, pp. 65-102.

(5.) Lane, A. C. Notes on the origin of Michigan bog limes. Vol. 8, pt. 3, Reports Michigan Geological Survey, pp. 199-223.

Disregarding the points in controversy, which are of no particular practical importance, it may be said that marls are deposited in lakes by spring or stream waters carrying lime carbonate in solution. The actual deposition is in part due to purely physical and chemical causes, and in part to the direct or indirect action of animal or vegetable life. The result, in any case, is that a calcareous deposit forms along the sides and over the bottom of the lake, this deposit consisting of lime carbonate, mostly in a finely granular form, interspersed with shells and shell fragments.

*Geographic distribution of marl deposits.*—The geographic distribution of marl deposits is intimately related to the geologic history of the region in which they occur. Marl beds are, as indicated in the preceding section, the result of the filling of lake basins. Lakes are not common except in those portions of the United States which were affected by glacial action, since lakes are in general due to the damming of streams by glacial material. Workable marl deposits, therefore, are almost exclusively confined to those portions of the United States and Canada lying north of the former southern limit of the glaciers.

Marl beds are found in the New England States, where they are seldom of important size, and in New York, where large beds occur in the central and western portions of the State. Deposits are frequent and important in Michigan, and in the northern portions of Ohio, Indiana, and Illinois. Marl beds occur in Wisconsin and Minnesota, but have not been as yet exploited for cement manufacture.

*Composition.*—As shown by the analyses below, marls are usually very pure lime carbonates. They, therefore, require the addition of considerable clay to bring them up to the proper composition for a Portland cement mixture.

The marls are readily excavated, but necessarily carry a large percentage of water. The mixture, on this account, is commonly made in the wet way, which necessitates driving off a high percentage of water in the kilns. Analyses of typical marls and clays are given in the following table.

*Analyses of marls and clays used in cement plants.*

	Marl.			Clay.		
Silica .....	0.25	3.0	1.60	40.48	52.0	63.75
Alumina ....	.10		1.55	20.95	17.0	16.40
Iron oxide .....					5.0	6.35
Lime carbonate ...	94.39	93.0	88.9	25.80	20.0	4.0
Magnesium carb'te	.38	1.5	.94	.99	....	2.1

*Alkali Waste.*

A very large amount of waste material results from the process used at alkali works in the manufacture of caustic soda. This waste material is largely a precipitated form of calcium carbonate, and if it is sufficiently free from impurities, it furnishes a cheap source of lime for use in Portland cement manufacture.

The availability of alkali waste for this purpose depends largely on what process was used at the alkali plant. Leblanc process waste, for example, carries a very large percentage of sulphides, which prevents its use as a Portland cement material. Waste resulting from the use of the ammonia process, on the other hand, is usually a very pure mass of lime, mostly in the form of carbonate, though a little lime hydrate is commonly also present. As pyrite is not used in the ammonia process, its waste is usually low enough in sulphur to be used as a cement material. The waste may carry a low or a very high percentage of magnesia, according to the character of the limestone that has been used. When a low-magnesia limestone has been used, the resulting waste is a very satisfactory Portland cement material.

The following analyses are fairly representative of the waste obtained at alkali plants using the ammonia process.



*Analyses of alkali waste.*

	1	2	3	4
Silica ( $\text{SiO}_2$ ) .....	0.60	1.75	1.98	0.98
Alumina ( $\text{Al}_2\text{O}_3$ ) .....	{	{ 0.61	{ 1.41	{ 1.62
Iron oxide ( $\text{Fe}_2\text{O}_3$ ) .....	{ 3.04	{	{ 1.38	{
Lime ( $\text{CaO}$ ) .....	53.33	50.60	48.29	50.44
Magnesia ( $\text{MgO}$ ) .....	0.48	5.35	1.51	4.97
Alkalies ( $\text{Na}_2\text{O}$ , $\text{K}_2\text{O}$ ) .....	0.20	0.64	0.64	0.50
Sulphur trioxide ( $\text{Co}_3$ ) .....	n.d.	n.d.	1.26	n.d.
Sulphur (S) .....	n.d.	0.10	n.d.	0.06
Carbon dioxide ( $\text{CO}_2$ ) .....	42.43	{	39.60	n.d.
Water and organic matter .....	n.d.	{ 41.70	3.80	n.d.

Of the analyses quoted in the preceding table, those in the first and third columns represent materials which are actually used in Portland cement manufacture in England and the United States. The alkali wastes whose analyses are given in the second and fourth columns are notably too high in magnesia to be advisable for such use.

*Blast furnace slag.*

True Portland cements, which must be sharply distinguished from the slag (or puzzolan) cements can be made from mixtures which contain blast furnace slag as one ingredient. In this case the slag is intimately mixed with limestone and the mixture is finely powdered. It is then burned in kilns and the resulting clinker pulverized.

The slags from iron furnaces consist essentially of lime ( $\text{CaO}$ ), silica ( $\text{SiO}_2$ ), and alumina ( $\text{Al}_2\text{O}_3$ ); though small percentages of iron oxide ( $\text{FeO}$ ), magnesia ( $\text{MgO}$ ), and sulphur (S), are commonly present. Slag may therefore be regarded as a very impure limestone or a very calcareous clay.

The slag used at a German Portland cement plant has the following range in composition.

*Analysis of slag used in Portland cement manufacture.*

Silicia ( $\text{SiO}_2$ ) .....	30.	35.
Alumina ( $\text{Al}_2\text{O}_3$ ) .....	10.	14.
Iron oxide ( $\text{FeO}$ ) .....	0.2	1.2
Lime ( $\text{CaO}$ ) .....	46.	49.
Magnesia ( $\text{MgO}$ ) .....	0.5	3.5
Sulphur trioxide ( $\text{SO}_3$ ) .....	0.2	0.6

*Clays and Shales.*

Clays are ultimately derived from the decay of older rocks, the finer particles resulting from this decay being carried off and deposited by streams along their channels, in lakes, or along parts of the sea coast or sea bottom as beds of clay. In chemical composition the clays are composed essentially of silica and alumina, though iron oxide is almost invariably present in more or less amount, while lime, magnesia, alkalis and sulphur are of frequent occurrence, though usually only in small percentages.

Shales are clays which have become hardened by pressure. The so-called "fire-clays" of the Coal Measures are shales, as are many of the other "clays" of commerce.

For use as Portland cement materials clays or shales should be as free as possible from gravel and sand, as the silica present as pebbles or grit is practically inert in the kiln unless ground more finely than is economically practicable. In composition they should not carry less than 55 per cent. of silica, and preferably from 60 to 70 per cent. The alumina and iron oxide together should not amount to more than one-half the percentage of silica, and the composition will usually be better the nearer the ratio  $\text{Al}_2\text{O}_3 + \text{Fe}_2\text{O}_3 = \text{SiO}_2$  is approached.

## 3

Nodules of lime carbonate, gypsum or pyrite, if present in any quantity, are undesirable; though the lime carbonate is not absolutely injurious. Magnesia and alkalis should be low, preferably not above 3 per cent.

Analyses of clays and shales used in various American Portland cement plants will be found on pages 27 and 30.

*Slate.*

Slate is, so far as origin is concerned, merely a form of shale in which a fine, even and parallel cleavage has been developed by pressure. In composition, therefore, it will vary exactly as do the shales considered on previous pages, and so far as composition alone is concerned, slate would not be worthy of more attention, as a Portland cement material, than any other shale.

Commercial considerations in connection with the slate industry, however, make slate a very important possible source of cement material. Good roofing slate is a relatively scarce material, and commands a good price when found. In the preparation of roofing slate for the market so much material is lost during sawing, splitting, etc., that only about 10 to 25 per cent. of the amount quarried is salable as slate. The remaining 75 to 90 per cent. is of no service to the slate miner. It is sent to the dump heap, and is a continual source of trouble and expense. This very material, however, as can be seen from the analyses quoted below, is often admirable for use, in connection with limestone, in a Portland cement mixture. As it is a waste product, it could be obtained very cheaply by the cement manufacturer.

*Composition of American roofing slates.*

	Max.	Average	Min.
Silica ( $\text{SiO}_2$ ) .....	68.62	60.64	54.05
Alumina ( $\text{Al}_2\text{O}_3$ ) .....	24.71	18.05	9.77
Iron oxides ( $\text{FeO}$ , $\text{Fe}_2\text{O}_3$ ) .....	10.66	6.87	2.18
Lime ( $\text{CaO}$ ) .....	5.23	1.54	....
Magnesia ( $\text{MgO}$ ) ....	6.43	2.60	0.12
Alkalies ( $\text{K}_2\text{O}$ , $\text{Na}_2\text{O}$ ) .....	8.68	4.74	1.93
Ferrous sulphide ( $\text{FeS}_2$ ) ....	....	0.38	....
Carbon dioxide ( $\text{CO}_2$ ) ....	....	1.47	....
Water of combination .....	....	3.51	....
Moisture, below $110^\circ\text{C}$ . ....	....	0.62	....

## CHAPTER 5.

### ECONOMIC CONSIDERATIONS AND METHODS OF MANUFACTURE.

Determining the possible value for Portland cement manufacture of a deposit of raw material is a complex problem, depending upon a number of distinct factors, all of which must be given due consideration. The more important of these factors are:

- (1.) Chemical composition of the material.
- (2.) Physical character of the material.
- (3.) Amount of material available.
- (4.) Location of the deposit with respect to transportation routes.
- (5.) Location of the deposit with respect to fuel supplies.
- (6.) Location of the deposit with respect to markets.

The natural raw materials used at present in Portland cement manufacture are obtained by one of three methods,—(a) quarrying; (b) mining, and (c) dredging. When the cement manufacturer is given an opportunity to choose between these different methods of excavation, his choice will depend partly on the physical character of the material to be excavated and partly on the topographical and geological conditions. Usually, however, there is no opportunity for a choice of methods, for in any given case one of the methods will be so evidently the only possible mode of handling the material as to leave no room for other considerations.

The three different methods of excavation will first be briefly considered, after which the cost of raw materials at the mill will be discussed.

*Quarrying.*—In the following pages the term “quarrying” will be used to cover all methods of obtaining raw materials from open excavations,—quarries, cuts or pits—whether the material excavated be a limestone, a shale or a clay. Quarrying is the most natural and common method of excavating the

raw materials for cement manufacture. If marl, which is usually worked by dredging, be excluded from consideration, it is probably within safe limits to say that 95 per cent. of the raw materials used at American Portland cement plants are obtained by quarrying. If marls be included, the percentages excavated by different methods would probably be about as follows: Quarrying, 88 per cent.; dredging, 10 per cent.; mining, 2 per cent.

In the majority of limestone quarries the material is blasted out and loaded by hand on to cars or carts. In a few limestone quarries a steam shovel is employed to do the loading, and in shale quarries this use of steam shovels is more frequent. In certain clay and shale pits, where the materials are of suitable character, the steam shovel does all the work, both excavating and loading the raw materials.

The rock is usually shipped to the mill as quarried without any treatment except sledging it to convenient size for loading. At a few quarries, however, a crushing plant is installed at the quarry, and the rock is sent as crushed stone to the mill. A few plants also have installed their driers at the quarry, and dry the stone before shipping it to the mill. Except the saving of mill space thus attained, this practice seems to have little to commend it.

*Mining.*—The term "mining" will be used, in distinction from "quarrying," to cover methods of obtaining any kind of raw material by underground workings, through shafts or tunnels. Mining is, of course, rarely employed in excavating materials of such low value per ton as the raw materials for Portland cement manufacture. Occasionally, however, when a thin bed of limestone or shale is being worked, its dip will carry it under such a thickness of other strata as to make mining cheaper than stripping and quarrying, for that particular case.

Mining is considerably more expensive work than quarrying, but there are a few advantages about it that serve to counterbalance the greater cost per ton of raw material. A mine can be worked steadily and economically in all kinds of weather, while an open cut or quarry is commonly in a more or less unworkable condition for about three months of the year. Material won by mining is, moreover, always dry and clean.

*Dredging.*—The term "dredging" will be here used to cover all methods of excavating soft, wet, raw materials. The fact

that the materials are wet implies that the deposit occurs in a basin or depression; and this in turn implies that the mill is probably located at a higher elevation than the deposit of raw material, thus necessitating up-hill transportation to the mill.

The only raw material for Portland cement manufacture that is extensively worked by dredging, in the United States, is marl. Occasionally the clay used is obtained from deposits overlain by more or less water; but this is rarely done except where the marl and clay are interbedded or associated in the same deposit.

A marl deposit, in addition to containing much water diffused throughout its mass, is usually covered by a more or less considerable depth of water. This will frequently require the partial draining of the basin in order to get tracks laid near enough to be of service.

In dredging marl the excavator is frequently mounted on a barge, which floats in a channel resulting from previous investigation. Occasionally, in deposits which either were originally covered by very little water or have been drained, the shovel is mounted on a car, running on tracks laid along the edge of the deposit.

The material brought up by the dredge may be transported to the mill in two different ways, the choice depending largely upon the manufacturing processes in use at the plant. At plants using dome or chamber kilns, or where the marl is to be dried before sending to the kiln, the excavated marl is usually loaded by the shovel on cars, and hauled to the mill by horse or steam power. At normal marl plants, using a very wet mixture, it is probable that the second method of transportation is more economical. This consists of dumping the marl from the excavator into tanks, adding sufficient water to make it flow readily, and pumping the fluid mixture to the mill in pipes.

*Cost of raw materials at mill.*—The most natural way, perhaps, to express the cost of the raw material delivered at the mill would be to state it as being so many cents uper ton or cubic yard of raw material; and this is the method followed by quarrymen or miners in general. To the cement manufacturer, however, such an estimate is not so suitable as one based on the cost of raw materials per ton or barrel of finished cement.

In the case of hard and comparatively dry limestones or shales, it may be considered that the raw material loses 33 1-3 per cent.

in weight on burning. Converting this relation into pounds of raw material and of clinker we find that 600 pounds of dry raw material will make about 400 pounds of clinker. Allowing something for other losses in the process of manufacture, it is convenient and sufficiently accurate to estimate that 600 pounds of dry raw material will give one barrel of finished cement. These estimates must be increased if the raw material carry any appreciable amount of water. Clays will frequently contain 15 per cent. or more of water; while soft chalky limestones, if quarried during wet weather, may carry as high as 15 to over 20 per cent. A Portland cement mixture composed of a pure chalky limestone and a clay might, therefore, average 10 to 20 per cent. of water; and consequently about 700 pounds of such a mixture would be required to make one barrel of finished cement.

With marls the loss on drying and burning is much greater. Russell states\* that according to determinations made by E. D. natural deposits, contains about 47 1-2 pounds of lime carbonate and 48 pounds of water. In making cement from a mixture of marl and clay, therefore, it would be necessary to figure on excavating and transporting over 1,000 pounds of raw material for every barrel of finished cement.

From the preceding notes it will be understood that the cost of raw materials at the mill, per barrel of cement, will vary not only with the cost of excavation, but with the kind of materials in use.

In dealing with hard dry materials, extracted from open quarries near the mills, the cost of raw materials may vary between 8 cents and 15 cents per barrel of cement. The lower figure named is probably about the lowest attainable with good management and under favorable natural conditions; the higher figure is probably a maximum for fairly careful management of a quarry under eastern labor conditions. When it is necessary to mine the materials, the cost will be somewhat increased. Cement rock has been mined at a cost equivalent to 10 cents per barrel of cement; but the figure is attained under particularly favorable conditions. The cost of mining and transportation may reach from this figure up to 20 cents per barrel.

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\*22nd Ann. Rept., U. S. Geol. Surv., pt. 3, p. 657.

## METHODS OF MANUFACTURE.

If, as in the present volume, we exclude from consideration the so-called "natural Portlands," Portland cement may be regarded as being an artificial product, obtained by burning to semi-fusion an intimate mixture of pulverized materials, this mixture containing lime, silica and alumina, varying in proportion only with certain narrow limits; and by crushing finely the clinker resulting from this burning.

If this restricted definition of Portland cement be accepted, four points may be regarded as being of cardinal importance in its manufacture. These are:

- (1) The cement mixture must be of the proper chemical composition.
- (2) The materials of which it is composed must be carefully ground and intimately mixed before burning.
- (3) The mixture must be burned at the proper temperature.
- (4) After burning, the resulting clinker must be finely ground.

The first named of these points—the chemical composition of the mixture—can be more advantageously discussed after the other three points have been disposed of. The subjects will, therefore, be taken up in the following order:

Preparation of the mixture for the kiln.

Burning the mixture.

Grinding the clinker, addition of gypsum, etc.

Composition and properties of Portland cement.

## PREPARATION OF THE MIXTURE FOR THE KILN.

The preparation of the mixture for the kiln involves the reduction of both of the raw materials to a very fine powder, and their intimate mixture. In practice the raw materials are usually crushed more or less finely, and then mixed, after which the final reduction to powder takes place. Two general methods of treatment—the dry and the wet—are in use at different plants. Unless the limey constituent of the mixture is a marl, already full of water, the dry method is almost invariably followed. This consists merely in keeping the materials in as dry



a condition as possible throughout the entire process of crushing and mixing; and, if the raw materials originally contained a little moisture, they are dried before being powdered and mixed. In the wet method, on the other hand, the materials are powdered and mixed while in a very fluid state, containing 60 per cent. or more of water.

**DRYING THE RAW MATERIALS.**—With the exception of the marls and clays used in the wet method of manufacture, Portland cement materials are usually dried before the grinding is commenced. This is necessary because the raw materials, as they come from the quarry, pit or mine, will almost invariably carry appreciable, though often very small, percentages of water, which greatly reduces the efficiency of most modern types of grinding mills, and tends to clog the discharge screens.

**PERCENTAGE OF WATER IN RAW MATERIALS.**—The percentage of water thus carried by the crude raw material will depend largely on the character of the material; partly on the method of handling and storing it; and partly on weather conditions.

In the case of hard limestones, freshly quarried, the water will commonly range from 1-2 per cent. to 3 per cent., rarely reaching or exceeding the higher figure except in the very wet quarries or during a rainy season. Such limestones, comparatively dry when quarried, are frequently sent to the grinding mills without artificial drying.

With the soft, chalky limestones, which absorb water very rapidly, the percentage can usually be kept down to 5 per cent. or less in dry weather; while prolonged wet weather may necessitate the handling at the mill of material carrying as high as 15 to 20 per cent. of water.

The clays present a much more complicated case. In addition to the hygroscopic or mechanically-held water that they may contain, there is also always present a certain percentage of chemically combined water. The amount of hygroscopic water present will depend on the treatment and exposure of the clay; and may vary from 1 per cent. or so in clays which have been stored and air-dried to as high as 30 per cent. in fresh clays. The chemically combined water will depend largely on the composition of the clay, and may vary from 5 to 12 per cent. The hygroscopic or mechanically held water of clays can be driven off at a temperature of 212° F., while the chemically

combined water is lost only at a low red heat. The total water, therefore, to be driven off from clays may range from 6 to 42 per cent., depending on the weather, the drainage of the clay pit, and the care taken in preventing unnecessary exposure to moisture of the excavated clay. The average total amount of moisture will probably be about 15 per cent.

In dealing with shales, the mechanically-held water will rarely rise above 10 per cent., and can commonly be kept well below that limit. An additional 2 to 7 per cent. of water will be carried, by any shale, in a state of chemical combination.

At a few plants marl is used, with clay, in a dry process. As noted elsewhere, the marls, as excavated, carry usually about 50 per cent. of water. This case presents a more difficult problem than do the other raw materials, because the vegetable matter usually present in marls is extremely retentive of water.

It will be seen, therefore, that cement materials may carry from 1 per cent. to 50 per cent. of water when they reach the mill. In a dry process it is necessary to remove practically all of this water before commencing the grinding of the materials. One reason for this is that fine pulverizing can not be economically or satisfactorily accomplished unless absolutely dry material is fed to the grinding machinery.

Another reason, which is one of convenience rather than of necessity, is that the presence of water in the raw materials complicates the calculation of the cement mixture.

*Methods and cost of drying.*—The type of dryer commonly used in cement plants is a cylinder approximately 5 feet in diameter and 40 feet or so in length, set at a slight inclination to the horizontal, and rotating on bearings. The wet raw material is fed in at the upper end of the cylinder, and it moves gradually toward the lower end, under the influence of gravity, as the cylinder revolves. In many dryers angle irons are bolted to the interior in such a way as to lift and drop the raw material alternately, thus exposing it more completely to the action of the heated gases, and materially assisting in the drying process. The dried raw material falls from the lower end of the cylinder into an elevator boot, and is then carried to the grinding mills.

The drying cylinder is heated either by a separate furnace or by waste gases from the cement kiln. In either case the pro-

ducts of combustion are introduced into the cylinder at its lower end, and drawn through it, and escape up a stack set at the upper end of the dryer.

The dryer above described is the simplest, and is most commonly used. For handling the small percentages of water contained in most cement materials it is very efficient, but for dealing with high percentages of water, such as are encountered when marl is to be used in a dry process, it seems probable that double-heating dryers will be found more economical. This type is exemplified by the Ruggles-Coles dryer, in which a double cylinder is employed. The wet raw material is fed into the space between the inner and outer cylinders, while the heated gases pass first through the inner cylinder, and then, in a reverse direction, through the space between the inner and outer cylinders. This double-heating type of dryer is employed in almost all of the slag cement plants in the United States, and is also in use in several Portland cement plants.

When vertical kilns were in use, drying floors and drying tunnels were extensively used, but at present they can be found in only a few places, being everywhere else supplanted by the rotary dryers.

The cost of drying will depend on the cost of fuel, the percentage of water in the wet material and the type of dryer. Even under the most unfavorable conditions five pounds of water can be expected to be evaporated per pound of coal used, while a good dryer will usually evaporate seven or eight pounds of water per pound of coal.

**GRINDING AND MIXING—DRY METHODS.**—Part at least of the grinding is usually accomplished before the drying, but for convenience the subjects have been separated in the present paper. Usually the limestone is sent through a crusher at the quarry or mill before being dried, and occasionally the raw material is further reduced in a Williams mill, etc., before drying, but the principal part of the reduction always takes place after the material has been dried.

After the two raw materials have been separately dried they may be mixed immediately, or each may be further reduced separately before mixing. Automatic mixers, of which many types are on the market, give a mixture in proportions determined upon from analysis of the materials.

The further reduction of the mixture is usually carried on in two stages, the material being ground to say 30 mesh in a ball mill, komminuter, Griffin mill, etc., and finally reduced in a tube mill. At a few plants, however, single stage reduction is practiced in Griffin or Huntington mills, while at the Edison plant at Stewartville, N. J., the reduction is accomplished in a series of rolls.

The majority of plants use either the Griffin mill and tube mill or the ball and tube mills, and there is probably little difference in the cost of operating these two combinations. The ball mill has never been quite as much of a success as its companion, the tube mill, and has been replaced at several plants by the komminuter.

**FINESS OF MIXTURE.**—After its final reduction, and when ready for burning, the mixture will usually run from 90 to 95 per cent. through a 100-mesh sieve. In the plants of the Lehigh district the mixture is rarely crushed as fine as when limestone and clay are used. Newberry\* has pointed out in explanation for this that an argillaceous limestone (cement rock) mixed with a comparatively small quantity of purer limestone, as in the Lehigh plants, requires less thorough mixing and less fine grinding than when a mixture of limestone and clay (or marl and clay) is used, for even the coarser particles of the argillaceous limestone will vary so little in chemical composition from the proper mixture as to affect the quality of the resulting cement but little, should either mixing or grinding be incompletely accomplished.

A very good example of typical Lehigh Valley grinding of raw material is afforded by a specimen examined\* by Prof. E. D. Campbell. This specimen of raw mix ready for burning was furnished by one of the best of the eastern Pennsylvania cement plants. A mechanical analysis of it showed the following results:

	Mesh of sieve.		
	50	100	200
Per cent. passing .....	96.9%	85.6%	72.4%
Per cent. residue .....	3.1%	14.4%	27.6%

The material, therefore, is so coarsely ground that only a trifle over 85 per cent. passes a 100-mesh sieve.

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\*Twentieth Ann. Rept. U. S. Geol. Surv., Pt. 6, p. 545.

\*Journal Amer. Chem. Soc., vol. 25.

#### GRINDING AND MIXING—SLAG-LIMESTONE MIXTURES.—

While the manufacture of Portland cement from a mixture of slag and limestone is similar in general theory and practice to its manufacture from a limestone-clay mixture, certain interesting differences occur in the preparation of the mixture. In the following paragraphs the general methods of preparing mixtures of slag and limestone for use in Portland cement manufacture will first be noted, after which certain processes peculiar to the use of this particular mixture will be described separately.

*General methods.*—After it had been determined that the pozzuolanic cement made\* by mixing slag with lime without subsequent burning of the mixture, was not an entirely satisfactory structural material, attention was soon directed toward the problem of making a true Portland cement from such slag. The blast-furnace slags commonly available, while carrying enough silica and alumina for a cement mixture, are too low in lime to be suitable for Portland cement. Additional lime must be added, usually in the form of limestone; the slag and limestone must be well mixed and the mixture properly burned. The general methods for accomplishing the proper mixture of the materials vary in details. It seems probable that the first method used in attempting to make a true Portland cement from slag, was to dump the proper proportion of limestone, broken into small lumps, into molten slag. The idea was that both mixing and calcination could thus be accomplished in one stage; but in practice it was found that the resulting cement was variable in composition and always low in grade. This method has accordingly fallen into disuse, and at present three different general processes of preparing the mixture are practiced at different European and American plants.

1. The slag is granulated, dried, and ground, while the limestone is dried and ground separately. The two materials are then mixed in proper proportions, the mixture is finely pulverized in tube mills, and the product is fed in a powdered state to rotary kilns.

2. The slag is granulated, dried, and mixed with slightly less than the calculated proper amount of limestone, which has been previously dried and powdered. To this mixture is added

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\*See Municipal Engineering, vol. 24, p. 335, May, 1903.

sufficient powdered slaked lime (say 2 to 6 per cent.) to bring the mixture up to correct composition. The intimate mixture and final reduction are then accomplished in ball and tube mills. About 8 per cent. of water is then added, and the slurry is made into bricks, which are dried and burned in a dome or chamber kiln.

3. Slag is granulated and mixed, while still wet, with crushed limestone in proper proportions. This mixture is run through a rotary calciner, heated by waste kiln gases, in which the temperature is sufficient not only to dry the mixture, but also to partly powder it, and to reduce most of the limestone to quicklime. The mixture is then pulverized and fed into rotary kilns.

Of the three general processes above described, the second is unsuited to American conditions. The first and third are adapted to the use of the rotary kiln. The third seems to be the most economical, and has given remarkably low fuel consumption in practice, but so far has not been taken up in the United States.

Certain points of manufacture peculiar to the use of mixtures of slag and limestone will now be described.

*Composition of the slag.*—The slags available for use in Portland cement manufacture are of quite common occurrence in iron-producing districts. Those best suited for such use are the more basic blast-furnace slags, and the higher such slags run in lime the more available they are for this use. The slags utilized will generally run from 30 to 40 per cent. lime. The presence of over 3 per cent. or so of magnesia in a slag is of course enough to render its use as a Portland cement material inadvisable; and on this account slags from furnaces using dolomite (magnesian limestone) as a flux, are unsuited for cement manufacture. The presence of any notable percentage of sulphur is also a drawback, though, as will be later noted, part of the sulphur in the slag will be removed during the process of manufacture.

*Granulation of slag.*—If slag be allowed to cool slowly it solidifies into a dense, tough material, which is not readily reduced to the requisite fineness for a cement mixture. If it be cooled suddenly, however, as by bringing the stream of molten slag into contact with cold water, the slag is "granulated," i. e., it breaks up into small porous particles. This granulated slag or "slag sand" is much more readily pulverized than a slowly

cooled slag; its sudden cooling has also intensified the chemical activity of its constituents so as to give it hydraulic properties, while part of the sulphur contained in the original slag has been removed. The sole disadvantage of the process of granulating slag is that the product contains 20 to 40 per cent. of water, which must be driven off before the granulated slag is sent to the grinding machinery.

In practice the granulation of the slag is effected by directing the stream of molten slag direct from the furnace into a sheet-iron trough. A small stream of water flows along this trough, the quantity and rate of flow of the water being regulated so as to give complete granulation of the slag without using an excessive amount of water. The trough may be so directed as to discharge the granulated slag into tanks or into box cars, which are usually perforated at intervals along the sides so as to allow part of the water to drain off.

*Drying the slag.*—As above noted, the granulated slag may carry from 20 to 40 per cent. of water. This is removed by treating the slag in rotary driers. In practice such driers give an evaporation of 8 to 10 pounds of water per pound of coal. The practice of slag drying is very fully described in Vol. 10 of the Mineral Industry, pages 84-95, where figures and descriptions of various driers are also given, with data on their evaporative efficiency. As noted earlier in this article, one of the methods of manufacturing Portland cement from slag puts off the drying of the slag until after it has been mixed with the limestone, and then accomplishes the drying by utilizing waste heat from the kilns. Kiln gases could of course be used anyway in the slag driers, but it so happens that they have not been so used except in plants following the method in question.

*Grinding the slag.*—Slag can be crushed with considerable ease to about 50 mesh, but notwithstanding its apparent brittleness it is difficult to grind it finer. Until the introduction of the tube mill in fact it was almost impossible to reduce this material to the fineness necessary for a cement mixture, and the proper grinding of the slag is still an expensive part of the process, as compared with the grinding of limestone, shales, or clay.

*Composition of the limestone.*—As the slag carries all the silica and alumina necessary for the cement mixture, the limestone to be added to it should be simply a pure lime carbonate.

The limestone used for flux at the furnace which supplies the slag will usually be found to be of suitable composition for use in making up the cement mixture.

*Economics of using slag-limestone mixtures.*—The manufacture of a true Portland cement from a mixture of slag and limestone presents certain undoubted advantages over the use of any other raw materials, while it has also a few disadvantages.

Probably the most prominent of the advantages lies in the fact that the most important raw material—the slag—can usually be obtained more cheaply than an equal amount of natural raw material could be quarried or mined. The slag is a waste product, and a troublesome material to dispose of, for which reasons it is obtained at small expense to the cement plant. Another advantage is due to the occurrence of the lime in the slag as oxide, and not as carbonate. The heat necessary to drive off the carbon dioxide from an equivalent mass of limestone is therefore saved when slag forms part of the cement mixture, and very low fuel consumption is obtained when slag-limestone mixture is burned.

Of the disadvantages, the toughness of the slag and the necessity for drying it before grinding are probably the most important. These serve to partly counterbalance the advantages noted above. A third difficulty, which is not always apparent at first, is that of securing a proper supply of suitable slag. Unless the cement plant is closely connected in ownership with the furnaces from which its slag supply is to be obtained, this difficulty may become very serious. In a season when a good iron market exists the furnace manager will naturally give little thought to the question of supplying slag to an independent cement plant.

The advantages of the mixture, however, seem to outweigh its disadvantages, for the manufacture of Portland cement from slag is now a large and growing industry in both Europe and America. Two Portland cement plants using slag and limestone as raw materials have been established for some time in this country, several others are in course of construction at present, and it seems probable that in the near future Alabama will join Illinois and Pennsylvania as an important producer of Portland cement from slag.



**GRINDING AND MIXING—WET METHODS.**—Wet methods of preparing Portland cement mixtures date back to the time when millstones and similar crude grinding contrivances were in use. With such imperfect machinery it was almost impossible to grind dry materials fine enough to give a good Portland cement mixture. The advent of good grinding machinery has practically driven out wet methods of manufacture in this country, except in dealing with materials such as marls, which naturally carry a large percentage of water. One or two plants in the United States do, it is true, deliberately add water to a limestone-clay mixture; but the effect of this practice on the cost sheets of these remarkable plants is not encouraging.

In preparing cement mixtures from marl and clay, a few plants dry both materials before mixing. It seems probable that this practice will spread, for the wet method of mixture is inherently expensive. At present, however, almost all marl plants use wet methods of mixing, and it is therefore necessary to give some space to a discussion of such methods.

Certain points regarding the location, physical condition, and chemical composition of the marls and clays used in such mixtures have important effects upon the cost of the wet process. As regards location, considered on a large scale, it must be borne in mind that marl deposits of workable size occur only in the Northern States and in Canada. In consequence the climate is unfavorable to continuous working throughout the year, for the marl is usually covered with water, and in winter it is difficult to secure the material. In a minor sense location is still an important factor, for marl deposits necessarily and invariably are found in depressions; and the mill must, therefore, just as necessarily, be located at a higher level than its source of raw material, which involves increased expense in transporting the raw material to the mill.

Glacial clays, which are usually employed in connection with marl, commonly carry a much larger proportion of sand and pebbles than do the sedimentary clays of more southern regions.

The effect of the water carried by the marl has been noted on an earlier page. The material as excavated will consist approximately of equal weights of lime carbonate and of water. This on the face of it would seem to be bad enough as a business proposition; but we find that in practice more water is often added to permit the marl to be pumped up to the mill.

On the arrival of the raw materials at the mill the clay is often dried, in order to simplify the calculation of the mixture. The reduction of the clay is commonly accomplished in a disintegrator or in edge-runner mills, after which the material is further reduced in a pug mill, sufficient water being here added to enable it to be pumped readily. It is then ready for mixture with the marl, which at some point in its course has been screened to remove stones, wood, etc., so far as possible. The slurry is further ground in pug mills or wet grinding mills of the disk type; while the final reduction takes place commonly in wet tube mills. The slurry, now containing 30 to 40 per cent. of solid matter and 70 to 60 per cent. of water, is pumped into storage tanks, where it is kept in constant agitation to avoid settling. Analyses of the slurry are taken at this point, and the mixture in the tanks is corrected if found to be of unsatisfactory composition. After standardizing, the slurry is pumped into the rotary kilns. Owing to the large percentage of water contained in the slurry the fuel consumption per barrel of finished cement is 30 to 50 per cent. greater, and the output of each kiln correspondingly less than in the case of a dry mixture.

It may be of interest, for comparison with the above description of the wet process with rotary kilns, to insert a description of the semi-wet process as carried on a few years ago at the dome kiln plant of the Empire Portland Cement Company at Warners, N. Y. The plant has been remodeled since that date, but the processes formerly followed are still of interest, as they resulted in a high-grade though expensive product.

At the Empire plant the marl and clay were obtained from a swamp about three-fourths of a mile from the mill. A revolving derrick with clam-shell bucket was employed for excavating the marl, while the clay was dug with shovels. The materials were taken to the works over a private narrow-gauge road, on cars, carrying about three tons each, drawn by a small locomotive. At the mill the cars were hauled up an inclined track, by means of a cable and drum, to the mixing floor.

The clay was dried in three Cummey "Salamander" driers, after which it was allowed to cool, and then carried to the mills. These mills were of the Sturtevant "rock emery" type, and reduced the clay to a fine powder, in which condition it was fed, after being weighed, to the mixer. The marl was weighed and sent directly to the mixer, no preliminary treatment being neces-

sary. The average charge was about 25 per cent. clay and about 75 per cent. marl.

The mixing was carried on in a mixing pan 12 feet in diameter, in which two large rolls, each about 5 feet in diameter, and 16-inch face, ground and mixed the materials thoroughly. The mixture was then sampled and analyzed, after which it was carried by a belt conveyor to two pug mills, where the mixing was completed and the slurry formed into slabs about 3 feet long and 4 to 5 inches in width and height. These on issuing from the pug mill were cut into a number of sections, so as to give bricks about 6 inches by 4 inches by 4 inches in size. The bricks were then placed on slats, which were loaded on rack cars and run into the drying tunnels. The tunnels were heated by waste gases from the kilns and required from twenty-four to thirty-six hours to dry the bricks.

After drying the bricks were fed into dome kilns, twenty of which were in use, being charged with alternate layers of coke and slurry bricks. The coke charge for a kiln was about four or five tons, and this produced 20 to 26 tons of clinker at each burning, thus giving a fuel consumption of about 20 per cent. as compared with the 40 per cent. or so required in the rotary kilns using wet materials. From thirty-six to forty hours were required for burning the charge. After cooling, the clinker was shoveled out, picked over by hand, and reduced in a Blake crusher, Smidth ball mills, and Davidsen tube mills.

*Composition of mixture.*—The cement mixture ready for burning will commonly contain from 74 to 77.5 per cent. of lime carbonate, or an equivalent proportion of lime oxide. Several analyses of actual cement mixtures are given in the following table. Analysis No. 1, with its relatively high percentage of magnesia, is fairly typical of Lehigh Valley practice. Analyses Nos. 2 and 3 show mixtures low in lime, while analysis No. 4 is probably the best proportioned of the four, especially in regard to the ratio between silica and alumina plus iron. This ratio, for ordinary purposes, should be about 3-, as the cement becomes quicker setting and lower in ultimate strength as the percentage of alumina increases. If the alumina percentage be carried too high, moreover, the mixture will give a fusible, sticky clinker when burned, causing trouble in the kilns.

*Analyses of cement mixtures.*

	1	2	3	4
Silica .....	12.62	13.46	13.85	11.77
Alumina and iron oxide .....	6.00	?	7.20	4.35
Carbonate of lime .....	75.46	73.66	73.93	76.84
Magnesia .....	2.65	?	?	1.74

**BURNING THE MIXTURE.**

After the cement mixture has been carefully prepared, as described in preceding pages, it must be burned with equal care.

In the early days of the Portland cement industry a simple vertical kiln, much like that used for burning lime and natural cement, was used for burning the Portland cement mixture. These kilns, while fairly efficient so far as fuel consumption was concerned, were expensive in labor, and their daily output was small. In France and Germany they were soon supplanted by improved types, but still stationary and vertical, which gave very much lower fuel consumption. In America, however, where labor is expensive while fuel is comparatively cheap, an entirely different style of kiln has been evolved. This is the rotary kiln. With the exception of a very few of the older plants, which have retained vertical kilns, all American Portland cement plants are now equipped with rotary kilns.

The history of the gradual evolution of the rotary kiln is of great interest, but as the subject can not be taken up here, reference should be made to the papers cited below\* in which details, accompanied often by illustrations of early types of rotary kilns are given.

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\*Duryee, E., The first manufacture of Portland cement by the direct rotary kiln process. *Engineering News*, July 26, 1900.

Lesley, R. W., History of the Portland cement industry in the United States. 8 vo. pp. 146, Philadelphia, 1900.

Lewis, F. H., The American rotary kiln process for Portland cement, in *The Cement Industry*, pp. 188-199, New York, 1900.

Matthey, H., The invention of the new cement burning method. *Engineering and Mining Journal*, vol. 67, pp. 555, 705; 1899.

Stanger, W. H., and Blount, B., The rotary process of cement manufacture. *Proc. Institution Civil Engineers*, vol. 145, pp. 44-136; 1901.

Editorial, The influence of the rotary kiln on the development of Portland cement manufacture in America. *Engineering News*, May 3, 1900.

The design, construction and operation of the vertical stationary kilns of various types is discussed in many reports in Portland cement, the most satisfactory single paper being probably that referred to below\*. As the subject is, in America at least, a matter of simply historical interest, no description of these kilns or their operation will be given in the present bulletin.

At present, practice in burning at the different American cement plants is rapidly approaching uniformity, though differences in materials, etc., will always prevent absolute uniformity from being reached. The kiln in which the material is burned is now almost invariably of the rotary type, the rotary process, which is essentially American in its development, being based upon the substitution of machines for hand labor wherever possible. A brief summary of the process will first be given, after which certain subjects of interest will be taken up in more detail.

*Summary of burning process.*—As at present used, the rotary kiln is a steel cylinder about 6 feet in diameter; its length, for dry materials, is usually 60 or 80 feet, while for wet mixtures an 80-foot, or even longer, kiln is frequently employed.

This cylinder is set in a slightly inclined position, the inclination being approximately one-half inch to the foot. The kiln is lined, except near the upper end, with very resistant fire brick, to withstand both the high temperature to which its inner surface is subjected and also the destructive action of the molten clinker.

The cement mixture is fed in at the upper end of the kiln, while fuel (which may be either powdered coal, oil, or gas), is injected at its lower end. The kiln, which rests upon geared bearings, is slowly revolved about its axis. This revolution, in connection with the inclination at which the cylinder is set, gradually carries the cement mixture to the lower end of the kiln. In the course of this journey the intense heat generated by the burning fuel first drives off the water and carbon dioxide from the mixture, and then causes the lime, silica, alumina, and

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\*Stanger, W. H., and Blount, B., Gilbert, W., and Candlot, E., (Discussion of the value, design and results obtained from various types of fixed kilns). Proc. Institution Civil Engineers, vol. 145, pp. 44, 48, 81, 82, 99, 100; 1901.

iron to combine chemically to form the partially fused mass known as "cement clinker." This clinker drops out of the lower end of the kiln, is cooled so as to prevent injury to the grinding machinery, and is then sent to the grinding mills.

*Theoretical fuel requirements.*—As a preliminary to a discussion of actual practice in the matter of fuel, it will be of interest to determine the heat units and fuel theoretically required in the manufacture of Portland cement from a dry mixture of normal composition.

In burning such a mixture to a clinker, practically all of the heat consumed in the operation will be that required for the dissociation of the lime carbonate present into lime oxide and carbon dioxide. Driving off the water of combination that is chemically held by the clay or shale, and decomposing any calcium sulphate (gypsum) that may be present in the raw materials, will require a small additional amount of heat. The amount required for these purposes is not accurately known, however, but is probably so small that it will be more or less entirely offset by the heat which will be liberated during the combination of the lime with the silica and alumina. We may, therefore, without sensible error, regard the total heat theoretically required for the production of a barrel of Portland cement as being that which is necessary for the dissociation of 450 pounds of lime carbonate. With coal of a thermal value of 13,500 B. T. U., burned with only the air supply demanded by theory, this dissociation will require  $25\frac{1}{2}$  pounds of coal per barrel of cement, a fuel consumption of only 6.6 per cent.

*Losses of heat in practice.*—In practice with the rotary kiln, however, there are a number of distinct sources of loss of heat, which result in a fuel consumption immensely greater than the theoretical requirements given above. The more important of these sources of loss are the following:

1. The kiln gases are discharged at a temperature much above that of the atmosphere, ranging from 300°F. to 2,000°F., according to the type of materials used and the length of the kiln.
2. The clinker is discharged at a temperature varying from 300°F. to 2,500°F., the range depending, as before, on materials and length of the kiln.

3. The air supply injected into the kiln is always greater, and usually very much greater, than that required for the perfect combustion of the fuel; and the available heating power of the fuel is thereby reduced.

4. Heat is lost by radiation from the ends and exposed surfaces of the kiln.

5. The mixture, in plants using a wet process, carries a high percentage of water, which must be driven off.

It is evident, therefore, that present-day working conditions serve to increase greatly the amount of fuel actually necessary for the production of a barrel of cement above that required by theory.

*Actual fuel requirements and output.*—Rotary kilns are nominally rated at a production of 200 barrels per day per kiln. Even on dry and easily clinkered materials and with good coal, however, such an output is not commonly attained with a 60-foot kiln, except in the Lehigh district. Normally a kiln working on a dry mixture will produce from 160 to 180 barrels of cement per day of twenty-four hours. In doing this, if good coal is used, its fuel consumption will commonly be from 120 to 140 pounds of coal per barrel of cement, though it may range as high as 160 pounds, and, on the other hand, has fallen as low as 90 pounds. An output of 175 barrels per day, with a coal consumption of 130 pounds per barrel, may therefore be considered as representing the results of fairly good practice on dry materials with a 60-foot kiln. In dealing with a wet mixture, which may carry anywhere from 30 to 70 per cent. of water, the results are more variable, though always worse than with dry materials. In working a 60-foot kiln on wet material, the output may range from 80 to 120 barrels per day, with a fuel consumption of from 150 to 230 pounds per barrel. Using a longer kiln, partly drying the mixture, and utilizing waste heat, will of course improve these figures materially.

When the heavy Western oils are used for kiln fuel, it may be considered that one gallon of oil is equivalent in the kiln to about ten pounds of coal. The fuel consumption, using dry materials, will range between 11 and 14 gallons of oil per barrel of cement; but the output per day is always somewhat less with oil fuel than where coal is used.

Natural gas in the kiln may be compared with good Pennsylvania coal by allowing about 20,000 to 30,000 cubic feet of gas

as equivalent to a ton of coal. This estimate is, however, based upon too little data to be as close as those above given for oil or coal.

*Effect of composition on burning.*—The differences in composition between Portland cement mixtures are very slight if compared, for example, to the differences between various natural cement rocks. But even such slight differences as do exist exercise a very appreciable effect on the burning of the mixture. Other things being equal, any increase in the percentage of lime in the mixture will necessitate a higher temperature in order to get an equally sound cement. A mixture which will give a cement carrying 59 per cent. of lime, for example, will require much less thorough burning than would a mixture designed to give a cement with 64 per cent. of lime.

With equal lime percentages, the cement carrying high silica and low alumina and iron will require a higher temperature than if it were lower in silica and higher in alumina and iron. But, on the other hand, if the alumina and iron are carried too high, the clinker will ball up in the kiln, forming sticky and unmanageable masses.

*Character of kiln coal.*—The fuel most commonly used in modern rotary kiln practice is bituminous coal, pulverized very finely. Coal for this purpose should be high in volatile matter, and as low in ash and sulphur as possible. Russell gives the following analyses of West Virginia and Pennsylvania coals used at present at various cement plants in Michigan.

*Analyses of kiln coals.*

	1	2	3	4
Fixed carbon ....	56.15	56.33	55.82	51.69
Volatile matter ....	35.41	35.26	39.37	39.52
Ash ....	6.36	7.06	3.81	6.13
Moisture ....	2.08	1.35	1.00	1.40
Sulphur ....	1.30	1.34	0.42	1.46

The coal as usually bought is either "slack" or "run of mine." In the latter case it is necessary to crush the lumps before proceeding further with the preparation of the coal, but with slack this preliminary crushing is not necessary, and the material can go directly to the dryer.



*Drying coal.*—Coal as bought may carry as high as 15 per cent. of water in winter or wet season. Usually it will run from 3 to 8 per cent. To secure good results from the crushing machinery it is necessary that this water should be driven off. For coal drying, as for the drying of raw materials, the rotary dryer seems best adapted to American conditions. It should be said, however, that in drying coal it is usually considered inadvisable to allow the products of combustion to pass through the cylinder in which the coal is being dried. This restriction serves to decrease slightly the possible economy of the dryer, but an evaporation of 6 to 8 pounds of water per pound of fuel coal can still be counted on with any good dryer. The fuel cost of drying coal containing 8 per cent. of moisture, allowing \$2 per ton for the coal used as fuel, will therefore be about 3 to 4 cents per ton of dried product.

*Pulverizing coal.*—Though apparently brittle enough when in large lumps, coal is a difficult material to pulverize finely. For cement kiln use, the fineness of reduction is very variable. The finer the coal is pulverized the better results will be obtained from it in the kiln; and the poorer the quality of the coal the finer it is necessary to pulverize it. The fineness attained may therefore vary from 85 per cent. through a 100-mesh sieve, to 95 per cent. or more, through the same. At one plant a very poor but cheap coal is pulverized to pass 98 per cent. through a 100-mesh sieve, and in consequence gives very good results in the kiln.

Coal pulverizing is usually carried on in two stages, the material being first crushed to 20 to 30 mesh in a Williams mill or ball mill, and finally reduced in a tube mill. At many plants, however, the entire reduction takes place in one stage, Griffin or Huntington mills being used.

*Total cost of coal production.*—The total cost of crushing (if necessary), drying and pulverizing coal, and of conveying and feeding the product to the kiln, together with fair allowance for replacements and repairs, and for interest on the plant, will probably range from about 20 to 30 cents per ton of dried coal, for a 4-kiln plant. This will be equivalent to a cost of from 3 to 5 cents per barrel of cement. While this may seem a heavy addition to the cost of cement manufacture, it should be remembered that careful drying and fine pulverizing enable the manu-

facturer to use much poorer—and therefore cheaper—grades of coal than could otherwise be utilized.

#### CLINKER GRINDING. GYPSUM.

*Clinker grinding.*—The power and machinery required for pulverizing the clinker at a Portland cement plant using the dry process of manufacture is very closely the same as that required for pulverizing the raw materials for the same output. This may seem, at first sight, improbable, for Portland cement clinker is much harder to grind than any possible combination of raw materials; but it must be remembered that for every barrel of cement produced about 600 pounds of raw materials must be pulverized, while only a scant 400 pounds of clinker will be treated, and that the large crushers required for some raw materials can be dispensed with in crushing clinker. With this exception, the raw material side and the clinker side of a dry-process Portland cement plant are usually almost or exactly duplicates.

The difficulty, and in consequence the expense, of grinding clinker will depend in large part on the chemical composition of the clinker and on the temperature at which it has been burned. The difficulty of grinding, for example, increases with the percentage of lime carried by the clinker; and a clinker containing 64 per cent. of lime will be very noticeably more resistant to pulverizing than one carrying 62 per cent. of lime. So far as regards burning, it may be said in general, that the more thoroughly burned the clinker the more difficult it will be to grind, assuming that its chemical composition remains the same.

The tendency among engineers at present is to demand more finely ground cement. While this demand is doubtless justified by the results of comparative tests of finely and coarsely ground cements, it must be borne in mind that any increase in fineness of grinding means a decrease in the product per hour of the grinding mills employed, and a consequent increase in the cost of cement. At some point in the process, therefore, the gain in strength due to fineness of grinding will be counterbalanced by the increased cost of manufacturing the more finely ground product.

The increase in the required fineness has been gradual but steady during recent years. Most specifications now require at

least 90 per cent. to pass a 100-mesh sieve; a number require 92 per cent.; while a few important specifications require 95 per cent. Within a few years it is probable that almost all specifications will go as high as this.

*Addition of gypsum.*—The cement produced by the rotary kiln is invariably naturally so quick-setting as to require the addition of sulphate of lime. This substance, when added in quantities up to  $2\frac{1}{2}$  or 3 per cent., retards the rate of set of the cement proportionately, and appears to exert no injurious influence on the strength of the cement. In amount over 3 per cent., however, its retarding influence seems to become at least doubtful, while a decided weakening of the cement is noticeable.

Sulphate of lime may be added in one of two forms: either as crude gypsum or as burned plaster. Crude gypsum is a natural hydrous lime sulphate, containing about 80 per cent. of lime sulphate and 20 per cent. of water. When gypsum is calcined at temperatures not exceeding  $400^{\circ}\text{F.}$ , most of its contained water is driven off. The "plaster" remaining carries about 93 per cent. of lime sulphate, with only 7 per cent. of water.

In Portland cement manufacture either gypsum or burned plaster may be used to retard the set of the cement. As a matter of fact, gypsum is the form almost universally employed in the United States. This is merely a question of cost. It is true that to secure the same amount of retardation of set it will be necessary to add a little more of gypsum than if burned plaster were used; but, on the other hand, gypsum is much cheaper than burned plaster.

The addition of the gypsum to the clinker is usually made before it has passed into the ball mill, komminuter, or whatever mill is in use for preliminary grinding. Adding it at this point secures much more thorough mixing and pulverizing than if the mixture were made later in the process. At some of the few plants which use plaster instead of gypsum, the finely ground plaster is not added until the clinker has received the final grinding and is ready for storage or packing.

#### CONSTITUTION OF PORTLAND CEMENT.

During recent years much attention has been paid by various investigators to the constitution of Portland cement. The chemical composition of any particular sample can, of course, be

readily determined by analysis; and by comparison of a number of such analyses, general statements can be framed as to the range in composition of good Portland cements.

The chemical analyses will determine what ingredients are present, and in what percentages, but other methods of investigation are necessary to ascertain in what manner these various ingredients are combined. A summary of the more important results brought out by these investigations on the constitution of Portland cement is here given.

It would seem to be firmly established that, in a well-burned Portland cement, much of the lime is combined with most of the silica to form the compound  $3 \text{ CaO}, \text{SiO}_2$ ,—tricalcic silicate. To this compound is ascribed, in large measure, the hydraulic properties of the cement; and in general it may be said that the value of a Portland cement increases directly as the proportion of  $3 \text{ CaO}, \text{SiO}_2$ . The ideal Portland cement, toward which cements as actually made tend in composition, would consist exclusively of tricalcic silicate, and would be therefore composed entirely of lime and silica in the following proportions:

Lime ( $\text{CaO}$ ) .....	73.6
Silica ( $\text{SiO}_2$ ) .....	26.4

Such an ideal cement, however, can not be manufactured under present commercial conditions, for the heat required to clinker such a mixture can not be attained in any working kiln. Newberry has prepared such mixtures by using the oxy-hydrogen blowpipe; and the electrical furnace will also give clinker of this composition; but a pure lime-silica Portland is not possible under present-day conditions.

In order to prepare Portland cement in actual practice, therefore, it is necessary that some other ingredient or ingredients should be present to serve as a flux in aiding the combination of the lime and silica, and such aid is afforded by the presence of alumina and iron oxide.

Alumina ( $\text{Al}_2\text{O}_3$ ) and iron oxide ( $\text{Fe}_2\text{O}_3$ ), when present in noticeable percentages, serve to reduce the temperature at which combination of the lime and silica (to form  $3 \text{ CaO}, \text{SiO}_2$ ) takes place; and this clinkering temperature becomes further and further lowered as the percentages of alumina and iron are increased. The strength and value of the product, however, also decrease as the alumina and iron increase; so that in actual

practice it is necessary to strike a balance between the advantage of low clinkering temperature and the disadvantage of weak cement, and to thus determine how much alumina and iron should be used in the mixture.

It is generally considered that whatever alumina is present in the cement is combined with part of the lime to form the compound  $2 \text{ CaA}, \text{ SiO}_2$ ,—dicalcic aluminate. It is also held by some, but this fact is somewhat less firmly established than the last, that the iron present is combined with the lime to form the compound  $2 \text{ CaO}, \text{ Fe}_2\text{O}_3$ . For the purposes of the present paper, it will be sufficient to say that, in the relatively small percentages in which iron occurs in Portland cement, it may for convenience be considered as almost equivalent to alumina and its action, and the two may be calculated together.



## PART II.

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### THE CEMENT RESOURCES OF ALABAMA.

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BY EUGENE A. SMITH.

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In Alabama is found an extensive series of limestones capable of furnishing excellent raw material for the manufacture of Portland cement, while the shales and clays necessary to complete the mixture are found in every county in the State. As a matter of convenience, the Portland cement materials of northern Alabama and of central and southern Alabama will be discussed separately, because there is a marked geologic as well as geographic distinction between the two portions of the State.

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### CHAPTER 1.

#### THE PORTLAND CEMENT MATERIALS OF NORTHERN ALABAMA.

The raw materials for the manufacture of Portland cement occurring in the Paleozoic formations of northern Alabama are limestones, shales, and clays. Of these the limestones belong mainly to the Lower Carboniferous and the Trenton formations; the shales to the Coal Measures, and the clays to the Cambrian, Lower Carboniferous, and Coal Measures. Although as yet these materials have not been utilized for this purpose in Alabama, they have been so used in other States, and there is no reason to doubt that the future will witness their utilization in Alabama.

## AVAILABLE LIMESTONES.

*General geology.*—In northern Alabama the combined effects of geologic structure and erosion have resulted in certain definite topographic types, with which the geologic outcrops are closely connected.

Structurally northern Alabama is made up of a series of parallel synclines and anticlines, trending usually a little north of east. The anticlines are sharp, narrow folds; the synclines are flat, wide basins. The effect of erosion has been to cut away the anticlines and the streams of the region now run along anticlinal valleys bordered by flat-topped synclinal plateaus.

The plateaus throughout most of northern Alabama are capped by conglomerates, shales, and sandstones of the Coal Measures. The lower Carboniferous limestones commonly outcrop along the sides and at the immediate base of the plateaus. The lower Silurian beds occur as long, narrow outcrops in the valleys. The middle of the valley is usually occupied by Cambrian shales and the Knox dolomite. The Trenton limestones would normally outcrop as two parallel bands in each valley—between the middle of the valley and the foothills of the plateaus. Faulting has, however, been so common that only one of these bands is usually present, the other being cut out by a fault.

*Lower Carboniferous.*—Limestones of suitable quality for cement manufacture occur in the Mountain limestone or Chester formation of the lower Carboniferous. Perhaps the most accessible occurrences of this rock are in the Tennessee Valley to the west of Tusculumbia and south of the river and railroad. Here the quarries of Fossick & Co. were formerly located. Their quarries at this time are farther eastward, but at a greater distance from the river, in Lawrence county north of Russellville. This outcrop extends thence eastward along the base of Little Mountain as far as Whitesburg, above which place to Guntersville the river flows through a valley floored with lower Carboniferous limestone. The Southern Railway passes over outcrops of this rock in most of the mountain coves east of Huntsville, and from Scottsboro to the Tennessee line the country rock is almost entirely of this formation. The Louisville and Nashville Railroad south of Decatur nearly to Wilhite is mostly in the same formation. These two lines, together



with the Tennessee river, would provide ample means of transportation for the rock or for the finished product. Analysis of the rock from the Fossick quarries is given in Table A.

In Browns Valley south of Brooksville the Mountain limestone is the prevailing rock across the valley, and at Bangor and Blount Springs, on the Louisville and Nashville Railroad, there are extensive quarries which have been worked for many years to supply rock for fluxing purposes to the furnaces of the Birmingham district. Analyses Nos. 2, 3, 4, 5, 6, 7, 8, and 9, Table A, show the composition of average samples from these quarries; 5 to 9, inclusive, are of carload samples.

From Brooksville to the Tennessee line a great thickness of this limestone is exposed along the western escarpment of Sand Mountain, below the sandstones of the Coal Measures, which there cap the mountain. In this area the river runs near the foot of the mountain and would afford the means of transportation.

In similar manner the lower Carboniferous limestone outcrops along the western flank of Lookout Mountain in Little Wills Valley, from near Attalla to the Georgia line, and south of Attalla it forms the lower part of the escarpments of Blount and Chandlers Mountains. The Alabama Great Southern Railroad passes very near to the outcrop from the Georgia line down to Springville, Ala. South of Springville large outcrops occur in Shades Valley, and at Trussville are quarries which have supplied the Birmingham furnaces. Analyses 10 to 17, inclusive, Table A, are of material from Trussville; and analyses 12 to 17, inclusive, represent average samples from carload lots delivered to the furnace.

In Murphrees Valley the main outcrop of this rock is on the western side, and quarries at Compton have for many years been worked to supply the Birmingham furnaces. Analyses 18, 19, and 20, Table A, of the rock from these quarries show somewhat varying composition, but by proper selection suitable material could be easily obtained.

In the valleys lying east of Shades Valley and in parts of Shades Valley itself this formation becomes one of prevailing shales and sandstones and the limestones are of limited occurrence and of inferior quality.

*Trenton limestone.*—The Trenton limestone outcrops in Alabama in three principle areas. In the Tennessee River Valley

some of the smaller streams which flow into the river from the north, like Flint River, Limestone Creek, Elk River, Bluewater Creek, and Shoal Creek, have eroded their valleys into the Trenton limestone. These areas are crossed at only a few points by the railroads leading out from Huntsville and Florence, and no commercial use has yet been made of the rock.

In the narrow anticlinal valleys below enumerated erosion has in most cases sunk the floors of the valleys into Cambrian strata, and, as a consequence, the Trenton limestone occupies a narrow belt on each side, near the base of the Red Mountain ridges. But since a fault usually occurs on one side of these valleys, the Red Mountain ridges and the accompanying Trenton limestone are more fully represented on the unfaulted side, which is the eastern side in all except Murphrees Valley. While the Trenton forms practically a continuous belt along the undisturbed side, extensive areas are sometimes found on the faulted side also. This is the case, for instance, at Vance, on the Alabama Great Southern Railroad, where the rock is quarried for flux for the furnace of the Central Iron Company at Tuscaloosa. Analysis 1 of Table B, shows its composition here. Other series of analyses from lower ledges in the quarry show only 1.22 per cent of silica, but more magnesia.

In cases where erosion has not gone so deep as to reach the Cambrian the Trenton may be found extending entirely across the valleys. This is the case in the lower part of Browns Valley from Brooksville to beyond Guntersville. Above Guntersville the Trenton is seen mainly on the eastern side of the valley. The river touches these outcrops at many points, and at Guntersville the railroad connecting that city with Attalla would afford an additional means of transportation. No developments have yet been made in this area.

The valley separating the Warrior from the Cahaba coal field is known as Rouns Valley in the southern and as Jones Valley in the northern part. In these the Trenton occupies a narrow, continuous belt, usually near the base of the eastern Red Mountain ridge, though in places it is high up on the ridge and even at its summit, as at Gate City, where the quarries of the Sloss Iron Company are located. Many analyses of the rock from these quarries have been made, and several are given in Table B, (Nos, 2, 3, 4, 5, 6).

In Murphrees Valley the continuous belt of the Trenton, as above explained, is on the western side, while the faulted remnants are on the eastern side. No quarries have been opened in the Trenton limestone here, but the Louisville and Nashville Railroad goes up the valley as far as Oneonta and would afford means of transportation.

In the Cahaba Valley, which separates the Cahaba coal field from the Coosa coal field, the Trenton is well exposed on the eastern side for the entire length of the valley from Gadsden down. It expands into wide areas near the southern end, where it has been quarried for lime burning, at Pelham, Siluria, Longview, Calera, and other places on the line of the Louisville and Nashville road. Analyses 7, 8 and 9 of Table B, show the composition of the rock in this region.

The Central of Georgia and the Southern railroads cross this belt about midway of its length at Leeds, in Jefferson County, and near its northern end it is crossed by the Louisville and Nashville Railroad, where a quarry at Rock Springs, on the flank of Colvin Mountain, supplies the rock for lime burning. Analysis 10 shows the character of the rock at this point.

At Pratts Ferry, on the Cahaba River, a few miles above Centreville, in Bibb County, the Trenton limestone makes high bluffs along the river for several miles, and is in most convenient position for easy quarrying.

Marble works have in former days been established here and should be again put in operation, since the marble is of fine quality and beautifully variegated. No analyses are available, but there is no doubt that much of the rock is sufficiently low in magnesia to be fit for use in cement making. Cahaba River and a short spur from the Mobile and Ohio Railroad would afford transportation facilities for this deposit.

In Big Wills Valley, which separates Sand and Lookout mountains, the Trenton limestone occupies perhaps 25 square miles, but it is crossed only by the railroad connecting Gadsden with Guntersville. No analyses are available.

In the great Coosa Valley region the Trenton outcrops are found mostly on the western border near the base of Lookout Mountain, as in Broomtown Valley, and in other valleys extending south toward Gadsden. While these belts have been utilized in the past for the old Gaylesville, Cornwall, and Round

Mountain furnaces, and possibly for some furnaces now in blast, no analyses are available.

Similarly, farther south, along this western border of the Coosa Valley, and running parallel with the Coosa coal field in Calhoun, St. Clair, and Shelby counties, there are numerous long narrow outcrops of Trenton limestone. The Calcis quarry of the Tennessee Coal, Iron and Railroad Company, on the Central of Georgia Railroad, near Sterritt, is upon one of these outcrops, and furnishes limestone with a very low and uniform percentage of silica and magnesia. Analyses 11, 12, 13, 14, 15, and 16 exhibit the quality of the rock as received at the Ensley Steel Works, but care is taken at the quarry to select ledges low in silica and magnesia, and the analyses therefore represent only the selected ledges and not the average run of the quarry as a whole.

Near Talladega Springs, Marble Valley, and Shelby are other occurrences of the rock, and a quarry a few miles east of Shelby furnace has for many years supplied that furnace with its flux. The quality of the material here is shown by analyses 17, 18, 19, and 20, Table B.

The Cambrian limestones contain generally a very considerable proportion of magnesia, and for this reason are not suited for Portland-cement manufacture, though admirably adapted for furnace stone.

*Marbles.*—Along the eastern border of the Coosa Valley, near its contact with the metamorphic rocks, there is a belt of limestone which, in places, is a white crystalline marble of great purity, as is shown by analyses 1 to 7, inclusive, of Table C. The Louisville and Nashville Railroad, from Calera to Talladega, passes close to this belt at many points. This marble has been quarried at several places for ornamental stone. It is mentioned here because it is near the railroad and completes the account of the limestone.

#### THE CLAYS.

The most important clays in the Paleozoic region occur in the Coal Measures, in the Lower Carboniferous, and in the Lower Silurian and Cambrian formations. But, inasmuch as a later formation—the Tuscaloosa of the Cretaceous—borders the Paleozoic on the west and south, and as it contains a great vari-

ety as well as abundance of clays, we shall include it here, although it is not one of the Paleozoics.

*Coal Measures.*—In this group are numerous beds of shale which have been utilized in the manufacture of vitrified brick and fire brick, but many of them will probably be adapted to cement making. A great body of these shales occurs in connection with the coal seams of the Horse Creek or Mary Lee group, in Jefferson and Walker counties, and in position where they are conveniently situated with reference to limestone and coal and also to transportation lines. They are therefore well worth the attention of those contemplating the location of cement plants.

On the property of Mr. W. H. Graves, near North Birmingham, overlying the coal seam mined by him, there are two beds of shale—one yellowish, the other gray. These two shales have been tested and analyzed, and their composition is shown in Nos. 1 and 2 of the Table D.

Similar shales are known to occur at Coaldale, in Jefferson County, at Pearce's Mills in Marion, and at Cedar Grove Coal Mines in Tuscaloosa. The Coaldale shale is manufactured into vitrified brick. The other two have not yet been utilized.

Analyses 3 and 4 of Table D will show the composition of the shales at Coaldale and Cedar Grove.

It may be of interest to note that Cedar Grove is, so far as yet known, the nearest place to the Gulf ports, where the three essentials in the manufacture of Portland cement, viz., limestone, shale and coal, occur together, and on a railroad.

So also most of the coal seams mined in Alabama rest upon clay beds which have not as yet been specially examined as to their fitness for cement making; but, in view of the proximity of the coal mines to the limestones, it might be worth while to investigate these underclays of the coal seams.

*Lower Carboniferous.*—Associated with the cherty limestones of the lowermost division of the Lower Carboniferous of some of the anticlinal valleys are beds of clay of excellent quality, much of it being of the nature of china clay.

Probably the best of the exposures of these clays are to be seen in Little Wills Valley, between Fort Payne and the Georgia border, and on the line of the Great Southern Railroad, where for many years quarries have been in operation in sup-

plying the material for tile works and potteries. The clays lie near the base of the formation close above the black shale of the Devonian, and average about 40 feet in thickness, though in places they reach 200 feet. The clay beds alternate with seams of chert which are from 2 to 8 inches in thickness, while the clay beds vary from 12 to 18 inches. The upper half of the clay is more gritty than the lower half which often contains material suitable for the manufacture of the finer grades of porcelain ware. Analyses 5 to 8, in Table D, show the composition of several varieties of clay from this section.

*Lower Silurian and Cambrian.*—Associated with the cherty limestones and brown iron-ore beds of the formations above named—beds of fine white clay, much of it china clay—are not uncommon. Analysis 9 of the table shows the composition of a white clay from the brown ore bank at Rock Run, in Cherokee County, where the clay is about 30 feet in thickness. Analyses 10 and 11 are also from Rock Run. No. 12, from near Gadsden, No. 13, from Blount County, and No. 14 from Oxanna, in Calhoun County, are clays which seem to be adapted to cement making. While no great number of the clays of these formations have been analyzed, they are known to be widely distributed in Calhoun, Talladega, Jefferson, Tuscaloosa, and other counties in connection with the brown ore deposits.

*Cretaceous.*—In many respects the most important formation of Alabama, in respect of its clays, is the lowermost division of the Cretaceous, which has been called the Tuscaloosa, and which is in part at least of the same geologic horizon as that of the Raritan clays of New Jersey. The prevailing strata of this formation are yellowish and grayish sands, but subordinated to them are great lenses of massive clay varying in quality from almost pure-white burning clay to dark-purple and mottled varieties high in iron.

The formation occupies a belt of country extending from the north-western corner of the State, around the edges of the Paleozoic formations to the Georgia line at Columbus. Its greatest width is at the northwest boundary of the State where it covers an area 30 or 40 miles wide in Alabama, and of about the same width in Mississippi. The breadth at Wetumpka and thence eastward to the Georgia line is only a few miles. The most important part of this belt is where it is widest in Elmore,

Bibb, Tuscaloosa, Pickens, Fayette, Marion, Lamar, Franklin, and Colbert counties, and the deposits are traversed by the lines of the Mobile and Ohio; the Alabama Great Southern; the Louisville and Nashville; the Southern; and the Kansas City, Memphis and Birmingham railroads; as well as by the Warrior and Tombigbee rivers.

These clays have been described in some detail, and many analyses and physical tests have been presented in the Bulletin No. 6 of the Alabama Geological Survey. From this bulletin have been selected the analyses which appear to indicate the fitness of the clays for cement making.

In Elmore county, in the vicinity of Coosada, along the banks of the river, about Robinson Springs, Edgewood, and Chalk Bluff, there are many occurrences of these clays, some of which have been used in potteries for many years. Analyses 15, from Coosada; 16, from Edgewood; and 17, from Chalk Bluff, are given in the table D.

In Bibb county the clay has been quarried very extensively at Bibbville and near Woodstock for making fire brick. For this purpose the material is carried to Bessemer by the Alabama Great Southern Railroad. No. 18, from Woodstock; and 19, from Bibbville, will represent the average quality of the clay from these beds, which are very extensive, both in thickness and in superficial distribution. The Mobile & Ohio crosses other extensive deposits in the southern part of the county, but no analyses are available.

The most important of the clay beds in Tuscaloosa county are traversed by the Mobile & Ohio Railroad and by the Alabama Great Southern.

Analysis 20, from Hull's; and analysis 21, from the Cribbs beds, are on the Alabama Great Southern; and 22 and 23 are from cuts of the Mobile & Ohio, a few miles west of the city of Tuscaloosa.

Many large beds are exposed along the Mobile & Ohio road in Pickens county also, but very few have been as yet investigated. No. 24 is from Roberts Mill, in this county.

In Lamar and Fayette counties the same conditions prevail as in Pickens and Tuscaloosa. Analysis 25 is of pottery clay from the Cribbs place, in Lamar; and No. 26 is of clay from Wiggins's, 4 miles west of Fayette; and 27 and 28 are clays from W. Doty's place, 14 miles west of that town, in Fayette county.

Marion is one of the banner counties of the State for fine clays, but it is touched by railroads only along its southern border and in the extreme northeastern corner. Although at present not available because inaccessible, the clays mentioned below are worth consideration: No. 29, from Glen Allen; No. 30, from Briggs Fredericks', in Sec. 8, T. 10, R. 13 W. This is from the great clay deposit which gives the name to Chalk Bluff and which underlies about two townships. No. 31 is from a locality about 16 miles southwest of Hamilton, the county seat.

No. 32 is from a locality near the Mississippi line, in section 20, T. 8, R. 15 W., in Franklin county, from land of Mr. Thomas Rollins.

Of the numerous fine clays of Colbert county analyses are given of two from Pegram station, on the Southern Railway, near the Mississippi State line. These are Nos. 33 and 34.



*Table A.*  
*Analyses of Lower Carboniferous Limestones.*

Number.	1	2	3	4	5	6	7	8	9	10
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .....	0.50	1.73	0.77	1.14	1.02	1.40	0.68	0.81	0.82	2.16
Iron and alumi- num oxide ....	1.45	.78	.35	.34	1.38	1.17	1.02	.89	.60	2.31
Calcium carb'te.	96.58	96.54	97.60	98.53	96.25	94.67	96.54	97.45	97.37	89.15
Magnesium carb't	2.58	.....	.....	.....	1.73	2.26	1.26	.35	.75	4.20
Sulphur .....	.....	.....	.....	.....	.....	.....	.....	.....	.029	.....

Number.	11	12	13	14	15	16	17	18	19	20
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .....	3.12	0.85	1.08	0.73	0.64	1.12	0.42	2.05	4.45	2.80
Iron and alumi- num oxide ....	2.32	.65	.61	.65	.62	.90	.37	.76	3.30	.70
Calcium carb'te.	85.87	96.64	96.91	97.60	97.48	96.38	97.32	89.64	86.35	94.59
Magnesium carb't	4.20	1.36	.90	.52	.76	1.10	1.39	8.15	.....	.....
Sulphur .....	.....	.024	.019	.018	.....	.....	.020	.....	.....	.....

1. Average sample from Fossick quarry, near Rockwood, Franklin County. Government Arsenal, Watertown, N. Y., analyst.

2. Average sample from Blount Springs quarry—a compact limestone. Henry McCalley, analyst.

3. Average sample from Blount Springs quarry—a granular oolitic limestone. Henry McCalley, analyst.

4. Average sample upper 75 feet, Blount Springs quarry. J. L. Beeson, analyst.

5-9. Average sample Blount Springs quarry. J. R. Harris, analyst.

10. 11. From Worthington quarry, near Trussville, Jefferson county. C. A. Meissner, analyst.

12-17. From Vanns, near Trussville. J. R. Harris, analyst.

18. Average of about 150 feet thickness of rock used for flux, Compton quarry, Blount county. J. L. Beeson, analyst.

19. 20. Stockhouse sample, Compton quarry. Wm. B. Phillips, analyst.

*Table B.*  
*Analyses of Trenton Limestones.*

Number.	1	2	3	4	5	6	7	8	9	10
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .....	4.48	5.70	2.43	3.65	3.29	3.82	0.39	0.15	0.78	1.00
Iron and alumi- num oxides ....	1.22	1.87	3.30	.91	1.49	1.96	.13	Tr	.35	.30
Calcium carb'te.	88.85	91.16	89.88	92.38	92.61	90.44	99.11	99.16	97.52	97.00
Magnesium carbt	3.52	.....	.....	.....	.....	.....	.75	.75	1.27	Tr
Sulphur .....	.....	.....	.....	.....	.....	.....	.....	.....	.....	Tr
Water, organic matter and loss	.....	.....	.....	.....	.....	.....	.....	.....	.....	.....

Number.	11	12	13	14	15	16	17	18	19	20
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .... ..	0.43	0.58	0.38	0.34	0.39	0.98	2.50	2.09	1.08	2.25
Iron and alumi- num oxides ....	.42	.25	.47	.46	.37	.52	1.40	1.01	.63	.68
Calcium carb'te.	98.49	95.78	98.35	96.53	94.27	96.92	96.70	93.77	98.91	95.40
Magnesium carbt	.16	2.89	.30	2.17	4.47	1.08	.....	2.48	.58	.94
Sulphur .... ..	.....	.....	.....	.....	.....	.....	.....	.....	.....	.....
Water, organic matter and loss	.....	.....	.....	.....	.....	.....	.....	.....	.....	.....

1. Average of several carloads flux rock from quarry at Vance, Tuscaloosa county, of Central Iron Company at Tuscaloosa. H. Buel, analyst.

2. Gate City quarry, Jefferson county. Average sample from the crusher. Henry McCalley, analyst.

3-6. Gate City quarry. J. W. Miller, analyst.

7, 8. Longview quarries, Shelby county. Used in lime burning. Report of Alabama State Geologist, 1875.

9. Jones quarry, near Longview. Report of Alabama State Geologist, 1875

10. Rock Spring quarry, Etowah county. Used in lime burning and for flux. Wm. B. Phillips, analyst.

11-16. Rock from Calcis quarry, St. Clair county. J. R. Harris, analyst.

17-20. Shelby quarry, Shelby county. Used for flux in Shelby furnaces. Report of Alabama State Geologist, 1875.

*Table C.*  
*Analyses of Crystalline Marbles.*

Number.	1	2	3	4	5	6	7
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .....	Tr	2.70	2.95	4.65	2.80	1.35	0.28
Iron and aluminum oxides.....		.40	1.15	.75	.48	.30	.28
Calcium carbonate .....	99.47	90.80	95.25	94.40	95.60	97.60	99.19
Magnesium carbonate .....	.38	Tr	.62	.41	.66	Tr	.14

1. Herd's upper quarry, Talladega county. Tuomey's Second Report.
2. Heard's quarry, sec. 16, T. 21, R. 4 E., Talladega county. Wm. B. Phillips, analyst.
3. Taylor's mill, Talladega county, white marble. Wm. C. Stubbs, analyst.
4. Taylor's mill, Talladega county, blue marble. Wm. C. Stubbs, analyst.
5. Taylor's mill, Talladega county. A. F. Brainerd, analyst.
6. Nix quarry, sec. 36, T. 20, R. 4 E., Talladega county, white marble. Wm. B. Phillips, analyst.
7. Gannt's quarry, sec. 2, T. 22, R. 3 E., Talladega county, white marble. A. F. Brainerd, analyst.

*Table D.*  
*Analyses of Clays—Paleozoic and Lower Cretaceous.*

Number.	1	2	3	4	5	6	7	8	9
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .....	61.55	57.80	57.22	58.50	79.80	82.04	66.25	82.11	60.50
Alumina .....	20.25	25.00	24.72	18.28	11.75	12.17	22.90	11.41	26.55
Ferric oxide .....	7.23	4.00	7.14	10.22	1.75	Tr	1.60	1.40	.30
Lime .....	Tr	2.10	.49	1.19	.75	Tr	Tr	Tr	.90
Magnesia .....	.99	.80	1.88	1.40	Tr	.33	Tr	.66	.65
Alkalies .....	1.25	1.80	.40	.70	1.50	.60	.75	1.80	2.70
Ignition .....	6.19	7.50	7.09		4.11	4.33	9.05	4.00	7.90
	98.66	99.00	98.93	.....	99.16	99.47	100.55	101.38	99.50

Number.	10	11	12	13	14	15	16	17	18
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .....	72.20	57.00	67.95	61.50	84.21	66.61	62.60	60.38	65.82
Alumina .....	22.04	17.80	20.15	26.20	9.75	21.04	26.98	20.21	24.58
Ferric oxide .....	.16	5.60	1.00	2.10	.69	2.88	.72	6.16	1.25
Lime .....	.50	2.10	1.00	.50	.70	.40	.40	.09	.....
Magnesia .....	.40	1.20	Tr	.43	.14	.58	.36	.72	Tr
Alkalies .....	.60	6.00	1.87	.70	.....	.70	.65	1.80	.60
Ignition .....	5.80	9.45	8.00	7.29	4.10	7.00	9.30	10.21	8.16
	101.70	99.15	99.97	98.72	99.59	99.21	101.01	99.57	100.41

Number.	19	20	21	22	23	24	25	26	27
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct
Silica .....	74.25	61.25	65.35	60.03	58.13	68.23	60.90	63.27	67.10
Alumina .....	17.25	25.60	21.30	24.66	24.68	20.35	18.98	19.68	19.37
Ferric oxide .....	1.19	2.10	2.72	3.69	3.85	3.20	7.68	3.52	2.88
Lime .....	.40	.25	.60	.13	.15	.34	Tr	1.30	Tr
Magnesia .....	Tr	.82	.86	.38	.32	Tr	Tr	Tr	.73
Alkalies .....	.52	1.35	Tr	Tr	1.78	.74	Tr	1.20	.67
Ignition .....	6.30	8.10	8.79	11.34	11.78	7.16	13.36	9.80	7.79
	99.39	99.47	99.62	100.23	100.51	100.02	100.92	98.77	98.54

Number.	28	29	30	31	32	33	34		
	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct	Pr ct		
Silica .....	65.58	68.10	65.49	70.00	67.50	66.45	64.90		
Alumina .....	19.23	21.89	24.54	21.31	19.84	18.53	25.25		
Ferric oxide .....	4.48	2.01	Tr	2.88	6.15	2.40	Tr		
Lime .....	Tr	.80	1.26	.20	.12	1.50	Tr		
Magnesia .....	Tr	.28	Tr	Tr	.10	1.25	Tr		
Alkalies .....	.....	.40	Tr	Tr	.....	Tr	.....		
Ignition .....	6.90	5.75	7.80	6.85	7.65	9.46	8.90		
	96.19	99.23	99.39	101.24	101.36	99.59	99.05		

Coal Measures..	{	1. Dark yellow shale from Coal Measures, W. H. Graves, near Birmingham, Jefferson county.
		2. Light gray shale from same locality.
		3. Shale from Coaldale, Jefferson county. Analysis by F. W. Miller.
		4. Shale over coal seam, Cedar Grove Coal Mines, near Vance, Tuscaloosa county.
Lower Carboniferous	{	5-7 Fire clay, near Valley Head, DeKalb county.
		8. China clay, Eureka mines, DeKalb county.

- |                        |   |   |
|------------------------|---|---|
| Silurian and Cambrian. | <p>9. China clay, Rock Run, Cherokee county (Dyke's ore bank.)</p> <p>10. Fire clay, Rock Run, Cherokee county.</p> <p>11. Pottery clay, Rock Run, Cherokee county.</p> <p>12. China clay, J. R. Hughes, Gadsden, Etowah county.</p> <p>13. Stoneware clay, Blount county.</p> <p>14. Stevens, Fire clay, Oxanna, Calhoun county; probably too much free sand.</p>  |   |
|                        | <p>15. Stoneware clay, Coosada, Elmore county.</p> <p>16. Pottery clay, McLean's, near Edgewood, Elmore co.</p> <p>17. Stoneware clay, Chalk Bluff, Elmore county.</p> <p>18. Fire clay, Woodstock, Bibb county.</p> <p>19. Fire clay, Bibbville, Bibb county.</p> <p>20. Fire clay, Hulls Station, A. G. So. R. R. Tuscaloosa co.</p> <p>21. Pottery clay, H. H. Cribbs, A. G. So. R. R., Tuscaloosa county.</p> |   |
|                        | Lower Cretaceous (Tuscaloosa)   | <p>22. Pottery clay, J. C. Bean, M. &amp; O. R. R., Tuscaloosa co.</p> <p>23. Fire clay, J. C. Bean, M. &amp; O. R. R., Tuscaloosa co.</p> <p>24. Stoneware clay, Roberts' Mill, Pickens county.</p> <p>25. Pottery clay, Cribbs' place, Lamar county.</p> <p>26. Stoneware clay, H. Wiggins, Fayette county.</p> <p>27-28. Pottery clay, W. Doty, Fayette county.</p> <p>29. Blue clay, R. R. cut, near Glen Allen, Marion county.</p> <p>30. China clay, Briggs Frederick, Marion county.</p> <p>31. Pottery clay, 10 miles S. W. Hamilton, Marion co.</p> <p>32. Pottery clay, Thos. Rollins, Franklin county.</p> <p>33. Pottery clay, J. W. Williams, Pegram, Colbert co.</p> <p>34. China clay, Pegram, Colbert county.</p> |

## CHAPTER II.

### THE PORTLAND CEMENT MATERIALS OF CENTRAL AND SOUTHERN ALABAMA.

The raw materials suitable for the manufacture of Portland cement, which occur in Central and Southern Alabama, are argillaceous limestones, purer limestones, and clays.

The limestones valuable as cement materials occur mainly at two horizons, viz., in the Selma chalk or Rotten limestone of the Cretaceous, and in the St. Stephens formation of the Tertiary. The clays available are residual clays from the decomposition of the two limestone formations above mentioned, the stratified clays of the Grand Gulf formation, and alluvial clays occurring in the river and creek bottoms. It is further possible that later investigation may show that some of the other stratified clays of the Tertiary formations are suitable, and this is especially likely to be the case with the clays of the lowermost Cretaceous or Tuscaloosa formation.

#### THE SELMA CHALK OR ROTTEN LIMESTONE.

*Geological horizon.*—The Cretaceous system in Alabama is susceptible of classification into four divisions, which are, in ascending order,

- 1, the Tuscaloosa, a formation of fresh-water origin, made up in the main of sands and clays in many alterations. In places the clays occur in deposits of sufficient size and of such a degree of purity as to make them of commercial value.

- 2, the Eutaw, which is of marine origin and composed of sands and clays more or less calcareous, but nowhere showing beds of limestone properly so called.

- 3, the Selma chalk, which is of marine origin, and is composed, in part at least, of the microscopic shells of Foraminifera. This formation, throughout the western part of the belt covered by it in Alabama, is about 1,000 feet in thickness, and is made up of beds of chalky and more or less argillaceous limestone. In a general way it

may be said that the lower and upper thirds of the formation contain 25 per cent. and upward of clayey matters mixed with the calcareous material, while the middle third will hold less than 25 per cent. of these clayey impurities.

4, the Ripley. This, like the preceding, is a marine formation, in which, generally, the calcareous constituents predominate, but in places it contains sandy and clayey beds.

From this summary it will be seen that the Selma chalk is the one of Cretaceous formations in Alabama which offers limestone in such quantity and of such composition as to be fit for Portland cement material.

*General description.*—As has been stated above, the Selma chalk is a calcareous formation throughout its entire thickness of about 1,000 feet. The rock, however, varies in composition between somewhat wide limits, and taking account of the composition we may readily distinguish three divisions of it. The rock of the upper or Portland division is highly argillaceous, holding from 25 per cent. and upward of clayey matters; portions of it are composed of calcareous clays or marls rather than limestone, and in these beds are found great numbers of fossils, mainly oysters. Along Tombigbee River these beds make the bluffs from Pace's Landing down nearly to Moscow, and on the Alabama they form the banks of the river from Elm Bluff down to Old Lexington Landing. The strata, as exhibited in these bluffs, consist of dark-colored, fossiliferous, calcareous clays, alternating with lighter-colored and somewhat more indurated ledges of purer, less argillaceous rock. At Elm Bluff, which is about 125 feet high, the upper half of the bluff is of this character. The lower half of the bluff is composed of rock more uniform in composition and freer from clay, and is the top of the middle part of the Selma formation (the Demopolis division), which is made up of limestone of more uniform character, containing, generally, less than 25 per cent. of clayey material.

In this middle or Demopolis division of the Selma formation the fossils are rarer than in either of the others, oysters and anomias being the most common forms. This variety of the rock forms the bluffs along Alabama River from Elm Bluff up to King's Landing. It is seen in its most typical exposure at White Bluff, where it is at least 200 feet in thickness, and

makes on the right bank of the river an almost perpendicular bluff. On Tombigbee River it extends from near Barton's Bluff past Demopolis up to Arcola and Hatch's Bluff. Its lowermost beds, a compact limestone of great purity, form the upper parts of Barton's and Hatch's Bluffs. On Little Tombigbee River the same rock makes the celebrated bluffs at Bluffport and at Jones Bluff (Epes), beyond which for several miles it is shown along the stream.

Judging from the width of its outcrop, this division of the Rotten limestone must be about 300 feet in thickness. It underlies the most fertile and typical "prairie" lands of the South. At intervals throughout this region the limestone rock appears at the surface in what are known as "bald prairies," so named from the circumstance that on these spots there is no tree growth. The disintegration and leaching out of the limestone leaves a residue of yellowish clay, which accumulates sometimes to a thickness of several feet in low places. This clay is used at the Demopolis plant in the manufacture of cement, and in most localities where suitable limestone is found the clay is present in sufficient quantity to supply the needs of the cement manufacturer.

At the base of this middle or Demopolis division occurs a bed consisting of several ledges of compact, hard, pure limestone, which weathers into curious shapes, and has received the names horse-bone rock and bored rock. This bed, as above mentioned, appears at the top of Hatch's Bluff; also at Arcola Bluff, and between Demopolis and Epes, at Jordan's Ferry, and other places. Where it outcrops across the country it makes a ridge easily followed and characterized by the presence on the surface of loose fragments of the limestone.

The lower part of the formation (the Selma division), like the upper, is composed of clayey limestone, in many places being rather a calcareous clay. The color is dark gray to bluish, and in most exposures there is a striping due to bands of light-colored, purer limestone alternating with the prevailing quality. Along Alabama River the strata of this division are seen in the bluffs from King's Landing up to Selma and beyond. On the Warrior River they are seen in the bluffs at Arcola, Hatch's, Millwood, and Erie, occupying in the last-named locality the upper part only of the bluff. On the Tombigbee, the bluffs at Gainesville, at Roe's, and Kirkpatrick's are formed mainly of







Plate II.—Caves in Limestone, below Roe's Bluff, Tombigbee River.

the rocks of this division, while above Roe's, at Jordan's, occurs the line of junction of this with the middle division. Near this line of division there is a very characteristic feature to be observed at many points, viz., about 10 or 15 feet below the hard ledges of pure limestone forming the base of the middle (Demopolis) division the dark-colored argillaceous rock shows a tendency to flake off and weather into caves, sometimes to be seen for long distances along the bluffs, as on Alabama River just above King's Landing, on the Tombigbee below Roe's Bluff, and at Jordan's Ferry. This peculiarity is illustrated in Plate II. The outcrop of the argillaceous rocks of this division gives rise to black prairie soils, in which beds of fossil shells, mainly oysters, are common.

It has been suggested that the argillaceous rocks of this and the uppermost division could be mixed with the purer limestone of the middle division in such proportions as to constitute a good cement mixture. In this case it would be easy to select localities near the junction of the two divisions where both varieties of the rock could be quarried, if not in the same pits, at least in pits closely adjacent. This would do away with the need of adding other clay to the limestone. Localities of this sort would be found along the border north and south of the belt of outcrop of the white Demopolis rock.

*Details of localities.*—The general characters of the rock of this formation have been mentioned above, and it remains to give details of the special localities examined, together with analyses of the limestones collected. In making the collections material from the middle or Demopolis division of the formation has been generally chosen, since most of the limestone of the formation which contains 75 per cent. and upward of carbonate of lime is to be found in this division. At the same time specimens of the more argillaceous material, especially of the lower (Selma) division of the formation, have been taken for comparison and analysis, with a view to ascertaining whether or not it will be practicable to provide a cement mixture by using the proper proportions of the purer and more argillaceous materials.

Inasmuch as suitable material for cement manufacture can be had in practically unlimited quantity all along the outcrop of the pure limestone of the Demopolis division, the location of the plants for the manufacture of this product will be determined by

other considerations than the quality of the rock. Chief among these will be the facilities for transportation, cheapness of fuel, cost of labor and abundance of it at command.

Examinations have consequently been confined to those localities which appear to be most favorably situated in these respects, and especially to those localities which are on navigable streams or on north-south railroad lines, or on both.

The first place considered on Tombigbee River is Gainesville, where the limestone appears on the river bluff in a thickness of 30 to 40 feet, beneath a heavy covering of Lafayette sands and pebbles. (Plate III.) A short distance inland from the river, however, the rock appears at the surface, and may be quarried without difficulty. Specimens have been taken from the different parts of the bluff near the ferry, which will show the composition of the limestone here (see analyses 1, 2, 3, and 4, Table E). Other specimens are from the Roberts place, 3 miles east of Gainesville—one of which was taken from the top of a 30-foot bluff; others from the surface 1 mile and 5 miles from the river (analyses 5 and 6.)

At Jones' Bluff, on the Tombigbee, near Epes station, on the Alabama Great Southern Railroad, the white limestone of remarkably uniform composition shows along the river bank for a distance of a mile or so, with an average height of perhaps 60 feet. (Plate IV.) Here the bare rock forms the surface, so that there would be no overburden to be removed in quarrying. The railroad crosses the river at this locality, which thus has the advantage of both rail and water transportation. From the lower end of this exposure down to Bluffport the white rock is seen at many points, e. g., below Lees Island, Hillman's (Plate V), Martin's Ferry, Braggs, etc. It generally has a capping of 15 to 20 feet of red loam and other loose materials.

Specimens have been analyzed from Epes and Hillmans (analyses 7, 8 and 9, Table E.)

At Bluffport (Plate VI) the white rock in places forms a bluff 100 feet or more in height along the right bank of the river for a distance of a mile or more. This is the counterpart of Jones' bluff, above mentioned, and the character of the material is shown by analysis No. 10. As at Epes, the rock extends up to the surface, so that the quarrying would be attended with little or no difficulty. Below the Bluffport bluffs the easterly course of the river brings it into the territory of the lower

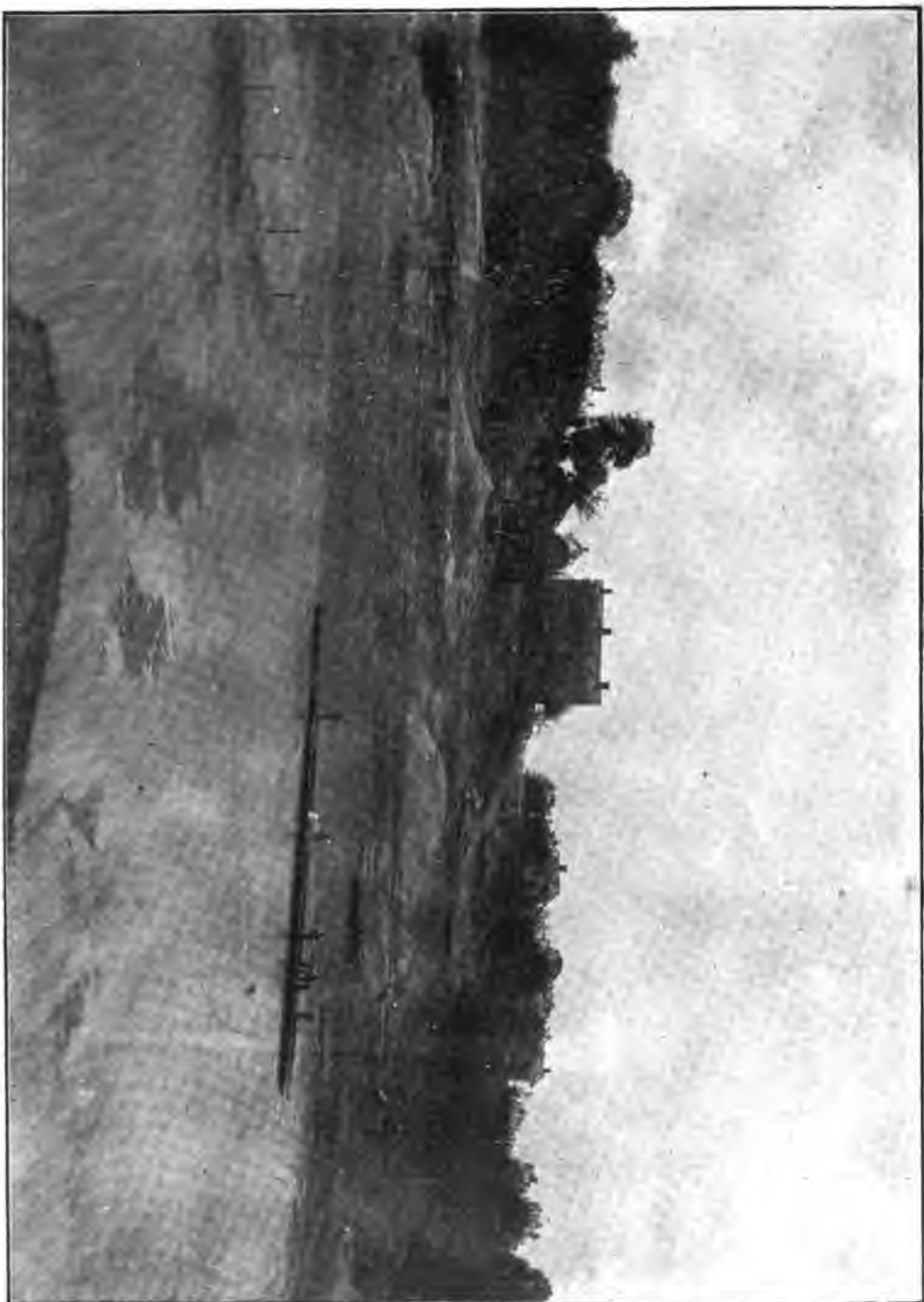


Plate III.—Galveston, Tombigbee River.





Plate IV.—Jones' Bluff, Tombigbee River, at Epps, looking down stream from bridge.





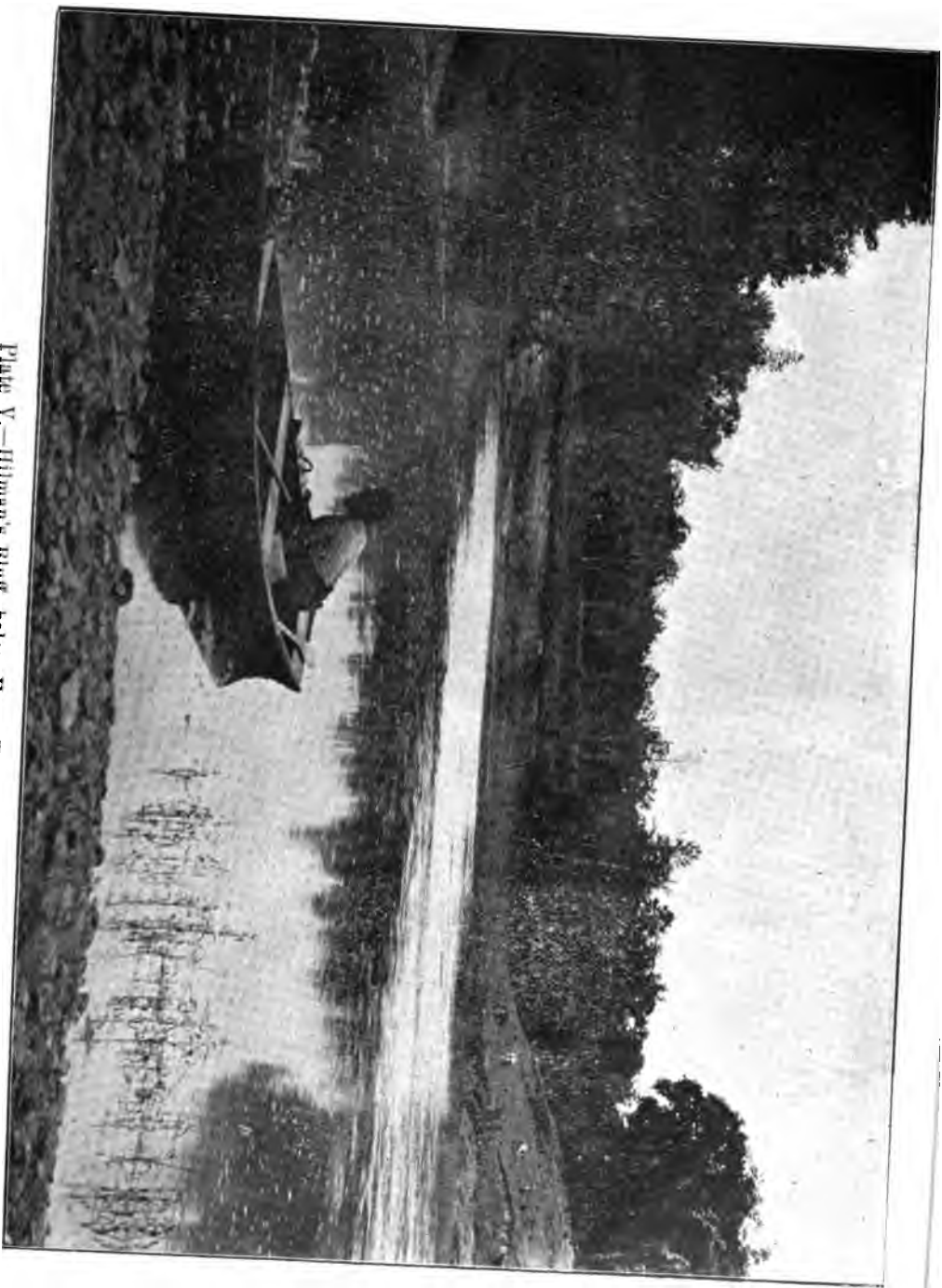
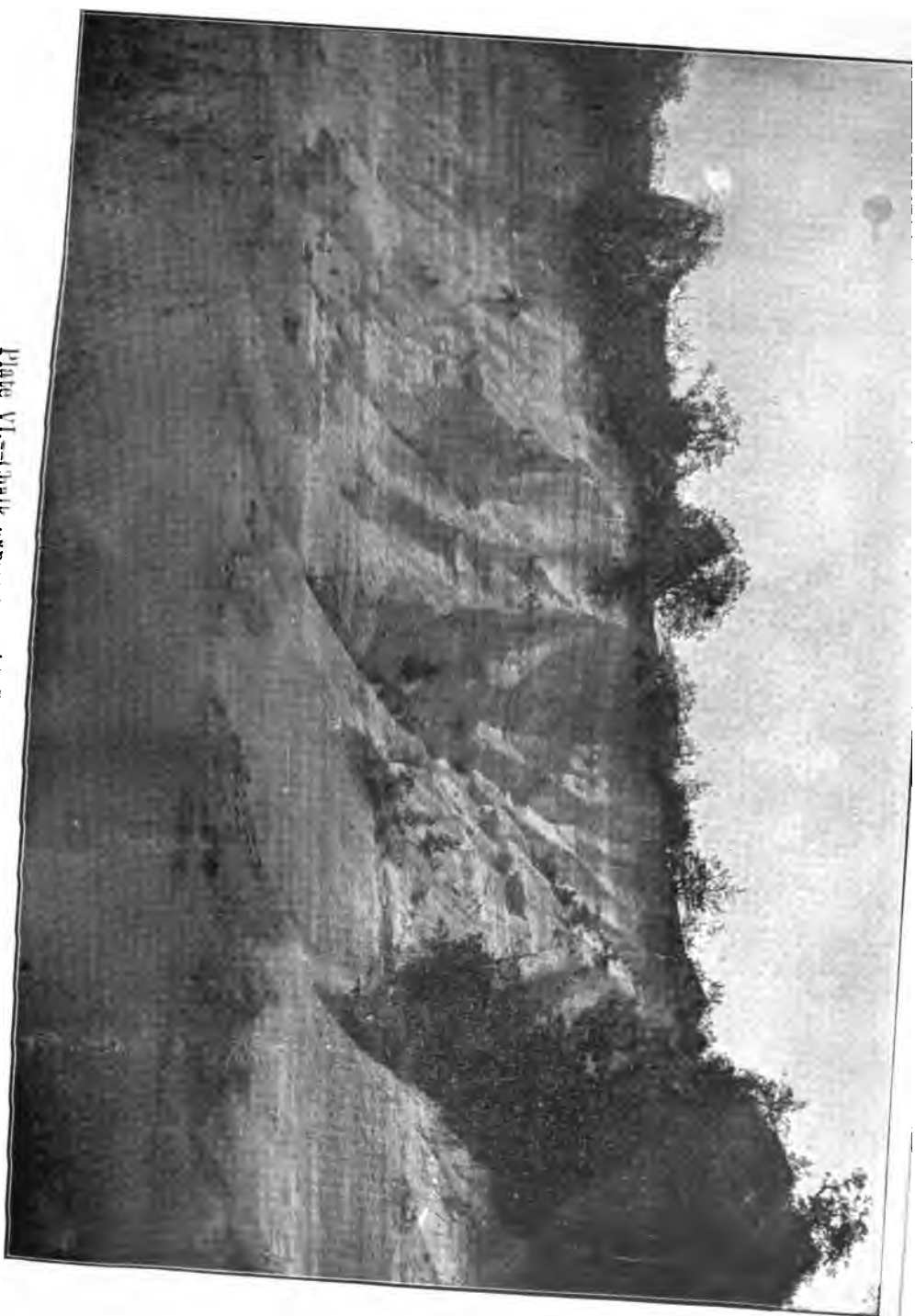


Plate V.—Hilman's Bluff, below Epes, Tombigbee River.



Plate VI.--Chalk exposure at Buffalo, Tombigbee River.





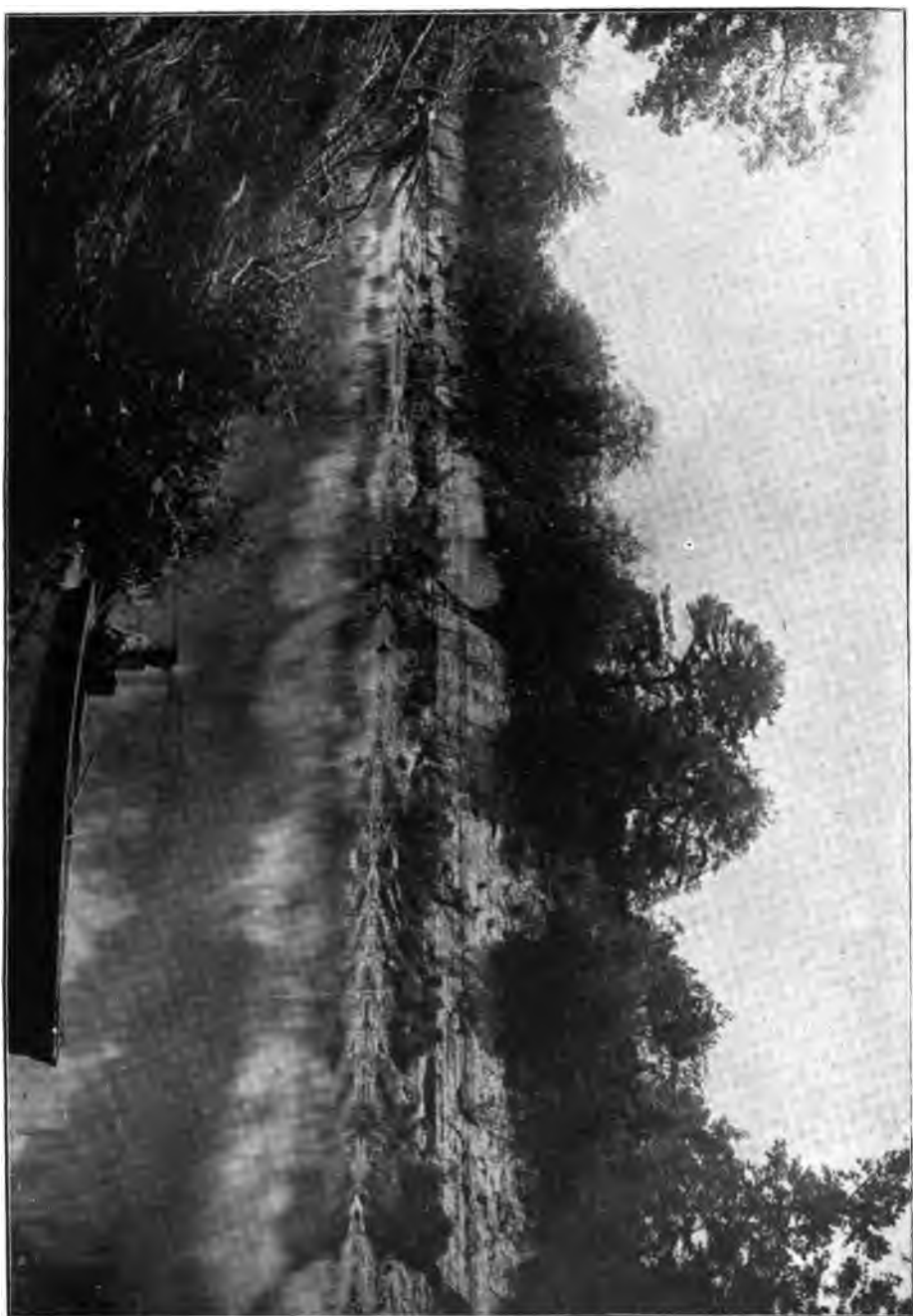


Plate VII.—Below Jordan's Ferry, Tombigbee River.



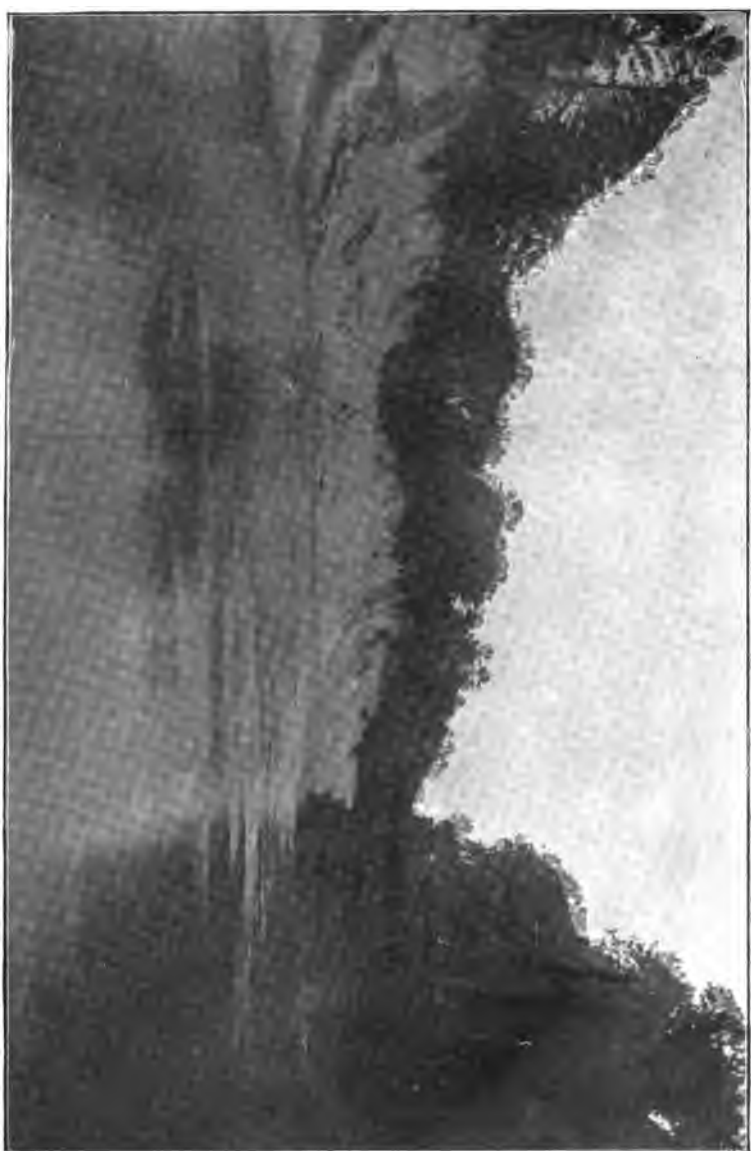


Plate VIII.—Koe's Bluff, Tombigbee River.





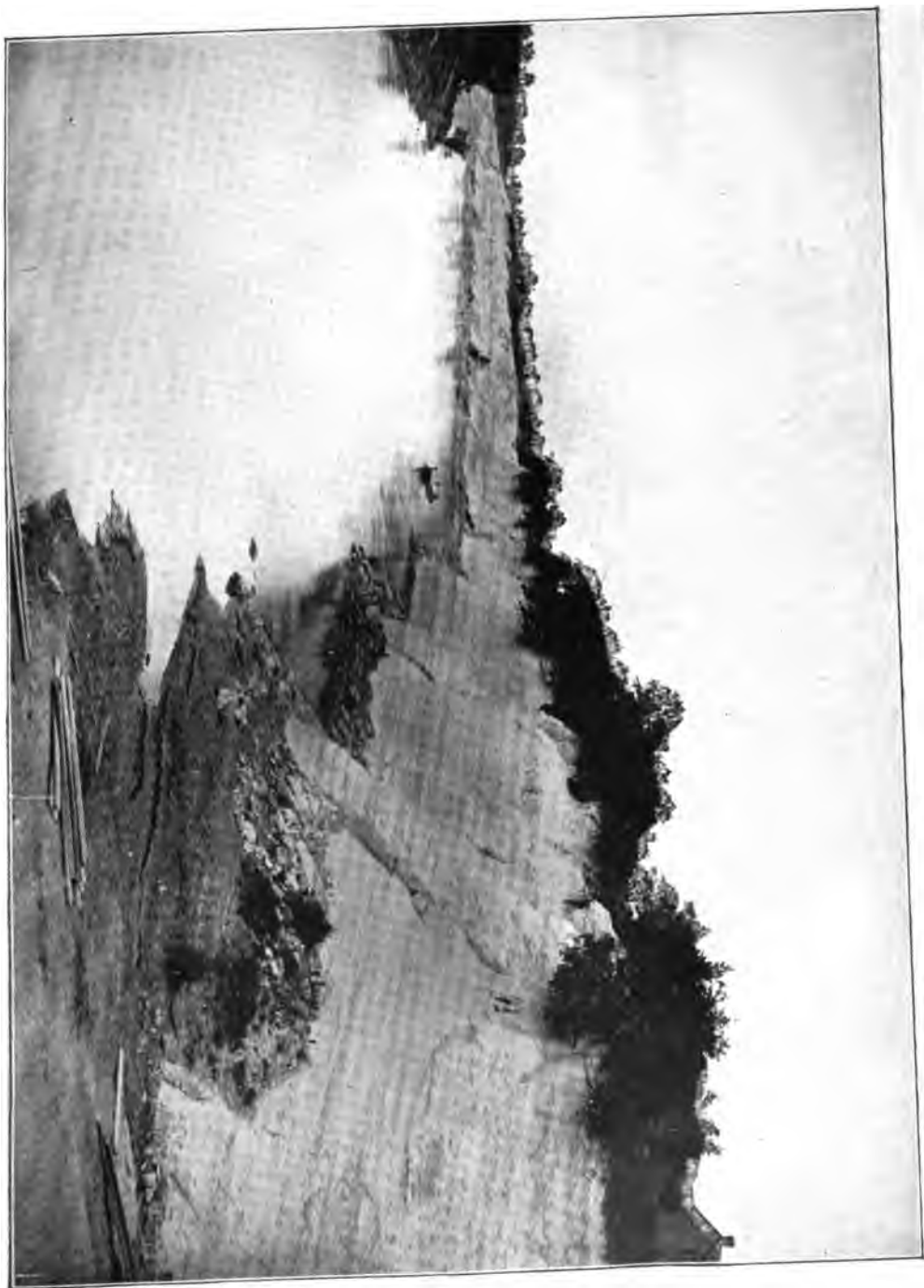


Plate IX.—Bluff of Selma Chalk, Demopolis, Tombigbee River, looking up stream from Steamboat Landing.



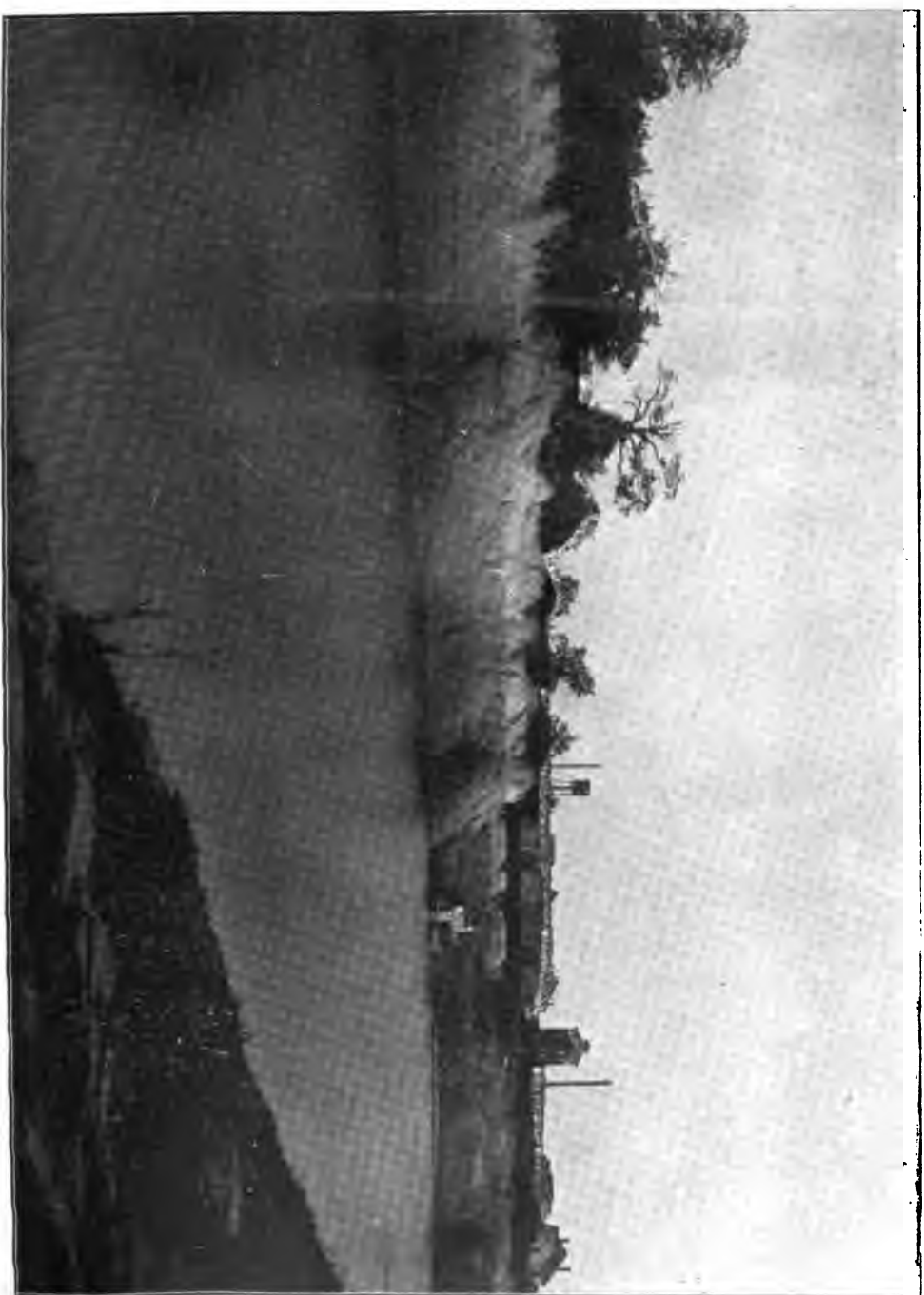


Plate X—Bluff at Demopolis, Tombigbee River, looking down stream Alabama River.





Plate XI.—Ilatch's Bluff, above Demopolis, on Warrior River.







Plate XII.—Alabama Portl





and Cement Works, at Demopolis.



strata of the formation, and we do not see the white rock again below Jordan's Ferry, (Plate VII) except in thin patches at tops of some of the bluffs. The character of the material of these lower beds may be seen from the analyses of specimens taken from Jordans and Belmont and Roe's bluff, Nos. 11, 12, 13, and 14. The two specimens from the last-named locality represent the composition of the prevailing dark-colored argillaceous rock and of the lighter-colored ledges. (Plate VIII.)

At Demopolis there is an important occurrence of the white rock extending along the left bank from a mile above the landing to about 2 miles below, with an average height perhaps of 40 or 50 feet. (Plates IX and X.) The rock is remarkably uniform in appearance and probably in composition (analysis 17.) At McDowell's the main bluff is on the right bank and the rock is of great purity, as shown by analysis 16. The exposures continue down to Pace's Landing, 9 miles below Demopolis, and beyond this the bluffs are much darker in color and striped with lighter bands, characteristic of the strata of the upper part of the formation. Thence down nearly to Moscow occur the exposures of these upper beds.

Above Demopolis at Arcola and Hatch's bluff the bluish clayey limestones of the Selma division are seen in force, with the lowermost ledges of the Demopolis division—the horse-bone rock—capping them. Two analyses of these varieties at Hatches will show well the contrast in their chemical composition (analyses 19 and 20. (Plate XI.)

From Demopolis eastward the line of the Southern Railway is located on the outcrop of this white rock, at least as far as Massillon, where it passes into the territory of the lower Selma division. Two miles from Demopolis on this road is the cement manufacturing plant of the Alabama Portland Cement Company, with six kilns in place. The quarry is on the opposite side of the railroad track from the kilns, but only a few hundred feet distant. (This plant with quarry in the foreground is shown in Plate XII.) The clay used is the residual clay from the decomposition of the limestone, and is obtained from the river bank a few yards away. The composition of the rock and of the clay used in the manufacture is shown by analyses 15, 18, and 31, Table E, and 1, Table G. A specimen taken from Knox wood station, between the cement works and Demopolis station, shows similar composition. The analyses below given (10, 11,

12 of Table G) show the chemical character of the cement manufactured at Demopolis.

At Van Dorn station the white rock outcrops in the fields over considerable territory, (Plate XIII), and just east of the station there is a deep cut through it. Analyses from about Van Dorn show sufficiently well the character of the material at these points (analyses 21 and 22 of Table E.)

About Uniontown the bare rock is exposed at numerous points, and the advantages of this place for the location of manufacturing plants seem to be very great. Specimens have been taken from the Bradfield and Shields places, west of the town, and from the Pitts place east, and from a point south of the town along the McKinley road. Other specimens have come from plantations near the road for several miles eastward and the analyses are appended (analyses 23, 24, 25, 26, 27 and 28).

The composition of the residual clay overlying the limestone at the Pitts place is shown in analysis No. 2 of Table G, and that of a similar clay from the "Graveyard Hill" on the Morgan place, by analysis No. 3 of same table.

South of Massillon, near the crossing of the Southern and Louisville and Nashville railroads, in the vicinity of Martin's station, the white rock shows in numerous exposures through the fields, making a country somewhat similar to that about Uniontown. At many points the rock has no overburden, and is admirably adapted to cheap quarrying. On the banks of Bogue Chitto Creek, near Martin's station, on the Milhous place, the rock is exposed in a bluff with a bed of plastic clay overlying, but here it is below a considerable thickness of red loam and sands of the Lafayette formation. The character of the rock at Milhous station, west of Martin's, may be seen from the analysis No. 29, Table E.

The same rocks make the great bluff of White Bluff, on Alabama River, (Plate XIV.) Specimens were selected from this bluff at two points—one about halfway down the bluff, the other twenty feet lower. Generally there is a capping of the red loam and sands of the Lafayette over the limestone, but near the upper end of the bluff the white rock extends to the summit, where it has a capping of plastic clay only. The character of the limestone from this locality is shown in analysis 30, Table E, and that of the overlying residual clay in analysis 4 of Table G.

At Elm Bluff, as has already been shown, the upper and middle divisions of the formation are in contact. (Plate XV.)



Plate XIII.—Exposure of Selma Chalk at Vandorn's Station, Marengo County.



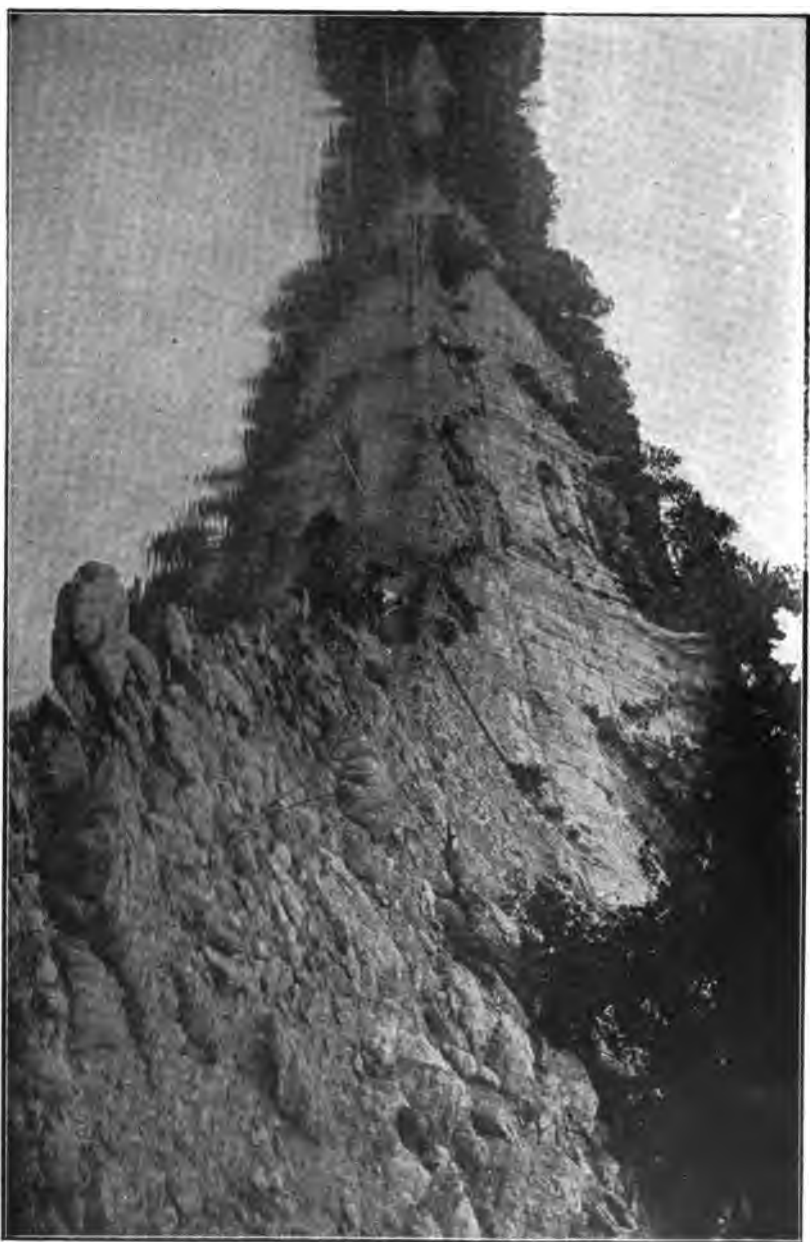


Plate XIV.—White Bluff, Alabama River.





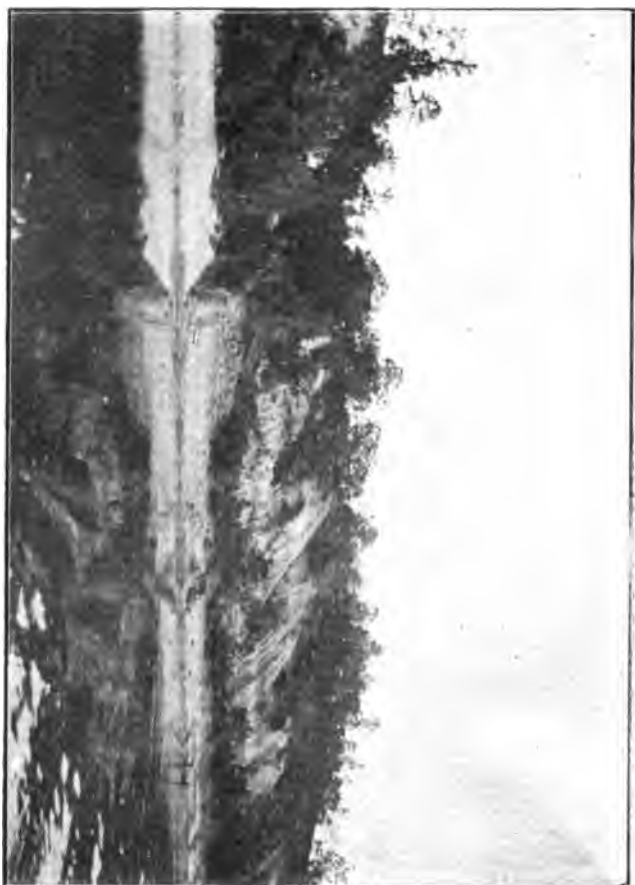


Plate XV — Elm bluff, Alabama River.



At King's Bluff the middle and lower parts of the formation are in contact. At the other bluffs of the river between King's Landing and Selma the rock of the lower division is exhibited. No. 32 (Table E) is of the rock at the steamboat landing in Selma; No. 33 of rock occurring near Selma; No. 34 from Cahaba; and No. 35 from Benton.

These analyses show that the rock of this division is in general too clayey for the best cement rock, but it might be mixed with the purer limestone of Unictown, or Demopolis in making up a cement mixture.

To summarize: From Demopolis eastward along the line of the Southern Railway, by Van Dorn, Gallion, Uniontown, Massillon, and thence by Martins and Milhous stations to White Bluff, the white or Demopolis type of rock appears at the surface in clean exposures at almost innumerable points, either immediately on the railroad or at very short distance from it. So far as the quality, quantity, and accessibility of the limestone rock are concerned, manufactories of cement might be located almost anywhere in this territory. From Demopolis westward the same conditions prevail up the river to Epes, and thence to Gainesville, beyond which point the white rock is to the west of the river at greater or less distance.

East of Alabama River the outcrop of the cement rock is crossed by the Louisville and Nashville Railroad (Repton branch), as before stated, between Berlin and Pleasant Hill stations. At Benton, on Alabama River, and on the railroad, the limestone has the composition shown by analysis 35.

On the Montgomery and Selma road, at the crossing of Pintala Creek near Manack station, the limestone is exposed in the creek banks and in the open fields, often with little or no overburden. In Table E are given analyses of a specimen from the fields along the wagon road (No. 36), and from the creek bank (No. 37.)

On the main branch of the Louisville and Nashville Railroad the white rock shows between the city and McGhees switch, and an analysis of a specimen from McGhees is given (No. 38.) Somewhat similar, but rather better, is the limestone from H. A. Jones, 8 miles south of Montgomery, shown in analysis No. 39.

Examinations have not been carried beyond Montgomery, but it is known that the white prairie rock is crossed by the Central of Georgia Railroad between Matthews and Fitzpatrick stations, and there seems to be no doubt that along this stretch of the road suitable rock will be found convenient to the line.

*Table E.*  
*Analyses of Crataceous Limestones.*

Locality.	Silica.	Iron and alumi- num oxides.	Calcium carbonate.	Magnesium carbonate.	Sulphuric anhydride.	Total sulphur.	Water and or- ganic matter.
1 Gainesville Bluff, Tombigbee river, 5 feet from top of bluff; R. S. Hodges, analyst .....	18.46	16.04	56.71	1.69	1.32	.....	5.78
2 Gainesville Bluff, Tombigbee river, lower part of bluff; R. H. Hodges, analyst .....	14.50	11.64	67.67	2.26	1.97	.....	1.96
3 Gainesville limestone; F. P. Dew- ey, analyst .....	18.42	10.79	65.21	1.57	.30	0.83	.....
4 Gainesville limestone; A. W. Dow, analyst .....	27.25	15.96	54.00	1.11	.44	1.23	.....
5 Robert's place, near Gainesville, top of bluff; R. S. Hodges .....	12.10	10.70	75.57	1.24	.69	.....	1.70
6 Robert's place near Gainesville, 5 feet above water; R. S. Hodges..	14.28	11.80	69.75	1.50	1.02	.....	1.65
7 Jones Bluff, at Epes; R. S. Hodges	4.78	6.42	86.28	1.02	.....	.....	1.30
8 Jones Bluff, at Epes; Dr. Mallett..	3.23	3.96	80.48	.53	.....	.....	2.22
9 Hillmans Bluff, below Epes; R. S. Hodges .....	10.08	9.47	77.43	1.30	.....	.....	1.99
10 Bluffport ferry, Tombigbee river; R. S. Hodges .....	8.10	5.40	85.10	1.25	.....	.....	.....
11 Jordans ferry, Tombigbee river; R. S. Hodges .....	.....	.....	67.28	1.87	.....	.....	1.53
12 Belmont Bluff, Tombigbee river; R. S. Hodges .....	21.00	15.60	55.84	2.12	.....	.....	5.44
13 Roes Bluff, Tombigbee river, main part of bluff; R. S. Hodges .....	20.40	15.76	55.82	2.10	.....	.....	5.93
14 Roes Bluff, Tombigbee river, light- colored ledges; R. S. Hodges .....	9.68	8.70	78.52	1.02	.....	.....	2.08
15 Demopolis limestone, F. P. Dew- ey; U. S. Mint, analyst .....	13.32	7.74	73.94	1.40	.27	.64	.....
16 McDowells Bluff, below Demo- polls; R. S. Hodges .....	3.82	3.86	90.40	1.15	.....	.....	.77
17 Demopolis limestone; Dr. J. W. Mallett, analyst .....	12.13	7.45	77.69	.72	.....	.....	2.49
18 Material used in Demopolis Cemen- tary Wks; R. S. Hodges, analyst.	7.64	7.62	80.71	1.05	1.62	.....	1.36
19 Hatch's Bluff, Warrior river above Demopolis; main part of bluff; R. S. Hodges .....	25.90	19.44	44.78	2.68	.....	.....	7.20
20 Hatch's Bluff, Warrior river above Demopolis; ledges at top of bluff; R. S. Hodges .....	1.78	2.34	93.52	1.38	.....	.....	.98

*Analyses of Crataecious Limestones.—Continued.*

Locality.	Silica.	Iron and alumi- num oxides.	Calcium carbonate.	Magnesium carbonate.	Sulphuric anhydride.	Total sulphur.	Water and or- ganic matter.
21 At VanDorn station, from road- side; R. S. Hodges .....	8.90	8.26	80.47	1.30	.....	.....	1.07
22 At VanDorn station, railroad cut east of station; R. S. Hodges.....	9.80	7.85	78.77	1.04	.....	.....	2.54
23 Uniontown, P. H. Pitts' Home place; R. S. Hodges .....	10.86	8.40	75.35	1.35	.....	.....	4.04
24 Uniontown, P. H. Pitts, Houston place; R. S. Hodges .....	13.58	9.20	72.21	1.98	.....	.....	3.03
25 Uniontown, P. H. Pitts, Rural Hill place; R. S. Hodges .....	12.10	9.80	74.52	1.17	.....	.....	2.41
26 Uniontown, 1 mile south on Mc- Kinley road; R. S. Hodges.....	7.56	7.18	83.45	1.53	.....	.....	.....
27 Uniontown, Bradfield place; R. S. Hodges .....	17.77	9.24	65.96	1.52	.....	.....	5.51
28 Uniontown, Shields place; R. S. Hodges .....	19.62	11.71	62.81	2.04	.....	.....	2.49
29 R. R. cut, Milhous station, So. Ry. Dallas county; R. S. Hodges.....	10.50	7.24	80.10	.98	.....	.....	1.18
30 White Bluff, Alabama river, lower part of bluff; R. S. Hodges.....	17.44	11.48	64.35	1.61	.....	.....	5.22
31 Limestone used in cement work. Demopolis; analysis furnished by T. G. Cairns .....	9.88	6.20	77.12	1.08	.....	.....	5.72
32 Limestone from bluff at steamboat landing, Selma; F. W. Miller, analyst .....	16.11	11.22	65.08	2.42	1.40	.....	3.37
33 Near Selma, white rock; O. M. Cawthon, R. S. Hodges.....	18.66	13.42	64.10	2.58	.08	.....	1.16
34 Limestone from Cahaba, Alabama river; Dr. Mallet.....	19.64	9.40	65.81	.79	.....	.....	3.58
35 Limestone from Benton, Alabama river; Dr. W. B. Phillips.....	19.74	11.67	54.83	5.14	.85	.....	4.96
36 Limestone from Manack station, Lowndes county; R. S. Hodges...	13.50	11.46	67.16	1.08	1.01	.....	5.79
37 Limestone, Manack station; Dr. B. B. Ross, analyst .....	13.20	9.00	74.26	1.46	.....	.....	.....
38 Limestone, McGhee's Switch, Montgomery co.; R. S. Hodges...	21.98	14.78	54.67	1.39	.11	.....	7.07
39 Limestone, H. A. Jones, 8 miles S. of Montgomery; R. S. Hodges....	14.90	14.34	63.28	1.47	.....	.....	6.01

## THE ST. STEPHENS LIMESTONE.

*General Description.*—The St. Stephens or White limestone formation of the Alabama Tertiary, which includes the uppermost of the Eocene strata, is in general equivalent to the Vicksburg limestone of the Mississippi geologists.

In Alabama it exhibits three rather well-defined phases, which in descending order are (1) the Upper or Salt Mountain division, observed at one locality only in Clarke county; (2) the Middle or St. Stephens division, and (3) the Lower or Jackson division. Of these it is only the middle division with which we are here concerned, since the first is, so far as known, restricted to one locality, and the third is seldom exposed along Alabama rivers and railroads.

The following section of the St. Stephens Bluff, Tombigbee River, (Plate VI), will give an idea of the strata of this division:

*Section of St. Stephens Bluff.*

	FEET.
1. Red residual clay . . . . .	1 to 5
2. Highly fossiliferous limestone holding mainly oysters, and full of holes, due to unequal weathering . . . . .	10 to 12
3. Orbitoidal limestone (chimney rock), a soft, nearly uniform porous limestone, making smooth perpendicular face of the bluff except where bands of harder limestone of very nearly similar composition alternate with the softer rock. Both varieties hold great numbers of the circular shells of <i>Orbitoides mantelli</i> . These harder ledges are nearly pure carbonate of lime, take a good polish, and are often burned for lime. . . . .	60
4. Immediately below 3, for 5 or 6 feet, the strata were not visible, being hidden by the rock falling from above, but the space seems to be occupied by a bluish clay. Then follows a soft rock somewhat of same consistency as No. 3 above, but containing a good deal of green sand. The fossils are mostly oysters and <i>Plagiostoma dumosa</i> . This bed is in places rather indurated superficially, and forms projecting ledges. . . . .	10 to 15

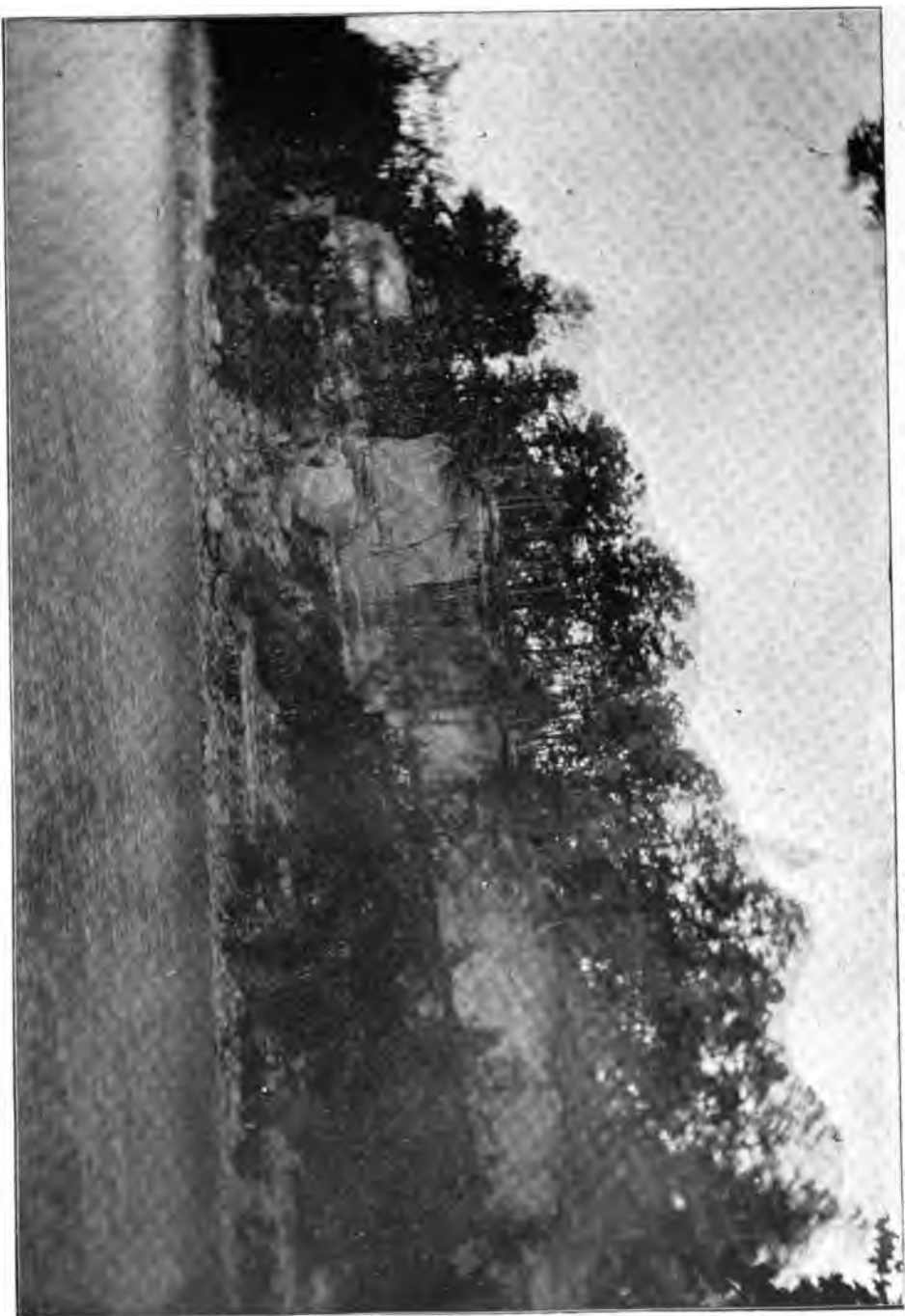


PLATE XVI.—St. Stephens Bluff, Tombigbee River.





5. Bluish clayey marl with much green sand, containing the same fossils as No. 4. It washes or caves out from under No. 4, which overhangs it..... 4 to 5
6. Massive joint clay, yellow on exposed surface, blue when freshly broken; no fossils observed. Extends below the water level to unknown depth; exposed ..... 3 to 4

The rock of this formation, which seems to be the best suited for cement material, is the soft "chimney rock" or orbitoidal limestone of bed No. 3 above. This usually quarried for chimneys and other constructions by sawing it out and dressing it down with a plane into blocks of suitable size, which are then laid like brick.

The numerous analyses given below will show that this rock is a purer limestone than most of the material of the Selma chalk of the Cretaceous formation above considered. In cement making it will, in consequence, require a larger proportion of clay to be mixed with it, and the question of obtaining suitable clay in sufficient quantity and in close proximity becomes one of some importance. The residual clay left after decomposition and leaching of the limestone seems to be fairly well adapted to the purpose. Besides this residual clay some analyses have been made of the clays of the river and creek bottoms of the country near the limestone outcrops, and of the clays of the Grand Gulf formation, which very generally in this section overlies the limestone. Some analyses of the last named clays have been made from material occurring near St. Stephens, and near Manistee Junction on the Repton Branch of the Louisville and Nashville Railroad. At this last-named locality the clay is present in sufficient quantity to be of value if the composition is suitable.

#### *Details of Localities.*

*St. Stephens.*—The first locality to be considered is the bluff at St. Stephens, a section of which has been given, and it may be taken as a typical section of the formation everywhere. At St. Stephens the whole of the soft orbitoidal limestone or "chimney rock" might be used, as the composition is uniform throughout. The overlying harder limestone has almost the same composition, but it is less easily crushed and handled. It may be quarried here from the surface down, as it is covered only by a thin layer of residual clay. The characters of the limestone and of the clay from here are sufficiently well shown

by the subjoined analyses (No. 1 of Table F, and 5 and 6, Table G.) The character of the clay of Grand Gulf formation near St. Stephens is shown in analysis No. 8 of Table G.

Below St. Stephens there is deep water to Mobile, with the exception of one bar, which may be removed without much trouble or expense.

*Oven Bluff.*—From Hobson's quarry, just above the Lower Salt Works Landing, down to Oven Bluff, a distance of 2 miles, the orbitoidal limestone or chimney rock occurs at the base of bluffs of Tertiary age.

At the quarry the hard limestone, which is being taken up for riprap work, lies, as at St. Stephens, just above the soft chimney rock. Along the stretch of river above described this chimney rock is seen in a bed 15 to 20 feet in thickness, just above the river bottom, and is easily accessible. Analysis 2, Table F, of a sample from Oven Bluff will show the quality of the limestone here. As regards clay three varieties have been examined, a residual clay from over the limestone, a swamp-bottom clay from the low grounds of Leatherwood Creek, and clay from strata of the Grand Gulf formation which here overlies the St. Stephens limestone. The analyses of these clays have not yet been made.

The first shoal in the river above Mobile is a few miles above Oven Bluff, so that from this place down there is a 9-foot channel at all seasons, which will give to Oven Bluff a certain advantage over other localities in regard to transportation. The shoal mentioned is one which can be removed, so that St. Stephens may be classed with Oven Bluff as regards transportation by water, except that the former is some miles farther from the Gulf than the latter.

Analyses by Dr. Mallett of other specimens of this chimney rock are here presented. No. 8, Table F, is of a specimen from Colonel Darrington's place, in the lower part of Clarke county near Gainestown, and 9 and 10 are from other localities in Clarke county near the rivers.

*Localities along the line of the Southern Railway.*—At Glendon station, a few miles east of Jackson, there is an exposure of the chimney rock close to the track. The rock here is about 20 feet thick, and the limestone is covered by a bed of red residual clay similar to that at St. Stephens and Oven Bluff. The same chimney rock may be seen along the road between the sta-

tion and Jackson, and no doubt it occurs from Glendon up to Suggsville station, within convenient reach of the railroad. At Suggsville station the same rock occurs along the road leading from station to the town. This place is within a short distance of the railroad.

Between Suggsville and Gosport, the country rock is the St. Stephens limestone, but no particular attention was given to it for the reason that no railroad crosses the county along this line.

*Along Alabama River.*—At Perdue Hill the St. Stephens rock outcrops near the base of the hills which descend to the terrace on which the town of Claiborne stands. The bluff at Caliborne Landing shows near the summit the calcareous clay or clayey limestone which lies at the base of the St. Stephens formation, and which is generally thought to be the equivalent of the Jackson group of the Mississippi geologists. It is quite possible that this rock, where it occurs in sufficient quantity, may be suitable for cement making, since it has a composition not far different from much of the Rotten limestone or Selma chalk. No investigations have yet been made concerning it, for the reason that there are comparatively few points where it appears in adequate thickness and in favorable localities as regards transportation.

At Marshall's Landing, just above the mouth of Randon's Creek, is the first exposure of the chimney rock along Alabama River. This occurs at the top of the bluff. It has the usual covering of residual clay, and the analyses presented (3 of Table F, and 7 of Table G,) will show the composition of the two. Below the orbitoidal or chimney rock at Marshall's there are 20 feet or more of a porous limestone, the analysis of which is given in Table F, No. 4. In the same bluff there are beds of calcareous clay which might possibly be used in mixing with the limestone. At the landing these would be difficult to quarry because of overlying strata, but they would certainly be found without cover along the bluffs above Marshall's if they should prove of value.

From Marshall's down to Gainestown Landing the river bluffs show beds of the limestone at numerous points. At Gainestown, the topmost bed of the St. Stephens, the hard crystalline limestone occurs not far above the water level in the river. This stone has been cut and polished and proves to be

a first-rate marble, inasmuch as it takes a good polish and shows agreeable variations in color. The soft chimney rock underlies the hard limestone here as at other points.

At Choctaw Bluff, some miles below Gainestown, there is the last exposure of the Tertiary limestones on the river. The material is an argillaceous limestone with numerous fossils, but it seems hardly likely to be of use in cement making.

*Between Alabama River and the main line of the Louisville and Nashville Railroad.*—A few miles east of Marshall's Landing, at Manistee Mills, the terminus of a sawmill road, there is a quarry of the chimney rock, conveniently situated as to transportation, since it is on the railroad. Across the country to the Repton Branch of the Louisville and Nashville Railroad, the St. Stephens limestone may, of course, be found at thousands of places, but no mention is made of these occurrences where they do not lie on railroad line.

Below Monroe Station, near Drewry on the Repton Branch, this road crosses the line of outcrop of the chimney rock, which at a number of points in the vicinity of Drewry lies within easy reach of transportation.

A few miles below Drewry, at Manistee Junction, there is a fine exposure of Grand Gulf clays in railroad cuts, both north and south of the station.

Analysis is given (No. 9 of Table G), of the clays from these cuts, from which their suitability from admixture with the limestone may be determined.

*On the main line of the Louisville and Nashville Railroad.*—The chimney rock may be found at many points below Evergreen in the vicinity of Sparta and Castleberry stations. There are many bluffs of this rock on the banks of Murder Creek in this vicinity, and there are several quarries from which the stone has been obtained for building purposes, within short distance of the railroad line. At the foot of Taliaferro's Heights the limestone forms high bluffs on the creek; at Ellis Williams Spring there are bluffs with the soft rock at the base and the hard horse-bone rock at the top, and on the creek bank, a few hundred yards away, is one of the quarries mentioned above. In fact the localities where the rock may be found within convenient distance of the railroad, and in a position favorable to

cheap quarrying, are numerous in all this region. No clays were seen except the usual residual clays from the decomposition of the limestone and a clay occurring close to Evergreen in the pits of Wild Brothers. Analyses 5, 6 and 7 of Table F, will show sufficiently well the character of the limestone in this section.

The Evergreen occurrences have attracted attention because of their location on the line of a great railroad system within short distance of tide water.

Farther to the east this limestone formation extends across Alabama and into Georgia and Florida, but as there is no north-south railroad east of the Louisville and Nashville at this time, the investigations have gone no further.

To summarize: While the St. Stephens limestone outcrops across the State from the Mississippi line to the Chattahoochee River, often occupying broad belts, attention has been concentrated on those localities which lie upon navigable streams or upon railroad lines terminating in Gulf ports. As compared with the Demopolis division of the Selma chalk, this limestone is more uniform in composition, higher in lime content, softer and more easily quarried and crushed, and in geographical position many miles nearer the Gulf.

Its thickness, on the other hand, is much less, although sufficient to supply an indefinite number of manufactories with raw material for cement.



Table F.  
Analyses of Tertiary Limestones.

Locality.	Silica.	Iron and alu- min um oxides.	Calcium carbonate.	Magnesium carbonate.	Sulphuric anhydride.	Water and or- ganic matter.
1 St. Stephens orbitoidal limestone, St. Stephens, Tombigbee river; R. S. Hodges, analyst .....	2.28	2.04	92.85	1.92	.13	.....
2 Orbitoidal limestone, "Chimney Rock," Oven Bluff, Tombigbee river; R. S. Hodges .....	4.18	3.26	89.32	2.38	.15	.81
3 Orbitoidal limestone, Marshall's Landing, Alabama river, Monroe county; R. S. Hodges .....	2.52	1.50	94.07	1.90	.08	.....
4 Limestone underlying the orbi- toidal rock, Marshall's Landing; R. S. Hodges .....	11.44	3.34	80.46	3.09	.15	1.52
5 Chimney Rock, Tallafarro's Heights, near Evergreen, Cone- cuh county; R. S. Hodges.....	2.84	2.16	91.31	1.83	.07	1.79
6 St. Stephens orbitoidal limestone, near Evergreen; Dr. W. B. Phil- lips, analyst .....	1.26	1.72	96.09	.65	.02	.65
7 Orbitoidal limestone near Ever- green; D. W. B. Phillips.....	2.75	2.73	93.31	.23	.02	.60
8 St. Stephens orbitoidal limestone, Col. Darrington's, near Oven Bluff, Clarke co; Dr. J. W. Mallet	1.69	2.12	94.84	.96	.....	.....
9 St. Stephens orbitoidal limestone, Clarke co. near river; Dr. J. W. Mallet, analyst .....	2.44	.27	94.85	.....	.....	.....
10 St. Stephens orbitoidal limestone, Clarke county near river; Dr. J. W. Mallet, analyst .....	4.15	1.29	93.19	1.09	.....	.....

*Table G.*  
*Analyses of Clays (Cretaceous and Tertiary) and Cement.*

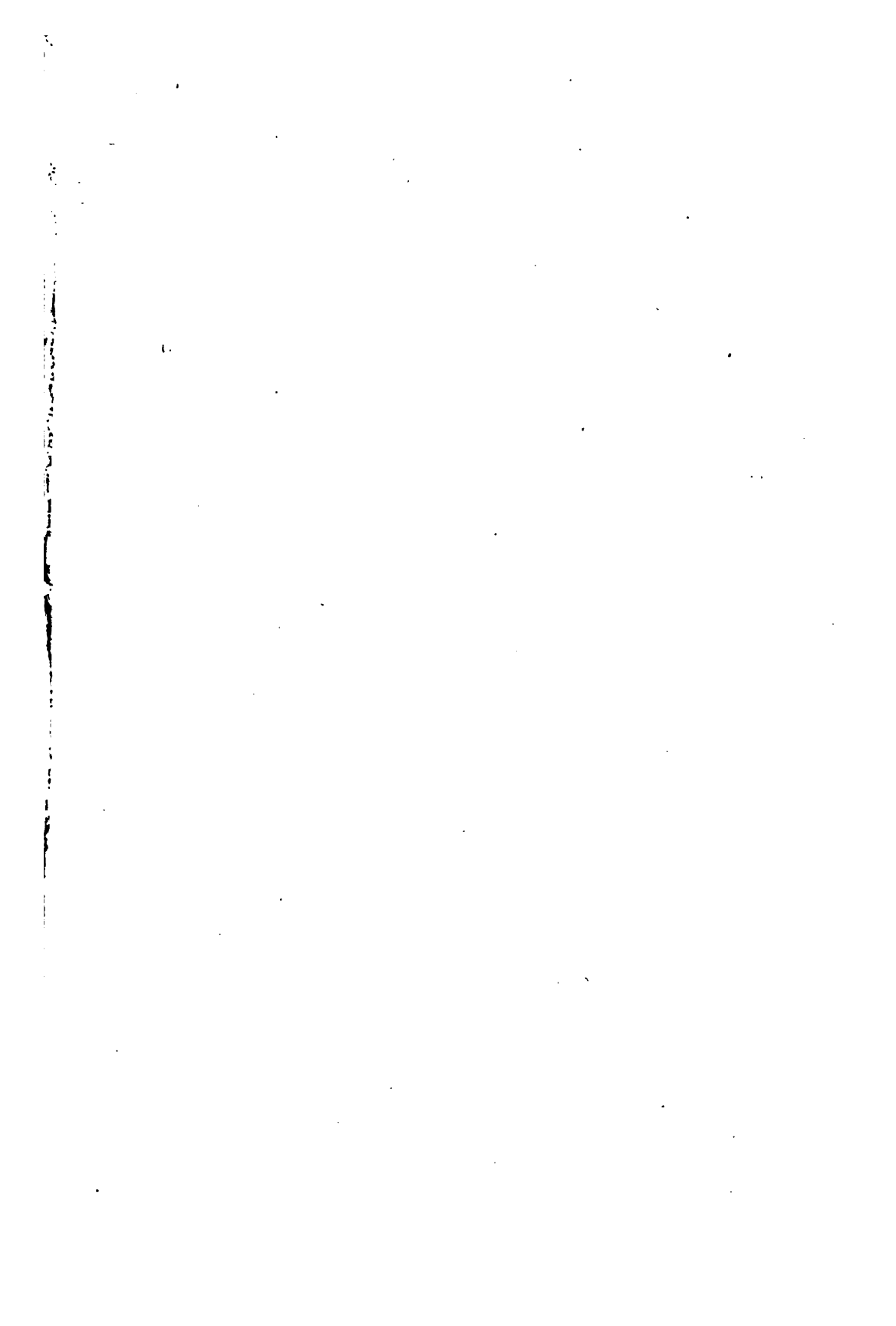
Number of analysis.	Silica.	Iron and alumi- num oxides.	Calcium oxide.	Magnesium oxide.	Sulphuric anhydride.	Total sulphur.	Ignition.
1 Residual clay over limestone, at Demopolis Cement Works; R. S. Hodges, analyst .....	55.64	26.25	.91	1.97	Tr	....	13.90
2 Residual clay over limestone at P. H. Pitts' Home Place, Uniontown; R. S. Hodges .....	69.57	19.04	.37	.....	.....	.....	9.68
3 Residual clay, Graveyard Hill, Morgan Place, Uniontown; R. S. Hodges .....	56.74	23.10	.70	1.27	Tr	....	13.80
4 Residual clay, Reid Place, White Bluff, Alabama river, Dallas co.; R. S. Hodges .....	56.90	27.71	.86	1.64	.09	....	11.26
5 Residual clay over orbitoidal limestone, St. Stephens, Washington county; R. S. Hodges .....	59.71	24.79	.37	.....	.....	.....	14.96
6 Residual clay over limestone, St. Stephens Bluff; R. S. Hodges .....	44.94	36.36	5.14	1.20	.....	.....	13.77
7 Residual clay over limestone at Marshall's Landing, Alabama river, Monroe county; F. W. Miller, analyst .....	51.30	33.22	1.37	.96	.41	....	9.42
8 Grand Gulf clay, west of St. Stephens, Washington co.; R. S. Hodges .....	60.68	25.60	.48	.38	Tr	....	9.92
9 Grand Gulf clay, Manistee Junc., Monroe county; F. W. Miller, analyst .....	66.60	25.86	.34	.34	.89	....	5.11
10 Cement manufactured by Alabama Portland Cement Co., Demopolis; A. W. Dow, U. S. Inspector of asphalts and cements, analyst .....	20.25	13.44	63.60	1.03	.41	0.99	.....
11 Cement manufactured by Alabama Portland Cement Co., Demopolis; analysis from T. G. Cairns, general manager .....	19.99	13.74	61.36	.61	.....	.....	.....
12 Cement manufactured by Alabama Portland Cement Co., Demopolis; R. S. Hodges, analyst .....	19.99	13.63	63.82	.83	1.16	....	.....











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